

The information in this document pertains to use in the UNITED STATES ONLY. Allowable Stress Design. Refer to the ALLJOIST Specifier Guide Canada for use i. Janada, Li.nit States Design.







ALLIOist

INSTALLATION GUIDE

Includes AJS® 140 / 150 / 20 / 190 / 25 and VERSA-LAM® BEAMS































CANADA

US Version

product manufactured in St. Jacques, New Brunswick

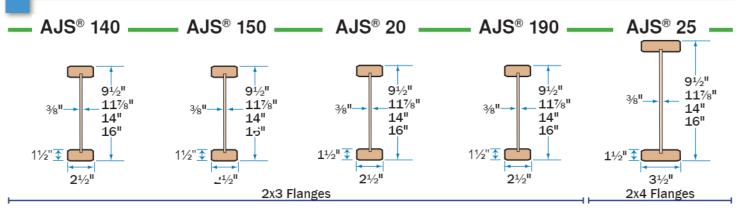
Lifetime Guaranteed Quality and Performance

Boise Cascade warrants its BCI® Joist, VERSA-LAM®, and ALLJOIST® products to comply with our specifications, to be free from defects in material and workmanship, and to meet or exceed our performance specifications for the normal and expected life of the structure when correctly stored, installed, and used according to our Installation Guide.

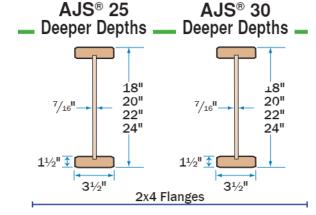
Boise Cascade has not evaluated the effects of any pressure or topical applications or treatments on its BCI® Joist, VERSA-LAM®, and ALLJOIST® products.

For information about Boise Cascade's engineered wood products, including sales terms and conditions. warranties and disclaimers, visit our website at www.BCewp.com

To locate your nearest Boise Cascade Engineered Wood Products distributor, call 1-800-232-0788







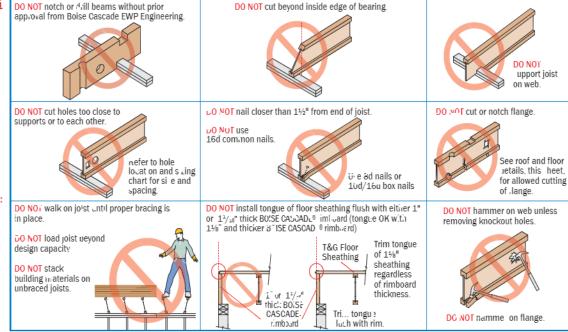
WARNING

THE FOLLOWING USES ARE NOT ALLOWED

SAFETY WARNING

DO NOT ALLOW WORKERS
ON AJS® JOISTS UNTIL ALL
HANGERS AJS® RIM OISTS
RIM BOARDS, AJS® BLOCKING
PANELS, WBRACING AND
TELLPORARY TX4 STRUT
LINES ARE INSTALLED AS
SMECIFIED BELOW SERIOUS
ACCIDENTS CAN RESULT FROM
INSUMFICIENT ATTENTION TO
PROPER BRACING DURING
CONSTRUCTION ACCIDENTS
CAN BE AVOIDED UNDER
NORMAL CONDITIONS EY
FOLLOWING THESE GLIDELINES:

- Juild a braced end wall at the end of the bay, or permanently install the first eight feet of AJS® Joists and the first course of sheathing. As an alternate, temporary sheathing may be nailed to the first four feet of AJS® Joists at the end of the bay.
- All hangers, AJS[®] rim joists, rim boards, AJS[®] blo king panels, and x-bracing must be competely installed and properly nailed as each AJS[®] Joist is set.
- Install temporar, 1x4 strut lines at no more than eight feet on center as additional AJS[®] coists are set. Nail the strut lines to the sheathed area, or braced end wall, and to each AJS[®] Juist with two 8d nails.



- The ends of cantilevers must be temporarily secured by strut lines on both the top and bottom llanges.
- Straighten the AJS® Joists to within ½ inch
 of true alignment before attaching strut lines
 and sheathing.
- Remove the temporary strut lines only as required to install the permanent sheathing.
- Failure to install temporary bracing may result in sideways buckling or roll-over under light construction loads
- DO NOT stack construction materials (sheathing, drywall, etc) in the middle of AJS® Joist spans, contact Boise Cascade EWP Engineering for proper storage and shoring information.

ALLJOIST® Residential Floor Span Tables

About Floor Performance

..omeowner's expectations and opinions vary greatly due to the subjective nature or rating a new floor. Communication with true intimate end user to determine their expectation is critical. *Vibration* is usually the cause of most complaints. Installing lateral bridging may help; however, squeaks may occur in not installed properly. Spacing the joists closer together does little to affect the perception of the floor's performance. The most common methods used to increase the performance and reduce vibration of wood floor systems is to

increase the joist depth, limit joist deflections, glue and screw a thicker, tongue-and-groove subfloor, install the joists vertically flumb with level-bearing supports and install a direct-attached ceiling to the bottom flunges of the joists.

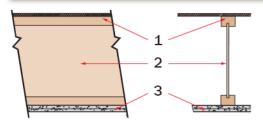
The floor span tables listed below offer three very different performance options, based on performance requirements of the homeowner.

		7	***.HREE	S'.AR ***	r	*	***FOUR	STAR ***	*	CHUTTON	★MiNIMJM ALLOWED I		CA UTION	
		standard for than L/300 perto mand ap, lications	deflection lindustry and determinent of the minimum of the may still be something the may be something th	sign communicor poists, 33	nity 3% Litts er, iloo- certain 1178	A floor that	is .00% stie	inited to L/96 or than the thr at 100% stiffe minating hom	ee star r than the	Live Load deflection limited to L/330. Fioors that meet the minimum building code L/360 criteria are structurally sound to carry the speci, ed loads; however, there is a much higher risk of floor performance issues. This table should only be used for applications where floor performance is not a co icern.				
Joist Depth	ALLJOIST® Series	í2" 0.c.	16" o c.	19.2 o r.	24" o.c.	1.2" 0.3	16" ა.c.	19.2" o.c.	24" ° C.	12" o.c.	16" o.c.	19.2" o.c	24" o.c.	
	140	17'–9"	16'–3"	15'–4"	1'-11"	13'–11"	12' -8"	11' 11"	11'- 1"	19'–8"	17'–0"	15'–6"	13'-11"	
	150	18'–1"	16'–7"	15'–8"	14'–7"	14'–2	12'-11	1 !' -2"	11' 3"	20'-0"	1ਤ'–3"	16'–8"	14'–11"	
. 1/2"	20	19'–1"	17'–5"	16'–ა"	15'–4"	14'–10"	15'-6'	12'- 9"	11'–10"	21'–1"	19'–3"	13'–2"	16'–4"	
	190	19'–4"	1/'–8"	16'-8"	15'–6"	15'–1"	13' -9"	1?' 11"	12'- 0"	21'–4"	19'–7"	18'–6"	17'–3"	
	25	21'-0"	19'–1"	18'–0"	16'-9"	16'–4"	14'–10"	14' -∪"	12' 11"	23'-2"	21'–1"	19'-3"	17'?"	
	140	?1'–2"	19'–4"	17'–8"	15'–1∪"	16'–7"	15' -1"	14'- 3"	"נ–'13	2?!-5"	19'–5"	17'–8"	15'–10"	
	150	?1'-7"	19'–8"	18'–7"	17'-0"	16' 10"	15'-4"	14'–6"	13'–5"	23' -10"	20'−10"	19'-0"	17'-0"	
11%"	20	22'-8"	20'-9"	19'–7"	18'–3"	17' 9"	16'-2"	15'–2"	14'–1"	25' 1"	22'–10"	20'-10"	18'–8"	
	190	23'-0"	21'-0"	19'–10"	18'–6"	18'–0"	16'–4"	15'–5"	14'-4"	25'–5"	_23'-3"	21'–11"	19'–0"	
	25	24'–11"	22'-9"	21'–5"	18'–3"	19' 6"	1.'-8"	16'–8"	15'–5"	27' -7"	<u>-</u> '4'-0"	21'-11"	18'–3"	
	140	24'-0"	21'–4"	19'–5"	17'–4"	18' 10"	15'-2"	16'–2"	15'-0"	24' -3"'	21'–4"	19'–5"	17'–4"	
	150	24'-6"	22'-4"	20'-11"	18'–9"	19'–2"	17'–6"	16'–5"	15'–3"	26'–6"	22'-11"	20'-11"	18'–9"	
4.7"	20	25'-9"	23'–6"	22'-2"	19'–1"	20' 2"	18'-4"	17'–3"	16'–0"	28' 5"	25'-1"	22'-11"	19'–1"	
	190	26'-1"	23'-10"	22'-6"	19'–1"	20' 5"	18'-7"	17'–6"	16'–3"	28' 10"	26'-4"	23'-11"	19'–1"	
	25	28'-4"	25'-10"	22'–11"	18'–4"	22'–1"	20'-1"	18' -11"	17' 6"	30'-5"	26'-4"	22'-11"	18'–4"	
	140	26'-6"	22'-11"	20'–11"	18'–9"	20'–10"	19' -0"	17'- 11"	16'–8"	26'–6"	22'-11"	20'-11"	18'–9"	
	150	27'-1"	24'-7"	22'-5"	19'–3"	21'–3"	19' -4"	18' 3"	16'-11"	28'–5"	24'-7"	22'-5"	19'–3"	
·6"	20	28'–6"	26'-0"	24'-2"	19'–3"	22'–4"	20'–4"	19' -1"	17' 9"	31'–3"	27'-0"	24'–2"	19'–3"	
	190	28'-11"	26'–5"	24'-2"	19'–3"	22'–8"	20' -7"	19'- 5"	18'–0"	31'–11"	28'–11"	24'-2"	19'–3"	
	25	31'–4"	27'–10"	23'–2"	18'–6"	24'–6"	22' -3"	20' 11"	18' 6"	32'-9"	27'–10"	23'–2"	18'–6"	

- Table values based on residential floor loads of 40 psf live load and 10 psf dead load (12 psf dead load for AJS[®] 25 joists).
- Table values assume that "fas" min. plywood/USB rated sheatning is glued and nailed to joists.
 Table values represent the most restrictive or simple or multiple span applications.
- Table values are the maximum allowable clear distance between supports. Analyze multiple
- Table values are the maximum anowable clear distance between supports. Analyze multiple
 span joists with Bu CaLC sizing software if the length of any span is less than bair the length
 of an adjacent span.
- Table values assume minimum bearing lengths without web stiffeners for joist dept.,s of 16 inches and less.
- Floor tile will increase dead load and may require specific deflection limits, contact Boise Cascade EWP Engineering for further information
- This table was designed to apply to a broad range of applications. It may be possible to
 exceed the limitations of this table by analyzing a specific application with the BC CALC[®]
 sizing software.

