Product Submittal/Substitution Request

TO:

PROJECT:

PROJECT LOCATION:

SPECIFIED ITEM:

Section | Page | Paragraph | Description

PRODUCT SUBMIT TAL / SUBSTITUTION REQUESTED:

DEWALT® Engineered By Powers® PE1000+(R) -

The attached submittal package includes the product description, specifications, drawings, and performance data for use in the evaluation of the request.

SUBMITTED BY:

Name: Signature:

Company:

Address:

Date: Telephone: Fax:

FOR USE BY THE ARCHITECT AND/OR ENGINEER

☐ **Approved** ☐ **Approved as Noted** ☐ **Not Approved**

(If not approved, please briefly explain why the product was not accepted.)

By: Date:

Remarks:

Questions or inquiries? Contact us at engineering@powers.com, or call 1.800.524.3244

DEWALT® PE1000+(R) Submittal Section:

Product Pages:

- General Information
- Design Tables
- Installation Instructions
- Ordering Information

Code Reports & Agency Listings:

- ICC–ES Approval: ESR–2583 (Cracked & Uncracked Concrete)
- Potable Drinking Water Certification From NSF International: NSF/ANSI–61

GENERAL INFORMATION

PE1000+®

Epoxy Injection Adhesive Anchoring System

PRODUCT DESCRIPTION

The PE1000+ is a two-component, high strength adhesive anchoring system. The system includes injection adhesive in plastic cartridges, mixing nozzles, dispensing tools and hole cleaning equipment. The PE1000+ is designed for bonding threaded rod and reinforcing bar hardware into drilled holes in concrete and solid masonry base materials.

GENERAL APPLICATIONS AND USES

- Bonding threaded rod and reinforcing bar into hardened concrete and grouted masonry units
- Evaluated for use in dry and water-saturated concrete (including water-filled holes)
- Cracked and uncracked concrete
- Seismic and wind loading
- Hammer-drill and diamond core drilled hole
- Oversized hammer-drilled holes in concrete, for short term loading only (contact DEWALT for details)
- Can be installed in a wide range of base material temperatures

FEATURES AND BENEFITS

+ Designed for use with threaded rod and reinforcing bar hardware elements
+ Consistent performance in low and high strength concrete (2,500 to 8,500 psi)
+ Evaluated and recognized for freeze/thaw performance
+ Evaluated and recognized for long term and short term loading (see performance tables for applicable temperature ranges)
+ Evaluated and recognized for variable embedments (see installation specifications)
+ Cartridge design allows for multiple uses using extra mixing nozzles
+ Mixing nozzles proportion adhesive and provide simple delivery method into drilled holes
+ Easy dispensing reduces applicator fatigue

APPROVALS AND LISTINGS

- International Code Council, Evaluation Service (ICC-ES) ESR-2583
- Code compliant with the 2018 IBC/IRC, 2015 IBC/IRC, 2012 IBC/IRC, and 2009 IBC/IRC
- Tested in accordance with ACI 355.4 and AC308 for use in structural concrete according to (Strength Design) ACI 318-14 Chapter 17 and ACI 318-11/08 Appendix D.
- Evaluated and qualified by an accredited independent testing laboratory for recognition in cracked and uncracked concrete including seismic and wind loading
- Compliant with NSF/ANSI Standard 61 for drinking water system components – health effects; minimum requirements for materials in contact with potable waterand water treatment
- Conforms to requirements of ASTM C 881 and AASHTO M235, Types I, II, IV and V, Grade 3, Classes B & C (also meets type III except for elongation)
- Department of Transportation listings – see www.DEWALT.com or contact transportation agency

GUIDE SPECIFICATIONS

CSI Divisions: 03 16 00 - Concrete Anchors, 04 05 19.16 - Masonry Anchors and 05 05 19 - Post-Installed Concrete Anchors. Adhesive anchoring system shall be PE1000+ as supplied by DEWALT, Towson, MD. Anchors shall be installed in accordance with published instructions and requirements of the Authority Having Jurisdiction.

