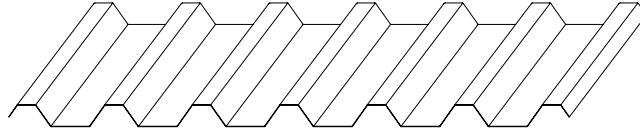


LOAD TABLES
ALUMINUM
ASTM B209
3003-H14
36" COVERAGE

SHORT RIB PANEL

BWK360



.032			FTY=17KSI		
POSITIVE BENDING			NEGATIVE BENDING		
Yt=	0.888 in.		Yt=	0.888 in.	
St=	0.173 cubic in./ft. (bend.)		St=	0.173 cubic in./ft. (bend.)	
Sb=	0.251 cubic in./ft. (bend.)		Sb=	0.251 cubic in./ft. (bend.)	
l=	0.153 in. ⁴ /ft. (defl.)		l=	0.153 in. ⁴ /ft. (defl.)	
M _a ⁺ =	0.280 ft-k/ft		M _a ⁺ =	0.193 ft-k/ft	
M _a ⁻ =	0.108 ft-k/ft		M _a ⁻ =	0.080 ft-k/ft	
P _{c,int} =	408 lb/ft		P _{c,int} =	408 lb/ft	
P _{c,end} =	198 lb/ft		P _{c,end} =	198 lb/ft	

L/120 DEFLECTION CRITERIA **.032**

LOAD (PSF)	DOWNWARD LOAD			UPWARD LOAD		
	SINGLE SPAN	DOUBLE SPAN	THREE SPAN	SINGLE SPAN	DOUBLE SPAN	THREE SPAN
10	8- 9	7- 8	8- 7	7- 11	8- 10	9- 10
15	7- 7	6- 2	6- 11	6- 6	7- 1	7- 11
20	6- 6	5- 3	5- 11	5- 7	6- 0	6- 9
25	5- 10	4- 8	5- 2	5- 0	5- 3	5- 11
30	5- 4	4- 2	4- 8	4- 7	4- 9	5- 4
35	4- 11	3- 10	4- 3	4- 3	4- 4	4- 10
40	4- 7	3- 6	4- 0	3- 11	4- 0	4- 6
45	4- 4	3- 3	3- 8	3- 9	3- 9	4- 2
50	4- 1	3- 1	3- 6	3- 6	3- 6	3- 11
55	3- 11	2- 11	3- 3	3- 4	3- 3	3- 8
60	3- 9	2- 9	3- 1	3- 3	3- 1	3- 6
65	3- 7	2- 7	2- 11	3- 1	2- 11	3- 4
70	3- 6	2- 6	2- 10	3- 0	2- 10	3- 2
75	3- 4	2- 5	2- 8	2- 11	2- 8	3- 0
80	3- 3	2- 4	2- 7	2- 9	2- 7	2- 11
85	3- 2	2- 2	2- 6	2- 8	2- 5	2- 9
90	3- 1	2- 2	2- 5	2- 7	2- 4	2- 8
95	3- 0	2- 1	2- 4	2- 7	2- 3	2- 7
100	2- 11	2- 0	2- 3	2- 6	2- 2	2- 6