Shaded values do not satisfy the requirements of the North Carolina State Building Code Reier to the TriBLE STAR table when spans exceed 20 feet.

One-Hour Floor/Ceiling Assembly



See the US version of the Boise Cascade Fire Design & Installation Guide for specific assembly information and other fire resistive options or contact your local Boise Cascade representative.

FIRE ASSEMBLY COMPONENTS

- Min. ¹³/₃₁-inch T&G Wood Structural Panels. A construction agreesive must be applied to the top of the joists prior to placing sheathing. The sheets shall be installed with their long edge perpendicular to the joists with endroists centered over the top tlatige of joists and staggered one joist spacing with adjacent sheets.
- 2 AuS® Joists at 24" o.c. or less.
- 3. Two layers ½" Type € or two layers %" Type X gypsum board

SOUND ASSEMBLY COMPONENTS When constructed with resilient channels

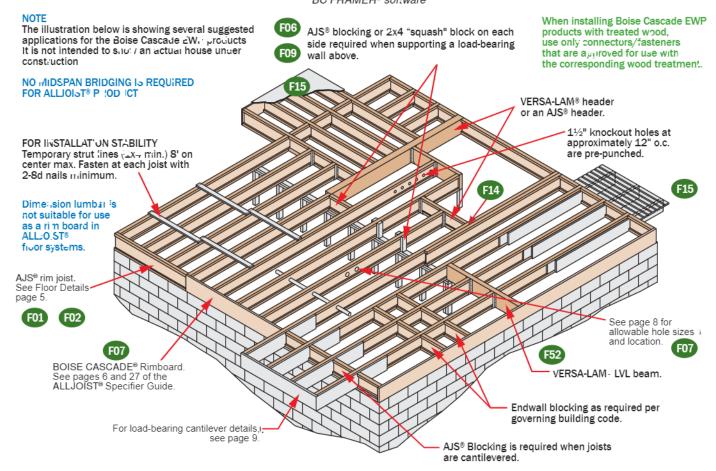
- Add carpet & pad to fire assembly:
- Add 3½" glass fiber insulation to fire assembly:
- Add an additional layer of minimum %" sheathing and 9½" glass fiber insulation to fire assembly:

STC=54	IIC=68
STC=55	IIC=46
STC=61	IIC=50

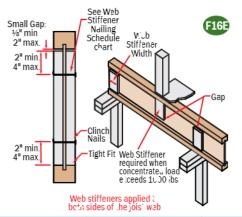
10

Floor Framing

Additional roof framing details available with BC FhAMER® software



Web Stiffener Requirements



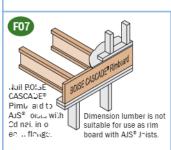
Structural Panel Web Stiffener										
	Minimum									
AJS® Series	In Hatiger	No Hangei	Minimum Width							
140 / 150 / 20 / 190	1"	1½	25/16 ¹⁰							
25	2x4 l	tical)								

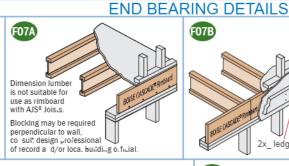
Web	Web Stiffener Nailing Schedule										
AJS® Series	Joisເ Depth	Nailing									
140 %50 20	∂½" – 11%'	3-10d									
190 25	14" - 16"	5-10d									

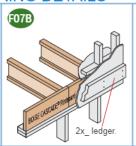
NOTES

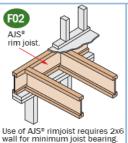
- Web stiffeners are optional except as noted below.
- Web stiffeners are always required for 18" and deeper AJS[®] joists at all bearing locations.
- Web stiffeners are always required in hangers that do not extend up to support
 the top flange of the AJS® Joist. Web stiffeners may be required with certain
 sloped or skewed hangers or to achieve uplift values. Refer to the hanger
 manufacturer's installation requirements.
- Web stiffeners are always required in certain roof applications. See Roof Framing Distails on page 7.
- Web stiffeners are always required under concentrated loads that exceed 1000 pounds. Install the web stiffeners snug to the top flange in this situation. Follow the nailing schedule for intermediate bearings.
- Web stiffeners may be used to increase allowable reaction values. See AJS[®] Design Properties on page 26 of the ASG or the BC CALC[®] software.

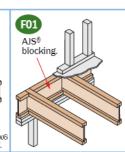
Additional floor framing details available with BC FRAMER® software

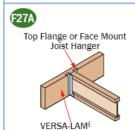


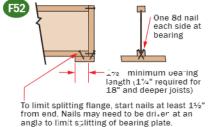


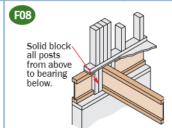


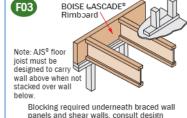






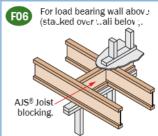


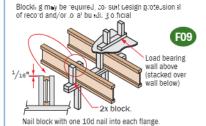




panels and shear walls, consult design professional of record.

INTERMEDIATE BEARING DETAILS



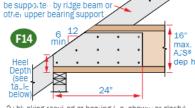




- 1. loculas.. bi polas are to be i.. full lientada with: pperfloorangiowerwar prite
- Capacities shown and for a builble. Squash plocks at each paist, SPF or potter.



Joist shall not be useu as a collar or rafter tension tie. ノx 3 mai raiter Raiter shall

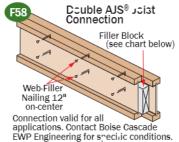


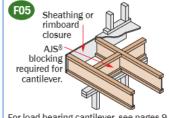
2/ blocking required at bearing (not shown for clarity). ²³/₂₂" min-plywood/ OSB rated sheatning as reinforce ment inclaif reinforcement with face, grain horizontal. Install on bour slues of the joist, light to bottom riange. 4" ga,, between :einforcement and bottom of lop flange Apply construction adhesine to contact surfaces and lasten with 3 rows of min. 1Cd box nails at 6 lo.c.

1100111010	accinate naming from each olde and olinon.												
Minimum Heel Depth													
End Wall		Roof Pitch											
Bearing	6/12	7/12	8/12	9/12	. 0/12	2/12							
2 x 4	43/8"	43/·6	4.4	4:/4"	41/4"	41/4							
2 x vi	3-,3"	33/16	25/16"	2¾"	29/16	21/4"							

ыаскег block F10 (minimum 12" wide) Nail with Joist 10 - 10d nails. Han ∠e Filler block. Nail with 10 - 10d nails

Backer block required where top flange ioist hanger load exceeds 250 lbs. Install tight to top .lange.





For load bearing cantilever, see pages 9 and 10. Uplift on backspan shall be considered in all cantilever designs.

LATERAL SUPPORT

- AJS® Joists must be laterally supported at the ends with hangers, AJS® rim joists, rim boards, AJS® blocking panels or no racing. AJS blocking panels or x-braking are required at cantilever supports
- Clocking may be required at intermediate bearings for floor diaphragm per IRC in high seismi areas, consult local building official.

MINIMUM BEARING LENGTH FOR AJS® JOISTS

- $1\frac{1}{2}$ inches is required at end supports. $3\frac{1}{2}$ inches is required at cartile.er and intermediate supports.
- Longer bearing lengths allow higher reaction values. Refer to the building code evaluation report or the LC CALC® software

NAILING REQUIREMENTS

- AJS® i'm joist, rim board or closure panel το AJS® Joist:
- Rims or closure panel 11/4 inches thick and less: 2-8d nails. one each in the top and bottom flange. AJS® 140/150/20/190 rim joist: 2-16d bo.. nails,
- one each in the top and bottom flange AJS® 25 rim joist. Toe-nail top flange to rim joist with 2 10d box nails one each side of flange.
- . AJS® rim joist, rim board or AJS® blocking panel to support.
- Cd nails at 6 inches on center.
- When used for shear transfer, follow the building designer's specification.

AJS® Joist to support.

- 2-8d nails, one on each side of the web, placed 132 inches minimum from the end of the AJS® Joist to limit splitting
- Sheathing to AJS® joist, rim joist, blocking:
- Prescriptive residential foor sheathing nailing requires 8d common nails @ 6" o.c. on edges and @ 12" o c. in tKe fi∃ld iRC Table R602 3(1). Cioser nail spacin i may be required par design professional of record.
- 14 gauge staples may be substituted for 8d nails if the staples penetrate at least 1 inch into the joist.
- Wood screws may be acceptable, contact local building official and/or Boise Cascade EWP Engineering for further information.

BACKER AND FILLER BLOCK DIMENSIONS

AJS® Series	Backer Block Thickness	Filler Block Thickness
140 150 20 190	1½" or two ½" wood panels	2x_ + 5%" wood panel
25	2x_lumber	Double 2x_lumber

- Cut backer and filler blocks to a maximum depth equal to the web depth minus $^{1}\!4^{\rm m}$ to avoid a forced fit.
- For deeper AJS® Joists, stack 2x lumber or use multiple

WEB STIFFENER REQUIREMENTS

• See Vv-b Stiffener Requirements on page 4.