SECTION CONTENTS

PACKAGING

Dual (side-by-side) Cartridge

- 13 fl. oz. (385 ml), 3:1 mix ratio
- 19.5 fl. oz. (585ml), 3:1 mix ratio

STORAGE LIFE & CONDITIONS

Two years in a dry, dark environment with temperature ranging from 41ºF to 95ºF (5ºC to 35ºC)

ANCHOR SIZE RANGE (TYPICAL)

- 3/8" to 1-1/4" diameter threaded rod
- No. 3 to No. 10 reinforcing bar (rebar)

SUITABLE BASE MATERIALS

- Normal-weight concrete
- Lightweight concrete
- Grouted concrete masonry

PERMISSIBLE INSTALLATION CONDITIONS (ADHESIVE)

- Dry concrete
- Water-saturated concrete (wet)
- Water-filled holes (flooded)

TENSION ZONE CRACKED CONCRETE
SEISMIC REGION QUALIFICATION
CODE LISTED ICC-ES ESR-2583 CONCRETE
DDA DEWALT DESIGN ASSIST

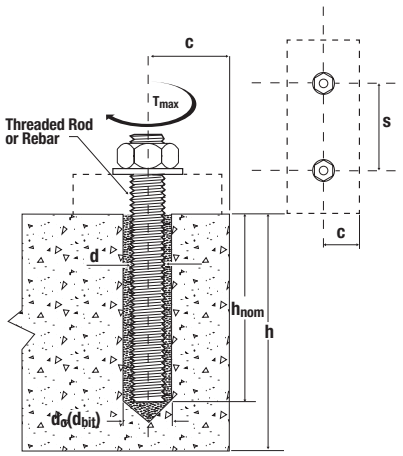
REFERENCE DATA (ASD)

Installation Table for PE1000+ (Solid Concrete Base Materials)

Dimension/Property	Notation	Units	Nominal Anchor Size									
Threaded Rod	-	-	**3/8"**	**1/2"**	**-**	**5/8"**	**3/4"**	**7/8"**	**1"**	**-**	**1-1/4"**	**-**
Reinforcing Bar	-	-	**#3**	**-**	**#4**	**#5**	**#6**	**#7**	**#8**	**#9**	**-**	**#10**
Nominal anchor diameter	d	in. (mm)	0.375 (9.5)	0.500 (12.7)		0.625 (15.9)	0.750 (19.1)	0.875 (22.2)	1.000 (25.4)	1.125 (28.6)	1.250 (31.8)	1.250 (31.8)
Carbide drill bit nominal size	d_o [d_{bit}]	in.	7/16 ANSI	9/16 ANSI	5/8 ANSI	11/16 or 3/4 ANSI	7/8 ANSI	1 ANSI	1-1/8 ANSI	1-3/8 ANSI	1-3/8 ANSI	1-1/2 ANSI
Diamond core bit nominal size	d_o [d_{bit}]	in.	-	5/8		3/4	7/8	1	1-1/8	-	-	-
Minimum nominal embedment	h_{nom}	in. (mm)	2-3/8 (61)	2-3/4 (70)		3-1/8 (79)	3-1/2 (89)	3-1/2 (89)	4 (102)	4-1/2 (114)	5 (127)	5 (127)
Minimum spacing distance	s_{min}	in. (mm)	1-7/8 (48)	2-1/2 (62)		3-1/8 (80)	3-3/4 (95)	4-3/8 (111)	5 (127)	5-5/8 (143)	6-1/4 (159)	6-1/4 (159)
Minimum edge distance	c_{min}	in (mm)	5d where d is nominal outside diameter of the anchor									
Minimum edge distance, reduced[4]	$c_{min,red}$	in (mm)	1-3/4 (44)	1-3/4 (44)	1-3/4 (44)	1-3/4 (44)	1-3/4 (44)	1-3/4 (44)	1-3/4 (44)	2-3/4 (70)	2-3/4 (70)	2-3/4 (70)
Max. Torque[1]	T_{max}	ft-lbs	15	33		60	105	125	165	200	280	280
Max. Torque[1,2] (A36/Grade 36 rod)	T_{max}	ft-lbs	10	25		50	90	125	165	N/A	280	N/A
Max. Torque[1,3] (Class 1 SS rod)	T_{max}	ft-lbs	5	20		40	60	100	165	N/A	280	N/A
Effective cross sectional area of threaded rod	A_{se}	in.² (mm²)	0.078 (50)	0.142 (92)		0.226 (146)	0.335 (216)	0.462 (298)	0.606 (391)	-	0.969 (625)	-
Effective cross sectional area of reinforcing bar	A_{se}	in.² (mm²)	0.110 (71)	0.200 (129)		0.310 (200)	0.440 (284)	0.600 (387)	0.790 (510)	1.000 (645)	-	1.270 (819)

1. Torque may not be applied until the full cure time of the adhesive has been achieved.
2. Applies to ASTM A36/F 1554 Grade 36 carbon steel threaded rods only.
3. These torque values apply to ASTM A193 Grade B8/B8m (Class 1) stainless steel threaded rods only.
4. For installations at the reduced minimum edge, $c_{min,red}$, the max torque, T_{max}, must be multiplied by a reduction factor of 0.45.