L/180 DEFLECTION CRITERIA **.032**

LOAD (PSF)	DOWNWARD LOAD			UPWARD LOAD		
	SINGLE SPAN	DOUBLE SPAN	THREE SPAN	SINGLE SPAN	DOUBLE SPAN	THREE SPAN
10	7- 8	7- 8	8- 7	7- 8	8- 10	9- 6
15	6- 8	6- 2	6- 11	6- 6	7- 1	7- 11
20	6- 1	5- 3	5- 11	5- 7	6- 0	6- 9
25	5- 8	4- 8	5- 2	5- 0	5- 3	5- 11
30	5- 4	4- 2	4- 8	4- 7	4- 9	5- 4
35	4- 11	3- 10	4- 3	4- 3	4- 4	4- 10
40	4- 7	3- 6	4- 0	3- 11	4- 0	4- 6
45	4- 4	3- 3	3- 8	3- 9	3- 9	4- 2
50	4- 1	3- 1	3- 6	3- 6	3- 6	3- 11
55	3- 11	2- 11	3- 3	3- 4	3- 3	3- 8
60	3- 9	2- 9	3- 1	3- 3	3- 1	3- 6
65	3- 7	2- 7	2- 11	3- 1	2- 11	3- 4
70	3- 6	2- 6	2- 10	3- 0	2- 10	3- 2
75	3- 4	2- 5	2- 8	2- 11	2- 8	3- 0
80	3- 3	2- 4	2- 7	2- 9	2- 7	2- 11
85	3- 2	2- 2	2- 6	2- 8	2- 5	2- 9
90	3- 1	2- 2	2- 5	2- 7	2- 4	2- 8
95	3- 0	2- 1	2- 4	2- 7	2- 3	2- 7
100	2- 11	2- 0	2- 3	2- 6	2- 2	2- 6

L/240 DEFLECTION CRITERIA **.032**

LOAD (PSF)	DOWNWARD LOAD			UPWARD LOAD		
	SINGLE SPAN	DOUBLE SPAN	THREE SPAN	SINGLE SPAN	DOUBLE SPAN	THREE SPAN
10	7- 0	7- 8	8- 7	7- 0	8- 10	8- 7
15	6- 1	6- 2	6- 11	6- 1	7- 1	7- 6
20	5- 6	5- 3	5- 11	5- 6	6- 0	6- 9
25	5- 1	4- 8	5- 2	5- 0	5- 3	5- 11
30	4- 10	4- 2	4- 8	4- 7	4- 9	5- 4
35	4- 7	3- 10	4- 3	4- 3	4- 4	4- 10
40	4- 4	3- 6	4- 0	3- 11	4- 0	4- 6
45	4- 2	3- 3	3- 8	3- 9	3- 9	4- 2
50	4- 1	3- 1	3- 6	3- 6	3- 6	3- 11
55	3- 11	2- 11	3- 3	3- 4	3- 3	3- 8
60	3- 9	2- 9	3- 1	3- 3	3- 1	3- 6
65	3- 7	2- 7	2- 11	3- 1	2- 11	3- 4
70	3- 6	2- 6	2- 10	3- 0	2- 10	3- 2
75	3- 4	2- 5	2- 8	2- 11	2- 8	3- 0
80	3- 3	2- 4	2- 7	2- 9	2- 7	2- 11
85	3- 2	2- 2	2- 6	2- 8	2- 5	2- 9
90	3- 1	2- 2	2- 5	2- 7	2- 4	2- 8
95	3- 0	2- 1	2- 4	2- 7	2- 3	2- 7
100	2- 11	2- 0	2- 3	2- 6	2- 2	2- 6

0.040			FTY=17KSI		
POSITIVE BENDING			NEGATIVE BENDING		
Yt=	0.888 in.		Yt=	0.888 in.	
St=	0.218 cubic in./ft. (bend.)		St=	0.218 cubic in./ft. (bend.)	
Sb=	0.317 cubic in./ft. (bend.)		Sb=	0.317 cubic in./ft. (bend.)	
l=	0.194 in. ⁴ /ft. (defl.)		l=	0.194 in. ⁴ /ft. (defl.)	
M _a ⁺ =	0.354 ft-k/ft		M _a ⁺ =	0.244 ft-k/ft	
M _a ⁻ =	0.151 ft-k/ft		M _a ⁻ =	0.128 ft-k/ft	
P _{c,int} =	654 lb/ft		P _{c,int} =	654 lb/ft	
P _{c,end} =	318 lb/ft		P _{c,end} =	318 lb/ft	