PROTECT AJS® JOISTS FROM THE WEATHER

 ACC® to sts is intended only for applications that provide permanent protect on from the weather. Bundles of product snown be covered and stored off of the ground on stickers

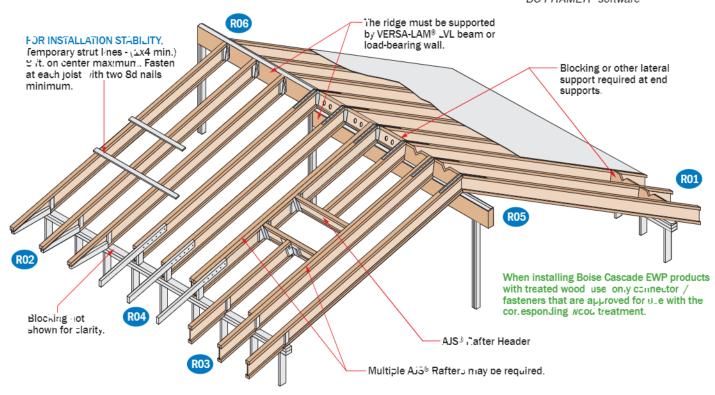
AJS® RIM JOISTS AND BLOCKING

Joist Depth	Vertical Load Transfer Capacity (plf)
9½"	1875
11%"	1680
14"	1500
16"	1340

1) Web stiffeners required at each end of blocking panel. Distance between stiffeners must be less than 24".

AJS® Rafters

Additional root traming details available with BC FRAMEH® software

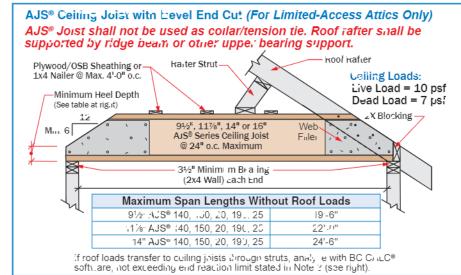


SAFETY WARNING

DO NOT ALLOW WORKERS ON AJS® JOISTS UNTIL ALL HANGERS, AJS® RIM JOISTS, RIM BOARDS, AJS® BLOCKING PANELS, X-BRACING AND TEMPORARY .x4 STRUT LINES ARE INSTALLED AS SPECIFIED BELOW.

SERIOUS ACCIDE:\TS CAN RESULT FROM INSUFFICIENT ATTENTION TO PROPER BRACING DURING CONSTRUCTION. ACCIDENTS CAN IE A/OIDED UNDER NORMAL CONDITIONS TRY FOLLOWING THESE GUIDELINES:

- Build a braced end wall at the end of the bay, or permanently install
 the first eight feet of AJ Joists and the first course of sheathing. As
 an alternate, temporary sheathing may be nailed to the first four feet of
 AvS® Joists at the end of the pay.
- Install temporary 1x4 strut lines at no more than eight feet on center as additional AJS[®] Joists are set. Nail the strut lines to the sheathed area, or braced end wall, and to each AJS[®] Joist with two 8d nails.
- The ends of cantilevers must be temporarily secured by strut lines on both the top and bottom flanges.
- Straighten the AJS® Joist to within ½ inch of true alignment before attaching strut lines and sheathing.
- Remove the temporary strut lines only as re-juired to install the permanent sheathing.
- Failure to install temporary bracing may result in sideways buckling or roll-over under light construction loads



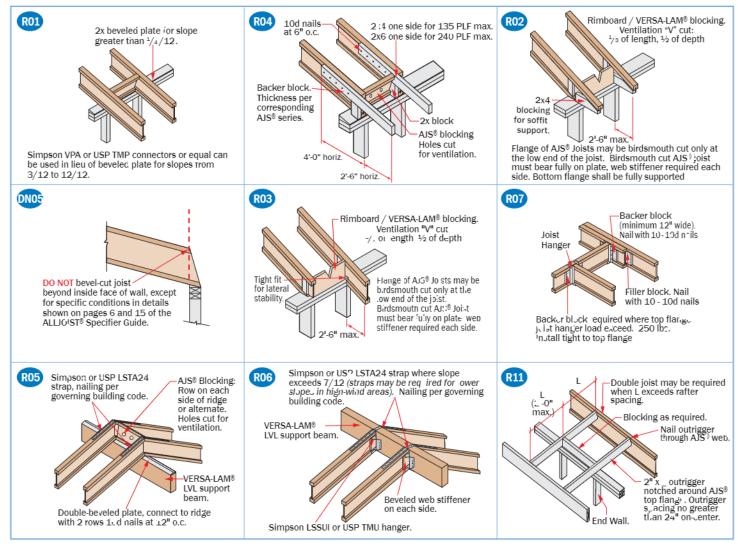
	Joist	End vVai:				
Minimum	Depth	2 x +	2 x .			
Heel	91/2"	21/2"	11/2"			
Depths	11%"	31/2"	21/2"			
	í4"	41/2"	31/2"			

Notes:

- Detail is to be used only for ceiling joists with no access to artic space.
- Ceiling joist must be designed to carry all roof load transferred through rafter struts as shown.
- AJS[®] ceiling joist end reaction may not exceed 550 pounds.
- 4) Minimum roof slope is 5/12.
- Nail roof rafter to AJS[®] top flange with 1-16d sinker or box nail.
- 6) 1x4 nails shall be continuous and nailed to an end wall braced to the roof diaphragm.
- 7) Install a 24" long web stiffener on each side of AJS® Joist at beveled ends. Nail roof rafter to AJS® Joist per building code requirements for ceiling joist to roof rafter connection.

Roof Framing Details

Additional roof framing details available with BC FRAMER® software



LATERAL SUPPORT

- AJS® Joists must be laterally supported at the ends with hangers. AJS® rim joists, rim boards, AJS® blockling pa; els or x-bracing. AJS® blockling panels or x bracing are required at cantile ensur ports
- Blocking may be required at intermediate bearings for loor diaphragm per IRC in high seismic areas, consult local building official.

MINIMUM BEARING LENGTH FOR AJS® JOIS. S

- 1½ inches is required at end supports. 3½ inches is required at cantilever and intermediate supports.
- Longer bearing lengths allow higher reaction values.
 Refer to the building code evaluation report or the BC CALC® software.

NAILING REQUIREMENTS

- AJS $^{\circ}$ rim joist, rim board or closure panel to AJS $^{\circ}$ Joist:
- Rims or closure panel 1¼ inches thick and less:
 2-8d nails, one each in the top and bottom flange.
- AJS® 140/150/20/190 rim joist: 2-16d Lox nails, one each in the top and bottom flange.
- AJS® 25 rim joist: Toe-nail top flange to rim joist with 2-10d box nails, one each side of flange
- AJS® rim joist, rim board or AJS® blocking panel to support:
 - 8d nails at 6 inches on center.
 - When used for shear transfer, follow the building designer's specification.

- AJS® Joist to support:
 - 2-8d nails, one on each side of the web, placed 1½ inches minimum from the end of the AJS® Joist to limit splitting.
- . Sheathing to AJS® joist, rim joist, blocking:
 - Prescriptive residential roof sheathing nailing requires 8d common nails @ 6" o.c. on edges and @ 12" o.c. in the field IRC Table R602.3(1). Closer nail spacing may be required per design professional of record.
 - 14 gauge staples may be substituted for 8d nails if the staples penetrate at least 1 inch into the joist.
 - Wood screws may be acceptable, contact local building official and/or Boise Cascade EWP Engineering for further information.

BACKER AND FILLER BLOCK DIMENSIONS

AJS® Series	Backer Block Thickness	Filler Block Thickness
140 150 20 190	11/8" or two 1/2" wood panels	2x _ + 5/8" wood panel
25	2 x _ lumber	Double 2 x _ lumber

- Cut backer and filler blocks to a maximum depth equal to the web depth minus 1/4" to avoid a forced fit.
- For deeper AJS[®] Joists, stack 2x lumber or use multiple pieces of ¾" wood panels.

WEB STIFFENER REQUIREMENTS

• See Web Stiffener Requirements on page 4.

PROTECT AJS® JOISTS FROM THE WEATHER

AJS® Joists are intended only for application, that
provide permanent proteintion from the weather.
 Bundles of AJS® Joists should be covered and stored
off of the ground on stickers.

MAXIMUM SLOPE

 Inless otherwise noted, all roof details are valid for slopes of 12 in 12 or less.

VENTILATION

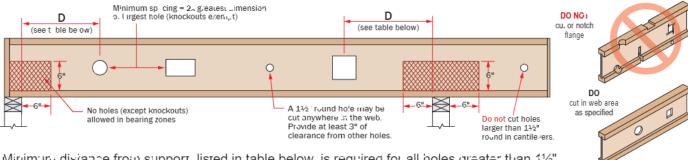
 The 1½ inch, pre-stamped knock-out holes spaced at 12 inches on center along the AJS® Joist may all be knocked out and used for cross ventilation.
 Deeper joists than what is structurally needed may be advantageous in ventilation design. Consult local building official and/or ventilation specialist for specific ventilation requirements.

BIRDSMOUTH CUTS

 AJS® Joists may be birdsmouth cut only at the low end support. AJS® Joists with birdsmouth cuts may cantilever up to 2'-6" past the low end support. The bottom flange must sit fully on the support and may not overhang the inside face of the support. High end supports and intermediate supports may not be birdsmouth cut.