Detail of Steel Hardware Elements used with Injection Adhesive System



Steel Description (General)	Steel Specification (ASTM)	Nominal Anchor Size (inch)	Minimum Yield Strength fy (ksi)	Minimum Ultimate Strength fu (ksi)
Carbon Rod	A 36 or F 1554, Grade 36	3/8 through 1-1/4	36.0	58.0
	F 1554 Grade 55		55.0	75.0
	A 193, Grade B7 or F 1554, Grade 105		105.0	125.0
Stainless Rod (Alloy 304 / 316)	F 593 Condition CW	3/8 through 5/8	65.0	100.0
		3/4 through 1-1/4	45.0	85.0
Grade 60 Reinforcing Bar	A 615, or A 767, A 996	3/8 through 1-1/4 (#3 through #10)	60.0	90.0
Grade 40 Reinforcing Bar	A 615	3/8 through 3/4 (#3 through #6)	40.0	60.0

Nomenclature

d = Diameter of anchor
d_{bit} = Diameter of drilled hole
h = Base material thickness
The minimum value of h should be $1.5h_{nom}$ or 3", whichever is greater.
h_{nom} = Minimum embedment depth

Allowable Load Capacities for PE1000+ Installed into Uncracked Normal-Weight Concrete with Threaded Rod and Reinforcing Bar (Based on Bond Strength/Concrete Capacity)[1,2,3,4,5,6]

Nominal Rod/Rebar Size (in. or #)	Minimum Embedment Depth (in.)	Minimum Concrete Compressive Strength, (f'_c)			
		3,000 psi	4,000 psi	5,000 psi	6,000 psi
		Tension (lbs)			
3/8 or #3	2-3/8	1,195	1,235	1,270	1,300
	3-1/2	1,760	1,825	1,875	1,915
	4-1/2	2,265	2,345	2,410	2,460
1/2 or #4	2-3/4	1,770	1,835	1,885	1,925
	4-3/8	2,820	2,915	2,995	3,065
	6	3,865	4,000	4,110	4,200
5/8 or #5	3-1/8	2,420	2,505	2,575	2,630
	5-1/4	4,145	4,290	4,405	4,505
	7-1/2	5,970	6,180	6,345	6,485
3/4 or #6	3-1/2	2,870	2,970	3,050	3,120
	6-1/4	5,715	5,915	6,075	6,210
	9	8,560	8,860	9,100	9,300
7/8 or #7	3-1/2	2,870	2,970	3,050	3,120
	7	7,285	7,540	7,745	7,915
	10-1/2	11,700	12,110	12,440	12,715
1 or #8	4	3,505	3,630	3,725	3,810
	8	9,570	9,905	10,175	10,400
	12	15,635	16,185	16,625	16,990
1-1/8 or #9	4-1/2	4,185	4,330	4,445	4,545
	9	12,025	12,445	12,785	13,065
	13-1/2	19,865	20,560	21,120	21,585
1-1/4 or #10	5	4,900	5,070	5,210	5,325
	10	15,030	15,560	15,980	16,335
	15	25,165	26,045	26,755	27,345

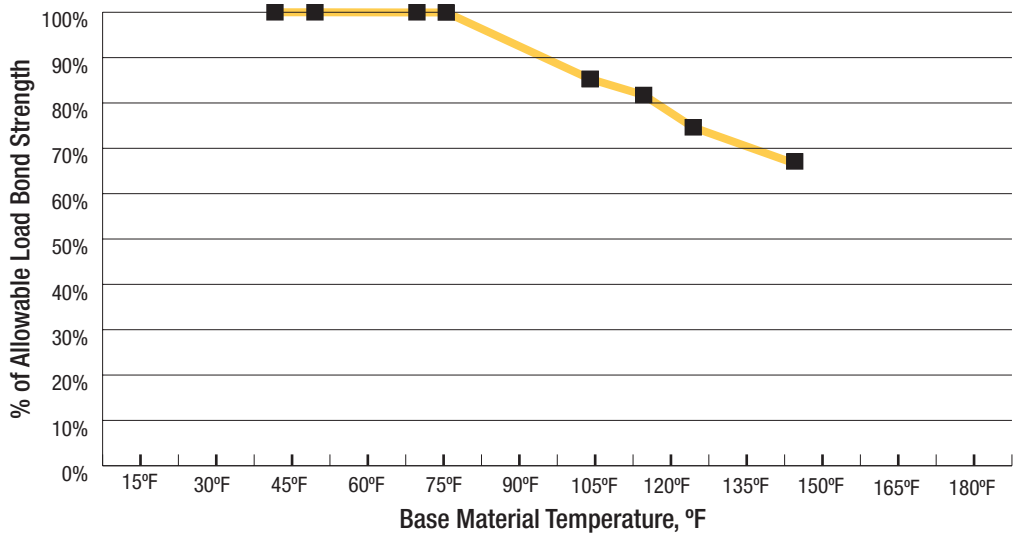
1. Allowable load capacities listed are calculated using an applied safety factor of 4.0. Consideration of safety factors of 10 or higher may be necessary depending on the application, such as life safety or overhead.
2. Linear interpolation may be used to determine allowable loads for intermediate embedments and compressive strengths.
3. The tabulated load values are applicable to single anchors installed at critical edge and spacing distances and at the minimum member thickness.
4. The tabulated load values are for applicable for dry concrete. Holes must be drilled with a hammer drill and an ANSI carbide drill bit. Installations into saturated (wet) concrete and in water-filled holes require a reduction in capacity for tabulated values of 30 percent and 50 percent, respectively. Contact DEWALT for more information concerning these installation conditions.
5. Adhesives experience reductions in capacity at elevated temperatures. See the in-service temperature chart for allowable load capacity reduction factors.
6. Allowable bond strength/concrete capacity must be checked against allowable steel strength in tension to determine the controlling allowable load. Allowable shear capacity is controlled by steel strength for the given conditions.