L/120 DEFLECTION CRITERIA **.040**

LOAD (PSF)	DOWNWARD LOAD			UPWARD LOAD		
	SINGLE SPAN	DOUBLE SPAN	THREE SPAN	SINGLE SPAN	DOUBLE SPAN	THREE SPAN
10	9- 6	9- 10	11- 0	9- 6	10- 7	11- 9
15	8- 4	7- 11	8- 10	8- 3	8- 7	9- 2
20	7- 6	6- 9	7- 7	7- 1	7- 4	8- 2
25	6- 11	6- 0	6- 9	6- 4	6- 6	7- 3
30	6- 4	5- 5	6- 1	5- 10	5- 10	6- 7
35	5- 10	5- 0	5- 7	5- 4	5- 4	6- 0
40	5- 5	4- 8	5- 2	5- 0	5- 0	5- 7
45	5- 2	4- 4	4- 10	4- 9	4- 8	5- 3
50	4- 10	4- 1	4- 7	4- 6	4- 4	4- 11
55	4- 8	3- 10	4- 4	4- 3	4- 2	4- 8
60	4- 5	3- 8	4- 1	4- 1	3- 11	4- 5
65	4- 3	3- 6	3- 11	3- 11	3- 9	4- 2
70	4- 1	3- 4	3- 9	3- 9	3- 7	4- 0
75	4- 0	3- 3	3- 7	3- 8	3- 5	3- 10
80	3- 10	3- 1	3- 6	3- 6	3- 4	3- 8
85	3- 9	3- 0	3- 4	3- 5	3- 2	3- 7
90	3- 7	2- 10	3- 3	3- 4	3- 1	3- 5
95	3- 6	2- 9	3- 1	3- 3	3- 0	3- 4
100	3- 5	2- 8	3- 0	3- 2	2- 10	3- 3

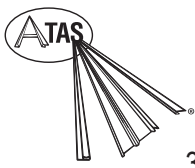
L/180 DEFLECTION CRITERIA **.040**

LOAD (PSF)	DOWNWARD LOAD			UPWARD LOAD		
	SINGLE SPAN	DOUBLE SPAN	THREE SPAN	SINGLE SPAN	DOUBLE SPAN	THREE SPAN
10	8- 4	9- 10	10- 3	8- 4	10- 7	10- 3
15	7- 3	7- 11	8- 10	7- 3	8- 7	9- 0
20	6- 7	6- 9	7- 7	6- 4	7- 4	8- 2
25	6- 1	6- 0	6- 9	6- 1	6- 6	7- 3
30	5- 9	5- 5	6- 1	5- 9	5- 10	6- 7
35	5- 5	5- 0	5- 7	5- 4	5- 4	6- 0
40	5- 3	4- 8	5- 2	5- 0	5- 0	5- 7
45	5- 0	4- 4	4- 10	4- 9	4- 8	5- 3
50	4- 10	4- 1	4- 7	4- 6	4- 4	4- 11
55	4- 8	3- 10	4- 4	4- 3	4- 2	4- 8
60	4- 5	3- 8	4- 1	4- 1	3- 11	4- 5
65	4- 3	3- 6	3- 11	3- 11	3- 9	4- 2
70	4- 1	3- 4	3- 9	3- 9	3- 7	4- 0
75	4- 0	3- 3	3- 7	3- 8	3- 5	3- 10
80	3- 10	3- 1	3- 6	3- 6	3- 4	3- 8
85	3- 9	3- 0	3- 4	3- 5	3- 2	3- 7
90	3- 7	2- 10	3- 3	3- 4	3- 1	3- 5
95	3- 6	2- 9	3- 1	3- 3	3- 0	3- 4
100	3- 5	2- 8	3- 0	3- 2	2- 10	3- 3