AJS® Joist Hole Location & Sizing

AJS® Joists are manufactured with 11/2" round perforated knockouts in the web at approximately 12" on center



Mirin	nun i di	รเลา	ce fro	in sup	port,	listed	in tab	le bel	ow, is	requi	rea fo	ı all h	oles g	jreate:	r than	11/2"
		M	IINIMUN	/I DISTA	NCE (D) FROM	ANY S	UPPOR	T TO TH	IE CEN	TERLIN	E OF TH	HE HOL	E		
Round Ho	ole Diamet	erįi,	2	3	4	5	6	6½	1	8	81/8	9	10	11	12	.3
Kectangul	a: Hole Si	de [ın]	-	-	2	4	6	β		-	-	-	-	-	-	-
		8	2'-0"	2'-5"	2'-11"	3'-5"	3'-10"	4'-0"								
Any ¹ / ₂ " Joist	Sµan [ft]	12	3'-0"	3'-0"	4'-5"	5'-1"	5'-10"	6'-0"								
00151		13	4'-u"	4'11"	5'-11"	ช'–1ป"	7'-9"	8'-0"								
Round Ho	ole Diamet	er [in]	2	3	4	5	6	6½	7	8	87/s	9	10	11	12	13
Rectangul	ar Hole Si	de [in]	-	-	-	2	3	4	5	7	8	-	-	-	-	-
		3	1'- u"	a'-ט"	·¹'–10"	2'-3"	2'-8"	2'-11"	3'-]"	3'-6	3'-11					
Any	Span	12	1'-5"	2'-1"	2'-9"	3'-5"	4'-∪"	4'-4"	4'-8"	5'-4"	5'-11"					
Joist	[ft]	16	1'-11"	2'–1⊍"	J'-8"	4'-6"	5'-5	5'-1υ"	ũ'− კ"	, '-1	, '- 10"					
		٠.ህ	2'-5"	3'-6"	4'-7"	5'-8"	б'-9"	.′'–3"	7'-10	8'-11"	'- 10"					
Round Hole Diamete: [in]		er [in]	ے'	3	4	5	6	6½	7	3	8%	9	10	11	12	13
ReJanaul	ar Hole Si	de [in]	-	-	-	-	2	3	3	5	6	ô	8	9	-	-
		8	1'−∪"	·'-1"	1'-2	ì'-4"	1'-8"	1'-1i"	2'-1"	2' -6"	2' -10"	2':1"	3'-4"	3' "		
		12	1'–∪"	1'-1"	1'-4	2'-0	2'-7"	2'-:1"	3'-2"	3'–1∪"	4'-4"	4'-5"	5'-u"	5'-7"		
Any	Span [ft]	16	1'-じ"	1'-1"	1'–10"	2'-0"	"5-از	კ'–1ﻧ"	4'-3"	5'-1"	5'-9"	5'-1i"	6'-8"	7'-6"		
Joist		20	1'-u"	1'-3"	2'-4"	3'-4"	4'-4"	4'-10"	5'-4"	6'-4 ⁻	7'-3"	7'-4	8'-5"	J'-5"		
		24	1'- u"	1' -7"	2'-9"	4'-0"	5'-2"	5' ¹ 'υ"	6'-5"	7'-8 [:]	8'-8"	8'–1u"	::0'-1"	11'-3"		
Round Ho	ole Diamet	er [in]	2	3	4	5	6	6½	7	8	87/s	9	10	11	12	13
Rectangul	ar Hole Si	de [in]	-	-	-	-	-	-	2	3	5	5	6	8	9	16
		8	1'-0"	1'–1"	1'–2"	1'-2"	1'-3	1'-3"	1'-3	1'-8"	2'-0"	2'-1"	2'-5"	2'-1∪"	3'-2"	3'-7"
		12	1'-J"	1'-1"	1'-2"	1'-2"	1'-4	i'-8"	1'–11"	2'-6 ·	3'-0"	3'-1	3'-8"	4' -3"	4'–10"	5'-5"
Any 16	Span [ft]	16	1'-0"	1'-1"	1'-2"	1'-2"	1'-10"	2'-2"	2'-7	3'-4"	4'-u"	4'- 2"	4'-11"	ე'-ა"	6'-5"	7'-2"
Joist	1.4	20	1' -0"	1'-1"	1'-2"	1'-4"	2'-5"	2'-3"	3'-3"	4'- 3"	5'-:"	5'- 2"	მ'–2"	7'-1"	0'-i"	9'-0
		24	1'-υ"	1'-1"	1'-2"	1'-7"	2'-9"	3'-4"	3'-11"	5'-1"	6'-1"	6' -3"	7'-4"	8'-6	9'-8"	1บ'–1บ

- · Select a table row based on joist depth and the actual joist span rounded up to the nearest table span. Scan across the row to the col'irin headed by the appropriate round hole dia .eter or rectangular hole side. Use the longest side of a rectangular hole. The table value is the closest that the centerline of the hole may be to the centerline of the nearest support.
- The entire web may be cut out. DJ NO. cut the flanges. Holes apply to either single or multiple joists in repetitive member conditions.
- · For multiple holes, the amount of uncut web between holes must equal at least twice the diameter (or longest side) of the largest hole.
- 1½" round κnockouts in the web may be removed by using a short piece of metal pipe and hanımer.
- · Holes may be positioned vertically anywhere in the web. The joist may be set with the 11/2" knockout holes turned either up or down.
- · This table was designed to apply to the design conditions covered by tables elsewhere in this publication. Use the BC CALC® software to check other hole sizes or holes under other design conditions. It may be possible to exceed the limitations of this table by analyzing a specific application with the BC CALC[®] software.

Reinforced Load Bearing Cantilever Tables

AJS® Joists

두	SS	SS.	Roo! To.al Lo.u [psi]								
t Depth [in]	Series	Truss in [ft]		35			45			55	
Joist [[ir	ts ts	p ar			J	oist S	Spaci	ng [in			
9	Joist	Roof Tru Span [f	16	19.2	24	16	19 2	24	16	19.2	24
		?4	J	.0	1	U	0	λ	O.	Ά	Χ
		26	0	0	1	0	1	Χ	1	X	Х
		28	0	.U	Χ	0	1	Χ	1	Х	×
		30	0	0	Χ	0	Χ	Χ	Χ	×	Χ
	91/2"	32	Û	0	Χ	1	Χ	Χ	Χ	X-	Χ
	σ	34	0	1	Χ	1	Χ	Χ	Χ	Χ	X
		36	С	1	Χ	1	Χ	λ	Χ	Α	Χ
		38	0	,	Χ	Χ	Χ	Χ	Χ	Χ	X
		4∪	0	Χ	Χ	λ	Χ	Χ	Χ	X	Ä
		24	0	U	Э	С	Ú	υ	0	0	Χ
		26	0	0	U	U	υ	1	0	0	Χ
		28	0	0	U	Ü	U	- 1	0	1	Χ
	_	30	0	0	U	0	υ	١.	0	1	Χ
	, ·	32	0	0	Ü	D	U	Х	Ü	1	Χ
		34	0	0	1	0	U		U	X	Χ
		36	0	0	1	L	1	X.	1	X	Χ
E.		38	0	0	1	Û	1	Χ	1	Х	Χ
÷		40	0	0	Χ	U	1	\.	1	X	Χ
AJS* 1:		24	0	0	0	0	0	û	0	0	W
Ą		26	0	0	0	0	0	WS	0	0	W
		28	0	0	0	G	0	WS	0	0	1
		30	0	0	0	0	0	WS	0	0	1
	4	32	0	0	0	û	0	WS	0	wS	Χ
		3.4	0	0	0	0	0	1	0	WS	Χ
		36	0	.0	W	0	0	1	0	1	×
		3٤	0	0	W	0	0	1	0	1	Χ
		4.	0	0	W	G	WS	Χ	0	1	Χ
		24	0	v	U	Û	υ	C	0	0	.VS
		26	0	0	J	U	U	WS	0	0	.VS
		28	0	0	υ	0	0	WS	0	0	.VS
		3υ	0	0	U	0	U	ŃS	U	0	.VS
	16	32	0	0	IJ	L	U	Иъ	J	WS	1
		3.4	0	0	.VS	0	IJ	'nS	J	WS	1
		36	0	0	:\\3	U	U	WS	0	WS	1
		38	0	0	.VS	0	".V <i>S</i>	WS	0	WS	Χ
		4.	0	0	WS	0	WS	1	0	WS	Χ

두	S	SS			Ro	of To	tal Lo	ad [p	sf]							
Depth	Series	Truss in [ft]		35			45			55						
Joist [[ir	Joist S	Roof Tr Span			J	oist !	Spaci	ng [in	1]							
9	3	೭೪	16	19.2	24	16	19.2	24	16	19 2	24					
		24	0	0	1	0	0	2	0	1	Χ					
		2∂	0	0	1	0	1	Χ	1	2	Χ					
		28	0	0	1	0	1	Χ	1	Χ	Χ					
		30	0	0	2	0	1	Χ	1	Х	Χ					
	91/2"	32	0	0	2	1		Χ	2.	Х	Χ					
	σ	34	0	1	Χ	1	2	Χ	_	Χ	Χ					
		36	0	1	Χ	1	Χ	Χ	Χ	Х	Χ					
		38	0	1	Χ	1	Χ	Χ	Χ	Χ	Χ					
		40	0	1	Χ	2	Χ	Χ	Χ	Χ	Χ					
		24	0	0	0	0	0	0	0	0	1					
		23	0	0	0	0	0	1	0	0	1					
		28	0	0	0	0	0	1	0	1	A					
	_	30	0	0	0	0	0	1	0	1	Х					
	117/8"	32	0	0	0	0	0	1	0	1	Χ					
	÷	34	0	0	1	0	0	2	0	1	Х					
		36	0	0	1	0	1	Χ	1	2	А					
0		38	0	0	1	0	1	Χ	1	2	Χ					
AJS® 1		40	0	0	1	0	1	Χ	1	Х	ΔI					
လိ		24	0	0	0	0	0	0	0	0	WS					
A		26	0	0	0	0	0	WS	0	0	WS					
		28	0	0	0	0	0	WS	0	0	1					
		30	0	0	0	0	0	WS	0	WS	1					
	4	32	0	0	0	0	0	WS	0	WS	1					
	`	34	0	0	WS	0	0	1	0	WS	Χ					
		36	0	0	WS	0	0	1	0	1	Χ					
		38	0	0	WS	0	W_	1	0	1	Χ					
		40	0	0	WS	0	W	1	W	1	Χ					
		24	0	0	0	0	0	0	0	0	.MS					
		26	0	0	0	0	0	WS	0	0	'S					
		28	0	0	0	0	0	WS	0	0	.JV5					
		30	0	0	0	0	0	WS	0	WS	.jÿS					
	16"	32	0	0	0	0	0	WS	0	WS	1					
		34	0	0	^\S	0	0	WS	0	WS	1					
		36	0	0	¹ S	0	0	WS	0	WS	1					
		38	0	0	NS	0	JS	WS	0	WS	1					
		40	0	0	N٥	0	WS	1	WS	WS	Χ					