Allowable Load Capacities for Threaded Rod and Reinforcing Bar (Based on Steel Strength)[1,2,3,4,5]

Nominal Rod Diameter or Rebar Size (in. or #)	Steel Elements - Threaded Rod and Reinforcing Bar																	
	A36 or F1554, Grade 36		A36 or F1554, Grade 55		A 193, Grade B7 or F1554, Grade 105		F 593, CW (SS)		ASTM A615 Grade 40 Rebar		ASTM A615 Grade 60 Rebar		ASTM A706 Grade 60 Rebar		ASTM A615 Grade 75 Rebar		ASTM A706 Grade 80 Rebar	
	Tension lbs. (kN)	Shear lbs (kN)	Tension lbs. (kN)	Shear lbs (kN)	Tension lbs. (kN)	Shear lbs (kN)	Tension lbs. (kN)	Shear lbs (kN)	Tension lbs. (kN)	Shear lbs (kN)	Tension lbs. (kN)	Shear lbs (kN)	Tension lbs. (kN)	Shear lbs (kN)	Tension lbs. (kN)	Shear lbs (kN)	Tension lbs. (kN)	Shear lbs (kN)
3/8 or #3	2,115 (9.4)	1,090 (4.8)	2,735 (12.2)	1,410 (6.3)	4,555 (20.3)	2,345 (10.4)	3,645 (16.2)	1,880 (8.4)	2,210 (9.8)	1,125 (5.0)	2,650 (11.8)	1,690 (7.5)	2,650 (11.8)	1,500 (6.7)	2,650 (11.8)	1,875 (8.3)	2,650 (11.8)	1,875 (8.3)
1/2 or #4	3,760 (16.7)	1,935 (8.6)	4,860 (21.6)	2,505 (11.1)	8,100 (36.0)	4,170 (18.5)	6,480 (28.8)	3,340 (14.9)	3,925 (17.5)	2,005 (8.9)	4,710 (21.0)	3,005 (13.4)	4,710 (21.0)	2,670 (11.9)	4,710 (21.0)	3,335 (14.8)	4,710 (21.0)	3,335 (14.8)
5/8 or #5	5,870 (26.1)	3,025 (13.5)	7,595 (33.8)	3,910 (17.4)	12,655 (56.3)	6,520 (29.0)	10,125 (45.0)	5,215 (23.2)	6,135 (27.3)	3,130 (13.9)	7,365 (32.8)	4,695 (20.9)	7,365 (32.8)	4,170 (18.5)	7,365 (32.8)	5,215 (23.2)	7,365 (32.8)	5,215 (23.2)
3/4 or #6	8,455 (37.6)	4,355 (19.4)	10,935 (48.6)	5,635 (25.1)	18,225 (81.1)	9,390 (41.8)	12,390 (55.1)	6,385 (28.4)	8,835 (39.3)	4,505 (20.0)	10,605 (47.2)	6,760 (30.1)	10,605 (47.2)	6,010 (26.7)	10,605 (47.2)	7,510 (33.4)	10,605 (47.2)	7,510 (33.4)
7/8 or #7	11,510 (51.2)	5,930 (26.4)	14,885 (66.2)	7,665 (34.1)	24,805 (110.3)	12,780 (56.8)	16,865 (75.0)	8,690 (38.7)	-	-	14,430 (64.2)	9,200 (40.9)	14,430 (64.2)	8,180 (36.4)	14,430 (64.2)	10,220 (45.5)	14,430 (64.2)	10,220 (45.5)
1 or #8	15,035 (66.9)	7,745 (34.5)	19,440 (86.5)	10,015 (44.5)	32,400 (144.1)	16,690 (74.2)	22,030 (98.0)	11,350 (50.5)	-	-	18,850 (83.8)	12,015 (53.4)	18,850 (83.8)	10,680 (47.5)	18,850 (83.8)	13,350 (59.4)	18,850 (83.8)	13,350 (59.4)
#9	-	-	-	-	-	-	-	-	-	-	23,985 (106.7)	15,290 (68.0)	23,985 (106.7)	13,590 (60.5)	23,985 (106.7)	16,990 (75.6)	23,985 (106.7)	16,990 (75.6)
1-1/4	23,490 (104.5)	12,100 (53.8)	30,375 (135.1)	15,645 (69.6)	50,620 (225.2)	26,080 (116.0)	34,425 (153.1)	17,735 (78.9)	-	-	-	-	-	-	-	-	-	-
#10	-	-	-	-	-	-	-	-	-	-	30,405 (135.2)	19,380 (86.2)	30,405 (135.2)	17,230 (76.6)	30,405 (135.2)	21,535 (95.8)	30,405 (135.2)	21,535 (95.8)