L/240 DEFLECTION CRITERIA **.040**

LOAD (PSF)	DOWNWARD LOAD			UPWARD LOAD		
	SINGLE SPAN	DOUBLE SPAN	THREE SPAN	SINGLE SPAN	DOUBLE SPAN	THREE SPAN
10	7- 6	9- 10	9- 4	7- 6	10- 1	9- 4
15	6- 7	7- 11	8- 2	6- 7	8- 7	8- 2
20	6- 0	6- 9	7- 5	6- 0	7- 4	7- 5
25	5- 6	6- 0	6- 9	5- 6	6- 6	6- 10
30	5- 3	5- 5	6- 1	5- 3	5- 10	6- 5
35	4- 11	5- 0	5- 7	4- 11	5- 4	6- 0
40	4- 9	4- 8	5- 2	4- 9	5- 0	5- 7
45	4- 7	4- 4	4- 10	4- 7	4- 8	5- 3
50	4- 5	4- 1	4- 7	4- 5	4- 4	4- 11
55	4- 3	3- 10	4- 4	4- 3	4- 2	4- 8
60	4- 2	3- 8	4- 1	4- 1	3- 11	4- 5
65	4- 0	3- 6	3- 11	3- 11	3- 9	4- 2
70	3- 11	3- 4	3- 9	3- 9	3- 7	4- 0
75	3- 10	3- 3	3- 7	3- 8	3- 5	3- 10
80	3- 9	3- 1	3- 6	3- 6	3- 4	3- 8
85	3- 8	3- 0	3- 4	3- 5	3- 2	3- 7
90	3- 7	2- 10	3- 3	3- 4	3- 1	3- 5
95	3- 6	2- 9	3- 1	3- 3	3- 0	3- 4
100	3- 5	2- 8	3- 0	3- 2	2- 10	3- 3

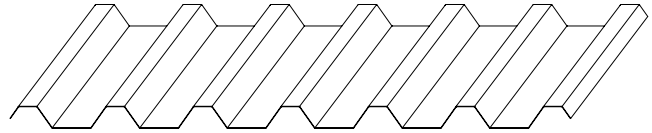
- Notes:
1. Minimum 1.5" bearing assumed.
 2. Connection of panel to supporting structure not investigated.
 3. Design thickness assumed 0.002" less than nominal thickness.
 4. Span lengths indicated by * are controlled by deflection.
 5. (+) signifies allowable moment based on tension.
(-) signifies allowable moment based on compression.
 6. Since allowable loads and spans can be affected by actual conditions of use, information in these tables is intended for use only by those qualified to assess these effects.
 7. Load tables are based upon section property analysis. Other factors such as fastener adequacy may apply to allowable span conditions per project.



LOAD TABLES
STEEL
ASTM A653
SS 50
36" COVERAGE

SHORT RIB PANEL

BWK360



L/180 Deflection Criteria

FY=50 KSI

20, 22, 24 GAUGE

AISI Section Properties (per foot of width)				
BWK360	(+) I	(+) S	(-) I	(-) S
24 ga.	0.112 IN. ⁴	0.125 IN. ³	0.092 IN. ⁴	0.112 IN. ³
22 ga.	0.148 IN. ⁴	0.169 IN. ³	0.123 IN. ⁴	0.154 IN. ³
20 ga.	0.184 IN. ⁴	0.214 IN. ³	0.155 IN. ⁴	0.199 IN. ³

(+/-) Allowable Wind Pressure- PSF										
PANEL GAUGE	No. of Spans	Span in Feet								
		4.0	4.5	5.0	5.5	6.0	6.5	7.0	7.5	8.0
24	1	139	97	71	53	41	32	25	21	
	2	186	147	119	98	82	70	60	50	41
	3	233	184	143	107	82	65	52	42	34
22	1	185	130	95	71	55	43	34	28	23
	2	256	202	164	135	114	97	83	67	55
	3	320	253	190	143	110	86	69	56	46
20	1	230	162	118	88	68	53	43	35	28
	2	331	262	212	175	147	125	103	84	69
	3	414	325	237	178	137	108	86	70	57

- Notes:
1. BWK360 section properties have been determined in accordance with the latest edition of the Cold Formed Steel Design Manual as published by the American Iron & Steel Industry (AISI).
 2. The section properties listed for BWK360 panel are to be used for the analysis of live loads acting perpendicular to the plane of the product.
 3. The Charted Load/ Span values account for the following:
 - A. Panel buckling strength
 - B. Deflection limit of L/180
 - C. Positive and negative wind considerations
 4. Load/ Span values do not include consideration of fastener capacity.
 5. Values include a 1/3 increase in "Allowable Wind Pressure".
 6. Since allowable loads and spans can be affected by actual conditions of use, information in these tables is intended for use by those qualified to assess these effects.
 7. Load tables are based upon section property analysis. Other factors such as fastener adequacy may apply to allowable span conditions per project.