두	S	s	Roof Total Load [psf]									
Jep.	erie	Truss n [ft]		35			45	-		55		
Joist Depth [in]	Joist Series	Roof Tru Span [f			J	oist :	Spaci	ng [ˈn]			
우	9	ಜ್ಯ	16	19.2	24	16	19.2	24	16	19.2	24	
		24	0	0	1	0	0	2	0	2	Χ	
		26	0	0	1	0	1	Χ	1	2	Χ	
		28	0	0	1	0	1	Χ	1	2	Χ	
		30	0	0	2	0	5	Χ	1	Χ	Χ	
	91/2"	32	0	0	2	1	2	Χ	2	Χ	Χ	
	0)	34	0	1	2	1	_	Χ	2	Χ	Χ	
		36	0	1	Χ	1	Χ	Χ	_	Χ	Χ	
		3.	0	1	Χ	1	Χ	Χ	Χ	Χ	Χ	
		40	0	2	Χ	2	Χ	Χ	Χ	Χ	Χ	
		24	0	0	0	0	0	WS	0	0	1	
		26	0	0	0	0	0	1	0	0	2	
		28	0	0	0	0	0	1	0	1	^	
		30	0	0	0	0	0	1	0	1	X	
	117/4"	32	0	0	WS	0	0	2	0	1	Х	
	-	34	0	0	1	0	0	Χ	0	1	X	
		36	0	0	1	0	1	Χ	1	2	^	
0		38	0	0	1	0	1	Χ	1	2	Х	
0		40	0	0	1	0	1	Χ	1	Χ	^	
AJS® 20		24	0	0	0	0	0	0	0	0	WS	
⋖		26	0	0	0	0	0	WS	0	0	WS	
		28	0	0	0	0	0	WS	0	0	1	
	_	30	0	0	0	0	0	WS	0	WS	1	
	4	3	0	0	0	0	0	WS	0	WS	1	
		34	0	0	WS	0	0	1	0	WS	Χ	
		36	0	0	WS	0	WS	1	0	1	Χ	
		38	0	0	WS	0	WS	1	0	1	Χ	
		40	0	0	WS	0	WS	1	WS	1	Χ	
		24	0	0	0	0	0	0	0	0	WS	
		26	0	0	0	0	0	WS	0	0	WS	
		28	0	0	0	0	0	WS	0	0	WS	
	_	30	0	0	0	0	0	WS	0	WS	WS	
	16"	32	0	0	0	0	0	WS	0	WS	1	
		34	0	0	WS	0	0	WS	0	WS	1	
		36	0	0	WS	0	WS	WS	0	WS	1	
		38	0	0	WS	0	WS	WS	WS	WS	1	
		40	0	0	WS	0	WS	1	WS	WS	Χ	

t Depth [in]	Series	Truss n (+)			Ro	of To	tal Lo	ad [p	sf]		
-E	Seri	흔		35			45			55	
Joist	Joist (Roof S.;			J	loist S	Spaci	ng [ir	1]		
윽	윽	500	16	19.2	24	16	19.2	24	16	19.2	24
		24	0	0	1	0	0	2	0	2	Х
		26	0	0	1	0	1	Χ	1	2	Χ
		28	0	0	1	0	1	Χ	1	Χ	Χ
		30	0	0	2	0	2	Χ	1	Χ	Χ
	1,2/10:	32	0	0	2	1	2	Χ	2	Χ	Χ
	٠.	34	0	1	Χ	1	2	Χ	2	Χ	Χ
		36	0	1	Х	1	Х	Χ	Χ	Χ	Χ
		38	0	1	Χ	2	Χ	Χ	Χ	Χ	Χ
		40	0	2	Χ	2	Χ	Χ	Χ	Χ	Χ
		24	0	0	0	0	0	WS	0	0	1
		26	0	0	0	0	0	1	0	0	2
		28	0	0	0	0	0	1	0	1	Χ
		30	0	0	0	0	0	1	0	1	Χ
	117/8"	32	0	0	WS	0	0	2	0	1	Χ
	#	34	0	0	1	0	0	Χ	0	1	Χ
		36	0	0	1	0	1	Χ	1	2	Χ
ð		38	0	0	1	0	1	Χ	1	Χ	Χ
AJS" 190		40	0	0	1	0	1	Χ	1	Χ	Х
Š		24	0	0	0	0	0	0	0	0	WS
Ą		26	0	0	0	0	0	WS	0	0	WS
		28	0	0	0	0	0	WS	0	WS	1
		30	0	0	0	0	0	WS	0	WS	1
	<u>'</u>	32	0	0	0	0	0	WS	0	WS	1
		34	0	0	WS	0	0	1	0	WS	Χ
		36	0	0	WS	0	WS	1	0	1	Χ
		38	0	0	WS	0	WS	1	WS	1	Χ
		40	0	0	WS	0	WS	1	WS	1	Χ
		24	0	0	0	0	0	0	0	0	WS
		26	0	0	0	0	0	WS	0	0	WS
		28	0	0	0	0	0	WS	0	0	WS
		30	0	0	0	0	0	WS	0	WS	WS
	16"	32	0	0	0	0	0	WS	0	WS	1
		34	0	0	WS	0	0	WS	0	WS	1
		36	0	0	WS	0	WS	WS	0	WS	1
		38	0	0	WS	0	WS	WS	WS	WS	1
		40	_	^	1110	^	1410	4	1410	1410	37

Joist Depth [in]	es es	SS.	Roof Total Load [psf]								
Jep Jep	eri	흔든		35			45			55	
T:E	Joist Series	pa d			J	loist :	Spaci	ng [in	1]		
号	9	Roof Truss Span [ft]	16	19.2	24	16	19.2	24	16	19.2	24
		24	0	0	1	0	0	Χ	0	2	Χ
		26	0	0	1	0	1	Χ	1	Χ	Χ
		28	0	0	2	0	1	Χ	1	Χ	Х
	_	30	0	0	2	0	2	Χ	2	Χ	Х
	91/2"	32	0	0	Χ	1	2	Χ	2	Χ	Х
	0,	34	0	1	Χ	1	Χ	Χ	Χ	Χ	Х
		36	0	1	Χ	1	Χ	Χ	Χ	Χ	Χ
		38	0	2	Χ	2	Χ	Χ	Χ	Χ	Χ
		40	0	2	Χ	2	Χ	Χ	Χ	Χ	X 2
		24	0	0	0	0	0	0	0	0	2
		26	0	0	0	0	0	1	0	0	2
		28	0	0	0	0	0	1	0	1	X
	=	30	0	0	0	0	0	2	0	1	Χ
	117/8"	32	0	0	0	0	0	2	0	1	Χ
	-	34	0	0	1	0	0	2	0	2	Χ
		36	0	0	1	0	1	Χ	1	2	Χ
ις.		38	0	0	1	0	1	Χ	1	Χ	X
8		40	0	0	0	0	1	Х	1	Χ	Χ
AJS® 25		24	0	0		0	0	0	0	0	0
⋖		26	0	0	0	0	0	0	0	0	0
		28	0	0	0	0	0	0	0	0	1
	_	30	0	0	0	0	0	0	0	0	1
	4	32	0	0	0	0	0	0	0	0	2
		34	0	0	0	0	0	1	0	0	2
		36	0	0	0	0	0	1	0	1	2
		38	0	0	0	0	0	1	0	1	Χ
		40	0	0	0	0	0	2	0	1	Χ
		24	0	0	0	0	0	0	0	0	0
		26	0	0	0	0	0	0	0	0	0
		28	0	0	0	0	0	0	0	0	WS
	=	30	0	0	0	0	0	0	0	0	WS
	16"	32	0	0	0	0	0	0	0	0	1
		34	0	0	0	0	0	0	0	0	1
		36	0	0	0	0	0	WS	0	0	1
		38	0	0	0	0	0	WS	0	0	1
		40	0	0	0	0	0	1	0	WS	2

KEY TO TABLE

0.... No Reinforcement Required

WS.... Web Stiffeners at Support

1.... Web Stiffeners Plus One Reinforce

2.... Web Stiffeners Plus Two Reinforcers

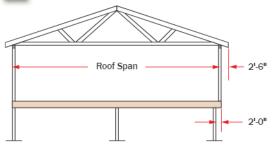
X.... Une Deeper Joists or Closer Spacing

Notes

- i. Cu. 48" iong reinfolders to match the joist depth. Use 1/4" APA Rister Sheathing, Exposure 1 1/24 Span Rating panels. The indee grain must be horizont 1 (measure the 1/8" dimension along the long edge of the panels.
- Fasten the reiniorcer to the joist flanges with °d ruils at ro. . When reinforcing both sides, stagger the nails to avoid spitting the joist flanges.
- Attach web stifleners per intermediate Web stiffener Nailing Schedule on page 4.
- Use the BC CALC[®] software to analyze conditions that are not covered by this table.

40 0 0 WS 0 WS 1 WS WS X

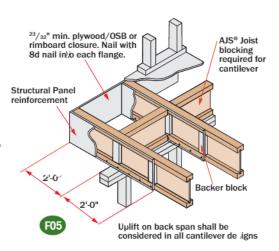
Reinforced Load Bearing Cantilever Detail



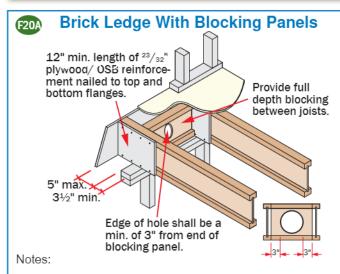
The tables and details on pages 9 and 10 indicate the
type of reinfurcements, if any, that are required for loadbear ing cantilevers up to a maximum length of alloff cantilevers longer than all-off cannot be reinforced.
 However longer cantilevers with lower loads
may be also wable without reinforcer.ent. A haryze
specific all plications with the BC CALC® software.

FLYWOOD / OSB REINFORCEMENT (If Required per Table on page 9)

- ²²/₂: Min. x 48" long plywood / OSB rated sheathing must match the full depth of the AJS® Joist. Nail to the AJS® Joist with of nails at 6" o.c. and nail with 4 dd nails into bacrer bloca. When reinforcing both sides stagger nails to limit splitting, install with horizontal face grain.
- These requirements assum a 100 PLF wall load and apply to AJS[®] Joists...dditional support may be required for other loadings. See BC JA_C[®] software.
- Contact Boise Cascade EWP Engineering for reinforcement requirements on AJS® Jo.st depths greater than 16".



Brick Ledge Load Bearing Cantilever



1. Usu 2"/12" min plywood/OSB raced sheathing initiall full depth of joist with face grain parallel to joist. Elywood reinforcement to bear fully on wall plate. Nail plywood to top

and buttom joist flan, es with 23

- (8d) nails at 3" or center except 91/2" joists install nails at 21/2" on center.
- Provide fuli deptn blocking between joists.
- 3. Edge of irole snall be at a minimum of 3" from end of blocking panel.