1. AISC defined steel strength (ASD) for threaded rod: Tensile = 0.33 • F_u • A_{nom}, Shear = 0.17 • F_u • A_{nom}
2. For reinforcing bars: The allowable steel tensile strength is based on 20 ksi for Grade 40 and 24 ksi for Grade 60 and higher, applied to the cross sectional area of the bar; allowable steel shear strength = 0.17 • F_u • A_{nom}
3. Allowable load capacities are calculated for the steel element type. Consideration of applying additional safety factors may be necessary depending on the application, such as life safety or overhead.
4. Allowable steel strength in tension must be checked against allowable bond strength/concrete capacity in tension to determine the controlling allowable load.
5. The tabulated load values are applicable to single anchors installed at critical edge and spacing distances and where the minimum member thickness is the greater of [h_{nom} + 1-1/4"] and [h_{nom} + 2d_{bit}]

In-Service Temperature Chart for Allowable Load Capacities



Ultimate Load Capacities for Threaded Rod Installed with PE1000+ into the Block Face of Grout-Filled Concrete Masonry Walls[1,2,4]

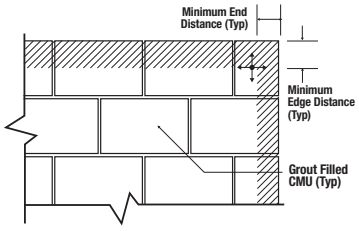
Nominal Rod Diameter d. in.	Drill Diameter d_{bit} in.	Minimum Embedment Depth in. (mm)	Minimum Edge Distance in. (mm)	Minimum End Distance in. (mm)	Ultimate Load[3]		Allowable Load	
					Tension lbs. (kN)	Shear lbs. (kN)	Tension lbs. (kN)	Shear lbs. (kN)
3/8	7/16	3 (76.2)	2-1/2 (63.5)	2-1/2 (63.5)	3,350 (14.9)	2,100 (9.3)	670 (2.9)	420 (1.9)
1/2	9/16	4 (101.6)	3 (76.2)	3 (76.2)	4,575 (20.3)	2,550 (11.3)	915 (4.1)	510 (2.3)
5/8	11/16	5 (127.0)	3-3/4 (95.3)	4 (101.6)	6,900 (30.7)	5,275 (23.5)	1,380 (6.1)	1,055 (4.7)

1. Tabulated load values are for anchors installed in minimum 8" wide, minimum Grade N, Type II, lightweight, medium-weight or normal-weight concrete masonry units conforming to ASTM C 90 that have reached a designated minimum compressive strength at the time of installation (f'm ≥1,500 psi). Mortar must be type N, S or M.
2. Anchor installations are limited to one per masonry cell. Shear loads may be applied in any direction.
3. The values listed are ultimate load capacities which should be reduced by a minimum safety factor of 5.0 or greater to determine the allowable working load. Consideration of safety factors of 10 or higher may be necessary depending on the application, such as life safety.
4. The tabulated values must be adjusted for increased in-service base material temperatures in accordance with the In-Service Temperature chart, as applicable.