F20B Brick Ledge Without Blocking Panels
Eod bearing wall. Brick Wall. 2x10 sill plate for 2x6 wall, 2x8 sill plate for 2x4 wall. Load bearing wall.
Notes:

- Use ²³/₃₂" min µlywood/∪SB rated sneathing. Install full deptn of joist witn face grain parallel to joist. Plywood reinforcement to bear fully on wall plate. Nail plywood to top and botton, joist flanges with 2½"
- (&d) nails at 3" on center except 9% joists, install nails at 2% on center.
- See page 5 for joist and rimboard connection details.

	Roof					Roc	of Live	L₋ad (psf)				
Joist	Truss		20 pst			30 psf			40 psf			50 psf	
Depth	Span							cing o					
(inches)	(ft)	12	16"	19.2"	12"	16"	19.2"	12"	16"	19.2"	12 '	16"	9.2"
	24'	0	0	0	0	0		0	- 1		0	1	
	26'	0	0	0	0	0	- 1	0	1	-	0	1	2
	28'	0	0	0	0	0		0	1	-	0	1	2
21/2"	30'	0	0	0	0	0	1	0	1			1	2
	32'	0	0		0	1		0	1	2		2	2
	34'	0	0		0	X		0	1		1	2	Ξ.
	36'	0	¥	X	0	X	.i	4	Χ	. <u>X</u> .	:	Х	./.
	24'	0	0	0	0	0	0	0	0	0	0	0	-
	26'	0	0	0	0	0	0	0	0	0	0	0	
	28'	0	0	0	0	0	0	0	0		0	1	
117/8"	30'	0	0	0	0	0	0	0	0		0	1	
	32'	0	0	0	0	0	0	0	0		0	1	1
	34'	0	0	0	0	1		0			0		1
	36'	0	0	0	0	1		0	1	1	0	1	X
	24'	0	0	0	0	0	0	0	0	0	0	0	0
	26'	0	0	0	0	0	0	0	0	0	0	0	0
	28'	0	0	0	0	0	0	0	0	0	0	0	1
1411	30'	0	0	0	0	0	0	0	0	0	0	0	1
	32'	0	0	0	0	0	0	0	0	0	0	0	
	34'	0	0	0	0	0	0	0	0		0	0	
	36'	Λ	0	Λ	0	0	Λ	Λ	0		Λ	4	v

Brick Ledge Reinforcement Table

Table Design Assumptions

Roof Lo ding: 15 psf dead load plus a 100 PLF wall self-weight, in addition to roof live load shown. Maximum 2'-6" overhangs assumed on roof trusses.

F:cor Loading: 40 psf live load plus 10 psf dead load, backs, ans not to exceed maximum floor spans shown on page 3.

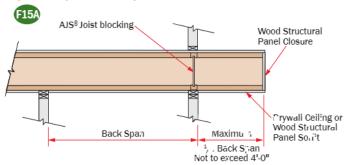
KEY TO TABLE:

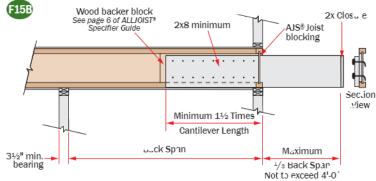
- 0 = No Reinforcement Required
- 1 = 'keinforcement Required One Side of Joist
- 2 = Reinforcement Requirea Both Sides of Joist
- Use Deeper Joists or Closer Spacing

Non-Load Bearing Wall Cantilever Details

AJS® Joists are intended only for applications that provide permanent protection from the weatner.

Fasten the 2x8 minimum to the AJS® Joist by nailing through the backer block and joist vieb with 2 rows of 10d nails at 6" on center. Clinch all nails.

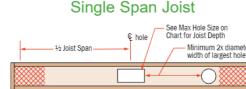




- These details apply to cantilevers with uniform loads only.
- It may be possible to exceed the limitations of these details by analyzing a specific application with the BC CALC® software.

Large Rectangular Holes in AJS® Joists

Hole size table based on maximum uniform load of 40 psr live load and 15 psf dead load, at maximum spacing of 24" on-center.



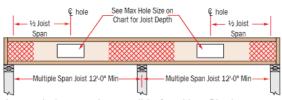
Notes:

Additional holes may but it in the web provided they meet the specifications as shown in the hole distance chalt shown above or as allowed using BC \cap AL \mathbb{P}^9 sizing software.

Simple Span Joist 6'-0" Mir

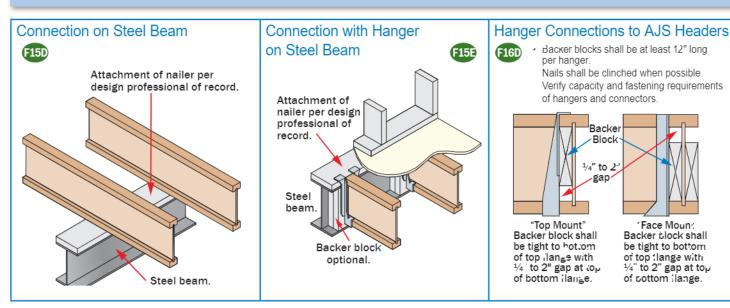
	Maximum Hole Size							
Joist Depth	Simple Span	Multiple Span						
91/2"	6" x 12"	6" 7"						
111%"	3" x 13"	8" *: 8"						
14"	9" x 16"	8" x 13"						
14	10" x 14"	9" :: 11"						
16"	11" x 16"	10" :: 14"						
10	12" x 15"	11" x 12"						



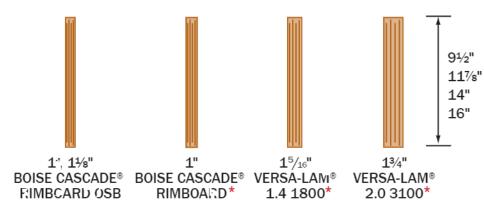


Larger holes may be possible for either Single or Multiple span joists; use BC CALC si ing software for specific analysis.

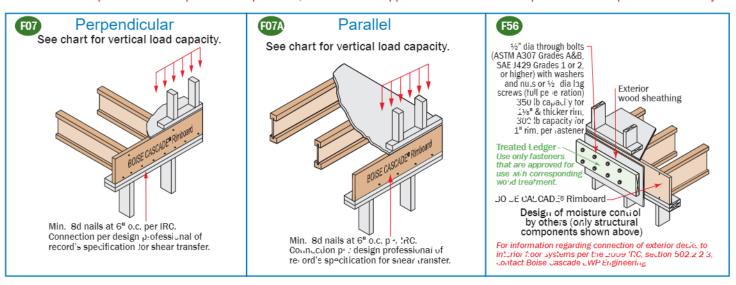
AJS® Joists — Connection Details



BOISE CASCADE® Rimboard Product Profiles



*18 – 24 inch deep rimboard are special order products, contact local supplier or Boise Cascade representative for product availability.



BOISE CASCADE® Rimboard Properties

		Ve	ertical Loa	ad Capac	ity			Specific		llowable.	Design Va	lues
Product	U	niform [p	lf]		Point [lb]			Gravity				
	16" Depth & Less	18" & 20" Depth & Less	22" & 24" Depth & sess	16" Depth & Less	18" & 20" Depth & Less	22" & 24" Depth & tess	Maximum Floor Diaphragm Lateral Capacity [lb/ft]	for Lateral Nail Design	Flexural Stress [lb/in²]	Modulus of Elasticity [lb/in²]	Horizontal Shear [lb/in²]	Compression Perpendicular to Grain [lb/in²]
1" B. JSE CASCADE® RIMBOARD (-) & 1" BUISE CASCADE® KI://BOARD OSB (2)	3300	1650	1650	2500	3500	J: 00	180	0.5	Limit	ed span ca	apabilities,	see note 2
11%" 30ISE CASCADE® RIM _P OARD OSE (2)	 400	3000	3000	3500	3500	3500	180	0.5	Limit	ed span ca	apabilities,	see note 2
15/ ₁₆ " VERSA-LAM [®] 1.4 1800 ⁽¹⁾	6000	5450	_	4450	4450	_	Permitted per building cod for all nominal 2 th thick framing blocked and unblocked diaphragms (+ nail spacing & greater)	0.5	1800	1 400,000	225	525
1¾" VERSA-LAM [®] 2.0 3100 ⁽¹⁾	5700	4300	_	4300	3900	_	Permitted per building code for all nominal 2* thick framing blocked and unblocked diaphragms (4" nail spacing & greater)	0.5	2800	2,000,000	285	750

		Closest Allo	wable Nail S	pacing - Narro	ow Face [in]	
Product	8d Box	8d Common	10d & 12d Box	16d Box	10d, 12d Common & 16d Sinker	16d Com lon
1" BOISE CASCADE ⁸ RIMBOARD ⁽²⁾	3	3	-	-	-	-
1" or 11/s" BOISE CASCADE® RIMBOARD OSB (2)	3	3	See	e note 2 for na	ailing informat	tion
15/18" VERSA-LAM® 1.4 1800 (1)	3	3	3	3	4	6
1¾" VERSA-LAM® 2.0 3100 (1)	2	3	3	3	4	6

Notes

- Per ICC ESR-1040.
- See Performance Rated Rim Boards, APA EWS #W345K for further p. odu:t information.
- Not all products and depths may be available, check with Boise Cascade representative for product availability.

AJS® Design Properties

								nd Rea	ction [lbs]		Interi	Reaction	[lbs]	
AJS®	Donath	\A/-:	Moment		14 . 406	Shear	1:⁄₂" Be	earing	3½" B	earing	3½" Be	earing	51/4" Be	earing
Joist Series	Depth [inches]	Weight [plf]	M, [ft-lbs]	El x 10 ⁶ [lb-in ²]	K x 10 ⁶ [lbs]	V [lbs]	No WS ⁽¹⁾	WS ⁽²⁾						
	91/2	2.2	2450	18.1	5.2	1160	950	1240	11.75	1480	2350	2450	2350	2450
AJS®	1111//8	2.5	3175	ა10	6.6	1490	953	1335	1215	1595	2390	2000	2390	2000
140	14	2.8	3825	457	7.8	1790	960	1420	1250	1700	2430	3130	2430	3130
	16	3.1	4435	623	9.0	2065	9/0	1500	1285	1800	2465	3435	2465	3435
	91/2	2.2	2820	194	5.2	1160	950	1240	1175	1480	2350	2450	2350	2450
AJS®	111//8	2.5	3650	331	6.6	1490	955	1335	1215	1595	2390	2800	2390	2800
150	14	2.8	4390	487	7.8	1790	960	1420	1250	1700	2430	3130	2430	3130
	16	3.1	5090	664	9.0	2065	970	1500	1285	1800	2465	3435	2465	3435
	91/2	2.5	3395	232	5.2	1160	950	1240	1175	1480	2350	2450	2350	2450
AJS®	1111//8	2.8	4400	394	6.6	1490	955	1335	1215	1595	2390	2800	2390	2800
20	14	3.0	5295	578	7.8	1790	960	1420	1250	1700	2430	3130	2430	3130
	16	3.3	6140	786	9.0	20ರ5	970	1500	1285	1800	2465	3435	2465	3435
	91/2	2.5	3895	244	5.2	1160	950	1240	1175	1480	2350	2450	2350	2450
AJS®	1111//8	2.8	5045	414	6.6	1490	955	1335	1215	1595	2390	2800	2390	2800
190	14	3.0	6070	608	7.8	1790	960	1420	1250	1700	2430	3130	2430	3130
	16	3.3	7040	327	9.0	2065	970	1500	1285	1800	2465	3435	2465	3435
	91/2	3.1	5370	322	5.3	1160	950	1240	1175	1480	2600	2850	2600	2850
AJS®	111//8	3.4	6960	545	67	1490	955	1335	1215	1595	2690	3190	2690	3190
25	14	3.7	8380	798	7.9	1790	960	1420	1250	1700	2,70	3500	2:70	3500
	16	3.9	9720	1082	9.1	2065	970	1500	1285	1800	2850	3800	2850	3800

NOTES:

- (1) No web stiffeners required.
- (2) Web stiffeners required.
- (3) Not applicable, web stiffeners required.
- ivioment, shear and reaction values based upon a load duration of 100% and may be adjusted for other load durations.
- Design values listed are applicable for Allowable Stress Design (ASD).
- No additional repetitive member increase allowed.

BUILDING CODE EVALUATION REPORT

- ICC ESR 1144 (IBC, IRC)

$$\Delta = \frac{5WI^4}{384 EI} + \frac{WI^2}{K}$$

 Δ deflection [in]

w = uniform load [lb/in]

, = clear span [in]

 Σ_i = benuing stiffness [lb-in²]

K = s ea. deformation coefficient [lb]

VERSA-LAM® Design Values

Grade	Width [in]	Depth [in]	Weight [lb/ft]	Allowable Shear [lb]	Allowable Moment [ft-lb]	Moment of Inertia [in ⁴]	Grade	Width [in]	Depth [in]	Weight [lb/ft]	Allowable Shear [lb]	Allowable Moment [ft-lb]	Moment of Inertia [in ⁴]	
40.5		31/2	1.5	998	776	5.4			5.4	8.0	5237	6830	63.3	
VERSA- STUD®	1½	51/2	2.4	1538	1821	20.8			5./2	8.4	5486	7457	72.8	
≥°.		71/4	3.2	2066	3069	47.6			7:/4	11.0	7232	12566	166.7	
		31/2	1.8	1164	1058	£.3			91/4	14.1	Э227	19908	346.3	
		5½	2.8	1829	2486	24.3			91/2	14.5	9476	20937	375.1	
		71/4	3.7	2411	4189	55.6								
		91/4	4.7	30.76	6636	115.4		51/4	111/4	17.1	11222	28814	622.9	
		91/2	4.8	3159	6979	125.0		11½	18.1	11845	31913	732.6		
	13/4	111/4	5.7	3741	9605	2Ω7.6	00	3100		14	21.3	13965	43552	1200.5
0		1111/8	6.0	3948	10638	244.2	25					16	24.4	15960
3100		14 7.1 4655 14517 460.2 %		18	27.4	17755	70011	2551.5						
0		16	8.1	5320	18682	597.3	VERSA-LAM®		20	30.4	19950	85428	3500.0	
© ⊘i		18	9.1	5985	23337	850.5	4							
≨		24	12.2	7980	40183	2012.0	4		24	36.5	23940	120549	6048.0	
j		51/2	5.6	3658	4971	48.5	82		91/4	16.6	12303	26544	461.7	
SS.		71/4	7.4	4821	8377	111.1			91/2	17.1	12635	27916	500.1	
VERSA-LAM®		91/4	9.4	6151	13272	230.8				111/4	20.2	14963	38419	830.6
>		91/2	9.ફ	6318	13958	250.1			111%	21.4	15794	42550	976.8	
	3½	111/4	11.4	7481	19210	415.3		7	14	25.2	18620	58069	1600.7	
	3/2	11∂s	12.1	7897	21275	488.4		,	16	20.2	21280	74728	2389.3	
		14	14.2	9310	29035	800.3								
		16	16.2	10640	37364	1194.7			18	32.4	23940	93348	3402.0	
		18	18.3	11970	46674	1701.0			20	36.0	26600	113904	4666.7	
		20	20.3	13300	56952	2333.3			24	43.2	31920	160732	8064.0	

VERSA-LAM® Design Properties

		Modulus of Elasticity	Bending	F!orizontal Sh∈ai	Tension Para:lel to Grain	Compression Parallel to Grain	Compression Perpendicular to Grain	Equivalent Specific Gravity for Fastener Design
Design Property	Grade	E(x 10 ⁶ psi) ^()	F _b (psi) ^{(2)(o)}	Ь, (psi) ⁽²⁾⁽¹⁾	F _t (psi) ⁽²⁾⁽⁵⁾	F _{c+} (psi) ⁽²⁾	F _c ⊥ (psi) ⁽¹⁾⁽⁶⁾	(SG)
VERSA-∟AM [®] Beams	2.0 3100	2.0	3100	285	2150	3000	750	0.5
√ERSA-LAM [®] Studs	1.7 2650	1.7	2650	285	1650	3000	750	0.5
VERSA-LAM® Columns	1.8 2750	1.8	2750	285	1825	3000	750	0.5

- 1. This value cannot be adjusted for load duration.
- 2. This value is based upon a load duration of 100% and may be adjusted for other load durations.
- Fiber stress bending value shall be multiplied by the depth factor, (12/d)¹¹⁹ where d = member depth [in].
- 4. Stress applied perpendicular to the gluelines.

- 5. Tension /alue shall be multiplied by a length factor, $(4/L)^{1/8}$ where L = member length [ft]. Use L = 4 for members less than four feet long.
- 6. Stress applied parallel to the gluelines.
- Design properties are limited to dry conditions of use where the maximum moisture content of the material will not exceed 16%.

Multiple Member Connectors

	Side-Loaded Applications												
			Maxim	um Unifor	m Side Lo	ad [plf]							
	Na	iled	½" D	ia. Through	Co.t ⁽¹⁾	5⁄8" D	ia. Throngai	Bolt"					
Number of Members	2 ruws 13d Sinkers @ 12" u c	3 rows 16d Ginkers (C 12" o.c.	2 rows @ 24" o.c. stagger ad	2 rows @ 12" o.c. stayger 'd	2 . uws @ 6" o.c. staggered	2 rows @ 24" o.c. s-aggered	2 rows @ 12" o.c. staggered	2 rcws @ 6" o.c. staggered					
		13/4" V	ERSA LAI	ท® (⊃epths	of 10" and	less)							
2	470	705	50 5	1010	2020	560	1120	2245					
3(2)	350	525	375	75ა	1515	420	840	1685					
4(3)	use bolt	schedule	335	670	1345	370	7 :5	1495					
			31/2"	VERSA-L	AM®								
2(3)	use bolt	schedule	855	1715	:1/.7	1125	2250	N/A					
		1	¾" VERSA	-LAM® (De	pths of 24")							
Number	Na	iled	1/2" D	ia. Through	Bolt(')	5⁄8" D	ia. Through	Bolt ⁽¹⁾					
Number	3 rows 16d	4 ro is 16d	3 rc we	3 rows @	3 rows &	100.00 (g)	3 rows @	3 rows @					

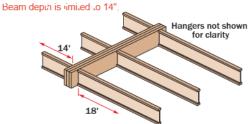
Number	Nailed		1/2" D	ia. Through I	Bolt ⁽¹⁾	%" Dia. Through Bolt()		
of Members	3 rows 16d Sinkers @ 12" o.c.	4 ro 's 16d انی Sinkers 2 o.c.	3 rcws & 21" o.c. 8" staggered	3 rows @ 18" o.c. 6" staggered	3 rows .බ 12" o.c. 4" siaggered	. ro s @ 2." o.c. 3" staggere	3 rows @ 18" o.c. 6" st iggerod	3 rows @ 12" o.c. 4" stagyered
2	705	940	755	1010	1515	840	1120	1685
3(2)	525	705	565	755	1135	630	840	1260
4(3)	use bolt schedule		505	670	1010	560	745	1120

- Design values apply to common bolts that conform to ANSI/ ACME standard B18.21-1981 (ASIM A307 Grades .88B, SE J429 Grades 1 or 2, or higher). A washer not less than a standard cut washer shall be between the wood and the bolt head and between the wood and the nut. The distance fror the edge of the bean to the bolt holes must be at least 2" for
- ,2" bolts and $21\!\!/\!\!2"$ for $5\!\!/\!\!6"$ bolts. Bolt holes shall be the same dia...eter as the bolt.
- The nail schedules shown apply to both sides of a 3-member heam.
- 7" wide beams :nust be top-loaded or loaded from both sides lesser side shall be no less than 25% of opposite side).

Designing Connections for Multiple VERSA-LAM® Members

When using multiple ply VER:SA-LAM® beams to create a wider member, the connection of the plies is as critical as determining the beam size. When side loaded beams are not connected properly, the inside plies do not support their share of the load and thus the load-carrying capacity of the full member decreases significantly. The following is an example of how to size and connect a multiple-ply VERSA-LAM® floor beam.

Give: Beam show below is supporting esidential floor load (40 psr live load, 10 psr lead load) and is a panning 16'-0".