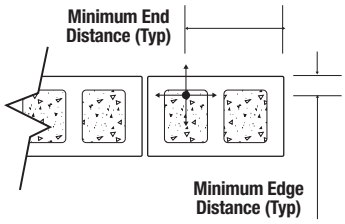
Load Capacities for Threaded Rod Installed with PE1000+ in the Top of Grout-Filled Concrete Masonry Walls[1,2,4]

Nominal Rod Diameter d. in.	Drill Diameter d_{bit} in.	Minimum Embedment Depth in. (mm)	Minimum Edge Distance in. (mm)	Minimum End Distance in. (mm)	Ultimate Load[3]		Allowable Load	
					Tension lbs. (kN)	Shear lbs. (kN)	Tension lbs. (kN)	Shear lbs. (kN)
1/2	9/16	6 (152.4)	1-3/4 (44.5)	3 (76.2)	5,950 (26.4)	1,450 (6.5)	1,190 (5.3)	290 (1.3)
5/8	11/16	8 (203.2)	1-3/4 (44.5)	4 (101.6)	9,450 (42.0)	1,700 (7.5)	1,890 (8.4)	340 (1.4)

1. Tabulated load values are for anchors installed in a minimum Grade N, Type II, lightweight, medium-weight or normal-weight masonry units conforming to ASTM C 90 that have reached a designated minimum compressive strength at the time of installation (f'm ≥1,500 psi). Mortar must be type N, S or M.
2. Anchor installations are limited to one per masonry cell. Shear loads may be applied in any direction.
3. The values listed are ultimate load capacities which should be reduced by a minimum safety factor of 5.0 or greater to determine the allowable working load. Consideration of safety factors of 10 or higher may be necessary depending on the application, such as life safety.
4. The tabulated values must be adjusted for increased in-service base material temperatures in accordance with the In-Service Temperature chart, as applicable.



Face Shell
Permissible Anchor Locations
(Un-hatched Area / Through Face Shell)



Top of Wall

STRENGTH DESIGN (SD)

Installation Specifications for Threaded Rod and Reinforcing Bar[1]

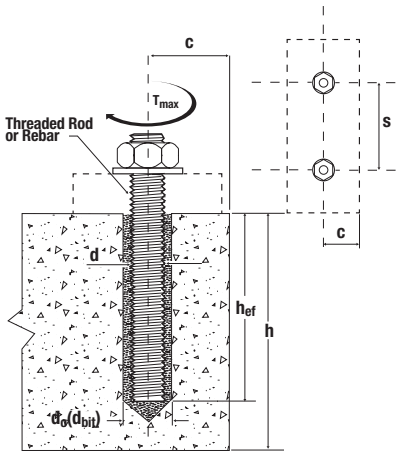
CODE LISTED ICC-ES ESR-2583

Dimension/Property	Notation	Units	Nominal Anchor Size									
Threaded Rod	-	-	3/8"	1/2"	-	5/8"	3/4"	7/8"	1"	-	1-1/4"	-
Reinforcing Bar	-	-	#3	-	#4	#5	#6	#7	#8	#9	-	#10
Nominal anchor diameter	d	in. (mm)	0.375 (9.5)	0.500 (12.7)		0.625 (15.9)	0.750 (19.1)	0.875 (22.2)	1.000 (25.4)	1.125 (28.6)	1.250 (31.8)	1.250 (31.8)
Carbide drill bit nominal size	d_o [d_{bit}]	in.	7/16 ANSI	9/16 ANSI	5/8 ANSI	11/16 or 3/4 ANSI	7/8 ANSI	1 ANSI	1-1/8 ANSI	1-3/8 ANSI	1-3/8 ANSI	1-1/2 ANSI
Diamond core bit nominal size	d_o [d_{bit}]	in.	-	5/8		3/4	7/8	1	1-1/8	-	-	-
Minimum embedment	$h_{ef,min}$	in. (mm)	2-3/8 (61)	2-3/4 (70)		3-1/8 (79)	3-1/2 (89)	3-1/2 (89)	4 (102)	4-1/2 (114)	5 (127)	5 (127)
Maximum embedment[4]	$h_{ef,max}$	in. (mm)	4-1/2 (114)	10 (254)		12-1/2 (318)	15 (381)	17-1/2 (445)	20 (508)	22-1/2 (572)	25 (635)	25 (635)
Minimum concrete member thickness	h_{min}	in. (mm)	h_{ef} + 1-1/4 (h_{ef} + 30)			h_{ef} + 2d_o						
Minimum spacing distance	s_{min}	in. (mm)	1-7/8 (48)	2-1/2 (62)		3-1/8 (80)	3-3/4 (95)	4-3/8 (111)	5 (127)	5-5/8 (143)	6-1/4 (159)	6-1/4 (159)
Minimum edge distance	c_{min}	in (mm)	5d where d is nominal outside diameter of the anchor									
Minimum edge distance, reduced5	$c_{min,red}$	in (mm)	1-3/4 (44)	1-3/4 (44)	1-3/4 (44)	1-3/4 (44)	1-3/4 (44)	1-3/4 (44)	1-3/4 (44)	2-3/4 (70)	2-3/4 (70)	2-3/4 (70)
Max. Torque[2]	T_{max}	ft-lbs	15	33		60	105	125	165	200	280	280
Max. Torque[2,3] (A36/Grade 36 rod)	T_{max}	ft-lbs	10	25		50	90	125	165	N/A	280	N/A
Max. Torque[2,4] (Class 1 SS rod)	T_{max}	ft-lbs	5	20		40	60	100	165	N/A	280	N/A
Effective cross sectional area of threaded rod	A_{se}	in.^{2} (mm^2)	0.078 (50)	0.142 (92)		0.226 (146)	0.335 (216)	0.462 (298)	0.606 (391)	-	0.969 (625)	-
Effective cross sectional area of reinforcing bar	A_{se}	in.^{2} (mm^2)	0.110 (71)	0.200 (129)		0.310 (200)	0.440 (284)	0.600 (387)	0.790 (510)	1.000 (645)	-	1.270 (819)