Find: A multiple 1%" ply VETsSA LAM® that is ad lighted to support the design loads and the member's proper connection schedule.

- 1. Calculate the tributary width that beam is supporting:
 - 14' / 2 + 18' / 2 = 16'
- 2. Jije PLF tables on pages 30-32 of ASG or BC CALC® to
 - A Triple VERSA-LAM® 2.0 3100 1¾" x 14" is found to adequately support the design loads
- Calculate the maximum plf load from one side the right side in this case).
 - Max. Side Lead = $(18^{\circ} / 2) \times (40 + 10 \text{ ps}) = 40 \text{ plf}$
- Go to the Multiple Member Connection Table, Side-Loaded Applications, 194" VERSA-LAM®, 3 members
- The proper connection schedule must have a capacity greater than the max. side load:

Nailed: 3 rows 16d sinkers @ 12" o.c:

525 plf is greater than 450 plf OK

Bolts: 3/1 diameter 2 rows @ 12" staggered: 755 plf is greater than 450 plf OK

Top-Loaded Applications

For top-loaded beams and beams with side loads with less than those shown:

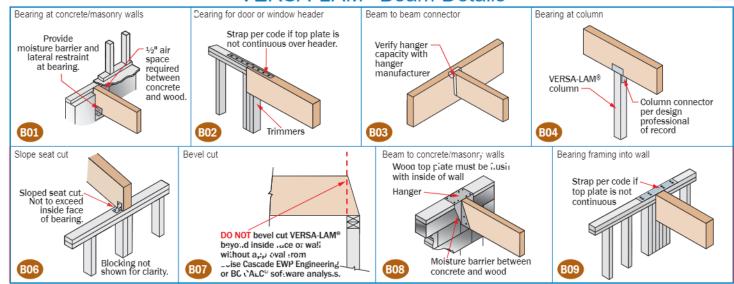
	For top-loaded beams and beams with side loads with less than those shown:				
	Plies	Depth	Nailing	Maximum Uniform Load From One Side	
Г		Deptris 11%" & less	2 mw. 16d box/sinker mails @ 12 lold.	4⊍0 p!f	
;	lieu 13⁄4" (2	Deptus 11" - 18:	з rows .6d box/sinker nails @ 12 o.c.	6δυ plf	
		Daptn = _4"	4 rows 16d box/sinkerils @ 12" o c.	800 p!f	
		Depths 11%" & less	2 rows 16d box/sinke, nails @ 12 o.c.	300 plf	
((c) 13/4" plies ~	Depths 14" 18"	ತ rows 16d box/sinkeri.'s @ 1∠್ o c.	4ن0 plf	
		Depth = 24"	4 rows 16d box/sinker nails @ 12" o.c.	600 p!f	
,	(4) 13/4" plies	Dupths 10" less	2 ows i/2" bɔʌs @ 24" o stagye:ed	ქან plf	
(⊔ер∴ ∷ 24	3 ເວພະ ./2 ' bo ts @ 24" າ.c. ະໝgge.ed ever, 8"	05 plf	
,	2) 21/" plice	Depths 18" & less	2 rows 1/2" bolts @ 24" o, staygered	855 plf	
((2) 3½" plies	Depth 20" - 24"	3 rows 1/2" bolts @ 24" c.c., staggered every 8"	1285 pl	

- 1. Beams wider than 7" must be designed by the engineer of record.
 2. AL values in these tables may be increased by £% for snow-load rous and by £5% or non snow load rous where the building.
 3. AL values in these tables may be increased by £% for snow-load rous and by £5%.
- 3. Use allowuble load tables or BC UALU™ software to size beams.

 An equitaient specific growty or 0.5 may ke used when destaining
- s_Peci.ic connections with VERUA-LAw™ 5. Connection values are based upon the 20ub NUS.
- 6. FastenMister Trus_Lot, "Imp.on Strong-fie SDS, and USP WS, crow may also use used to connect multiple menues VERSA-LAM learns either it Boise Cassade EWP Engin, either for further info.metion

VERSA-LAM® Beam Details/Allowable Nailing/Holes

VERSA-LAM® Beam Details



VERSA-LAM® Installation Notes

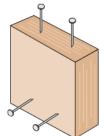
- Minimum of ½" air space between beam and wall pocket or a Jequate barrier must be provided between beam and concrete/masonry.
- Ad:quate bearing shall be provided. If not shown on plans, please refer to load tables in your region's Specifier Guide.
- VERSA-LAM[®] beams are intended for interior applications only and should be kept as dry
 as possible during construction
- Continuous lateral support of top of beam shall be pro.ided (side or top bearing framing).

Closest Allowable Nail Spacing

VERSA-LAM® & VERSA-RIM® Products									
	Nailing Parallel to Glue Lines (Narrow Face) ⁽¹⁾							Nailing Perpendicular to Glue Lines (Wide Face)	
Nail Size	VERSA-LAM ⁹ 1.4 1800 Rimboard 1 ⁵ / ₁₆ "		VERSA-LAM® 1¾"		VERSA-LAM® 3½" & Wider		All Products		
	O.J. [inches]	End [inches]	O.o. [inches]	End [inches]	O.C. [inches]	End [inches]	O.C. [inches]	End [inches]	
8d Frox	3	11/2	2	1	2	1/2	2	1/2	
3J Co mon	3	2	3	2	2	1	2	1	
10d & 12d Box	3	2	3	2	2	1	2	1	
16d ∂ox	3	2	3	2	2	1	2	1	
10d ୟ 12d Common	4	3	4	3	2	2	2	2	
16d Sinker	4	3	4	3	2	2	2	2	
16d Common	6	4	6	3	2	2	2	2	

- Offset and stagger nail rows from floor sheathing and wall sole plate.
- Simpson Strong-Tie A35 and LP 14 connectors may be attached to the side VERSA-LAM®/VERSA-RIM®.
 Use nails as specified by Simpson Strong-Tie.

Nailing Parallel to Glue Lines (Narrow Face)



Nailing Ferpendicular to Glue Lines (Wide Face)

Nailing Notes

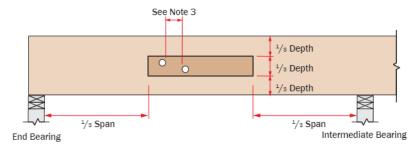
For 3/4" thickness and greater, 2 rows or halls (such as no, a metal strap) are anowed (use ½" minimum onset between rows and stagger nails).

Allowable Holes in VERSA-LAM® Beams

Notes

- 1. Square and rectangular holes are not permitted
- Round holes may be drilled or cut with a hole saw anywhere within the shaded area of the beam.
- The horizontal distance between adjacent holes must be at least two times the size of the larger hole.
- Do not drill more than three access holes in any four foot long section of beam.
- 5. The maximum round hole diameter permitted is:

Beam Depth	Max. Hole Diameter				
5 ¹ /2"	3/4"				
71/4"	1"				
91/4" and greater	2"				



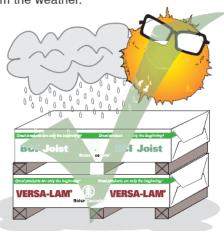
- These limitations apply to holes drilled for plumbing or wiring access only. The size and location of holes drilled for fasteners are governed by the provisions of the National Design Specification® for W od Construction.
- Beams deflect under load. Size holes to provide clearance where required.
- This hole chart is valid for beams supporting uniform load only. For beams supporting concentrated loads or for beams with larger holes, contact Boise Cascade EWP Engineering.

Handling and Storage of Engineered Wood Products

Site Storage

Site Handling

Protect AJS® Joists and VERSA-LAM® from the weather.



Keep at least 3½" off the ground, more in wetter areas. Align stickers one above another and space a maximum of 15 feet apart.



Unload from truck carefully using appropriate equipment.



DO NOT lift AJS® Joists by top flange.



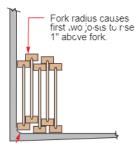
Do not d.op AJS® Joists from height.



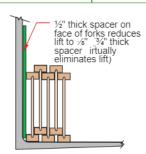
Avoid lifting AJS® Joists horizontally.

Leave AJS® Joists banded together until ready to install.





Frange crushing or breakage of outer AJJ® Joist may occur at fork radius.



This damage can be prevented by making sure that all joicts are sitting on the flat portion of the forks. This can be accomplished by making sure that the forks are not fully elitended into the unit. A spacer may be required on the front face of the forks in order to assure that the joists sit on the flat portion of the firms and do not dome in contact with the radius area of the forks. This procedure will equalize the pressure on the flanges of all joists.

AJS® Fork Radius Crushing

BCI® Joists, VERSA-LAM® and ALLJOIST® must be stored, installed and used in apprordance with the Boise Cascade EWP Installation Guide, building codes, and to the extent not inconsistent with the Boise Cascade EWP Installation Guide, usual and customary building practices and standards. VERSA-LAM®, ALLJOIST®, and ECI® Joists must be wrapped, covered, and stored out of the ground on stickers at all times prior to installation. VERSA-LAM®, ALLJOIST® and BCI® Joists are intended only for applications that assure no exposure to weather or the elements and an environment that is free from moisture from any source, or any pest, organism or substance which degrades or damages wood or glue bonds. Failure to correctly store, use or install VERSA-LAM®, ALLJOIST®, and BCI® Joist in accordance with the Boise Cascade EWP Installation Guide will void the limited warranty.

If in doubt ask! For the closest Boise Cascade EWP distributor/support center, call

BOISE CASCADE TREE-IN-A-CIRCLE, BCL BC CALC, BC CC. UiviN, BC FRAMER, BC RIM BOARD, BCISE CLULA.//I, SIMPLE FRAMING SYSTEM. VERSA-LAM, VERSA-STRAND, and VERSA-STUD are tracemarks or Boise Cascade Company or its affiliates.

The information in this document pertains to use in the UNITMD STATES ONLY, Allowable Stress Design Refer to the ALLoOiST Specifier Guide Carrada for use in Canada, Limit States Design.

Your Dealer is:

For information about Boise Cascade Engineered Wood Products,

in luding sales terms and conditions, warranties and disclaimers,

visit our website at www BCewp.ccm



Great products are only the beginning.®

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If no dealer is listed, call 1-800-232-0788