For SI: 1 inch = 25.4 mm, 1 ft-lbf = 1.356 N-m. For pound-inch units: 1 mm = 0.03937 inch, 1 N-m = 0.7375 ft-lbf.

1. For use with the design provisions of ACI 318-14 Ch.17 or ACI 318-11 Appendix D as applicable, ICC-ES AC308 Section 4.2 and ESR-2583
2. Torque may not be applied to the anchors until the full cure time of the adhesive has been achieved
3. These torque values apply to ASTM A36/F 1554 Grade 36 carbon steel threaded rods only.
4. These torque values apply to ASTM A197 Grade B8/BBM (Class 1) stainless steel threaded rods only
5. For installation at the reduced minimum edge distance, $c_{min,red}$, the max torque, T_{max} must be multiplied by a reduction factor of 0.45.
6. The maximum embedment is limited to 12 diameters for the horizontal and upwardly inclined installations and for installations in water-filled (flooded) holes with a carbide drill bit.

Detail of Steel Hardware Elements used with Injection Adhesive System



Steel Description (General)	Steel Specification (ASTM)	Nominal Anchor Size (inch)	Minimum Yield Strength f_y (ksi)	Minimum Ultimate Strength f_u (ksi)
Carbon Rod	A 36 or F 1554, Grade 36	3/8 through 1-1/4	36.0	58.0
	F 1554 Grade 55		55.0	75.0
	A 193, Grade B7 or F 1554, Grade 105		105.0	125.0
Stainless Rod (Alloy 304 / 316)	F 593 Condition CW	3/8 through 5/8	65.0	100.0
		3/4 through 1-1/4	45.0	85.0
Grade 60 Reinforcing Bar	A 615, or A 767, A 996	3/8 through 1-1/4 (#3 through #10)	60.0	90.0
	A 706		60.0	80.0
Grade 40 Reinforcing Bar	A 615	3/8 through 3/4 (#3 through #6)	40.0	60.0

Steel Tension and Shear Design for Threaded Rod in Normal Weight Concrete (For use with load combinations taken from ACI 318-14 Section 5.3)

CODE LISTED ICC-ES ESR-2583

Design Information		Symbol	Units	Nominal Rod Diameter[1] (inch)						
				3/8	1/2	5/8	3/4	7/8	1	1-1/4
Threaded rod nominal outside diameter		d	inch (mm)	0.375 (9.5)	0.500 (12.7)	0.625 (15.9)	0.750 (19.1)	0.875 (22.2)	1.000 (25.4)	1.250 (31.8)
Threaded rod effective cross-sectional area		A_{se}	inch² (mm²)	0.0775 (50)	0.1419 (92)	0.2260 (146)	0.3345 (216)	0.4617 (298)	0.6057 (391)	0.9691 (625)
ASTM A 36 and ASTM F 1554 Grade 36	Nominal strength as governed by steel strength (for a single anchor)	N_{sa}	lbf (kN)	4,495 (20.0)	8,230 (36.6)	13,110 (58.3)	19,400 (86.3)	26,780 (119.1)	35,130 (156.3)	56,210 (250.0)
		V_{sa}	lbf (kN)	2,695 (12.0)	4,940 (22.0)	7,860 (35.0)	11,640 (51.8)	16,070 (71.4)	21,080 (93.8)	33,725 (150.0)
	Reduction factor for seismic shear	$\alpha_{V,seis}$	-	0.80	0.80	0.80	0.80	0.80	0.80	0.80
	Strength reduction factor for tension[2]	ϕ	-	0.75						
	Strength reduction factor for shear[2]	ϕ	-	0.65						
ASTM F 1554 Grade 55	Nominal strength as governed by steel strength(for a single anchor)	N_{sa}	lbf (kN)	5,810 (25.9)	10,640 (47.3)	16,950 (75.4)	25,085 (111.6)	34,625 (154.0)	45,425 (202.0)	72,680 (323.3)
		V_{sa}	lbf (kN)	3,485 (15.5)	6,385 (28.4)	10,170 (45.2)	15,050 (67.0)	20,775 (92.4)	27,255 (121.2)	43,610 (194.0)
	Reduction factor for seismic shear	$\alpha_{V,seis}$	-	0.80	0.80	0.80	0.80	0.80	0.80	0.80
	Strength reduction factor for tension[2]	ϕ	-	0.75						
	Strength reduction factor for shear[2]	ϕ	-	0.65						
ASTM A 193 Grade B7 and ASTM F 1554 Grade 105	Nominal strength as governed by steel strength (for a single anchor)	N_{sa}	lbf (kN)	9,685 (43.1)	17,735 (78.9)	28,250 (125.7)	41,810 (186.0)	57,710 (256.7)	75,710 (336.8)	121,135 (538.8)
		V_{sa}	lbf (kN)	5,815 (25.9)	10,640 (7.3)	16,950 (75.4)	25,085 (111.6)	34,625 (154.0)	45,425 (202.1)	72,680 (323.3)
	Reduction factor for seismic shear	$\alpha_{V,seis}$	-	0.80	0.80	0.80	0.80	0.80	0.80	0.80
	Strength reduction factor for tension[2]	ϕ	-	0.75						
	Strength reduction factor for shear[2]	ϕ	-	0.65						
ASTM F 593 CW Stainless (Types 304 and 316)	Nominal strength as governed by steel strength (for a single anchor)	N_{sa}	lbf (kN)	7,750 (34.5)	14,190 (63.1)	22,600 (100.5)	28,430 (126.5)	39,245 (174.6)	51,485 (229.0)	82,370 (366.4)
		V_{sa}	lbf (kN)	4,650 (20.7)	8,515 (37.9)	13,560 (60.3)	17,060 (75.9)	23,545 (104.7)	30,890 (137.4)	49,425 (219.8)
	Reduction factor for seismic shear	$\alpha_{V,seis}$	-	0.70	0.70	0.80	0.80	0.80	0.80	0.80
	Strength reduction factor for tension[3]	ϕ	-	0.65						
	Strength reduction factor for shear[3]	ϕ	-	0.60						
ASTM A 193 Grade B8/B8M, Class 1 Stainless (Types 304 and 316)	Nominal strength as governed by steel strength (for a single anchor)[4]	N_{sa}	lbf (kN)	4,420 (19.7)	8,090 (36.0)	12,880 (57.3)	19,065 (84.8)	26,315 (117.1)	34,525 (153.6)	55,240 (245.7)
		V_{sa}	lbf (kN)	2,650 (11.8)	4,855 (21.6)	7,730 (34.4)	11,440 (50.9)	15,790 (70.2)	20,715 (92.1)	33,145 (147.4)
	Reduction factor for seismic shear	$\alpha_{V,seis}$	-	0.70	0.70	0.80	0.80	0.80	0.80	0.80
	Strength reduction factor for tension[2]	ϕ	-	0.75						
	Strength reduction factor for shear[2]	ϕ	-	0.65						
ASTM A 193 Grade B8/ B8M2, Class 2B Stainless (Types 304 and 316)	Nominal strength as governed by steel strength (for a single anchor)	N_{sa}	lbf (kN)	7,365 (32.8)	13,480 (60.0)	21,470 (95.5)	31,775 (141.3)	43,860 (195.1)	57,545 (256.0)	92,065 (409.5)
		V_{sa}	lbf (kN)	4,420 (19.7)	8,085 (36.0)	12,880 (57.3)	19,065 (84.8)	26,315 (117.1)	34,525 (153.6)	55,240 (245.7)
	Reduction factor for seismic shear	$\alpha_{V,seis}$	-	0.70	0.70	0.80	0.80	0.80	0.80	0.80
	Strength reduction factor for tension[2]	ϕ	-	0.75						
	Strength reduction factor for shear[2]	ϕ	-	0.65						

For SI: 1 inch = 25.4 mm, 1 lbf = 4.448 N. For pound-inch units: 1 mm = 0.03937 inches, 1 N = 0.2248 lbf.

1. Values provided for steel element material types are based on minimum specified strengths and calculated in accordance with ACI 318-14 Eq. 17.4.1.2 and Eq. 17.5.1.2b or ACI 318-11 Eq. (D-2) and Eq. (D-29), as applicable, except where noted. Nuts and washers must be appropriate for the rod. Nuts must have specified proof load stresses equal to or greater than the minimum tensile strength of the specified threaded rod.
2. The tabulated value of ϕ applies when the load combinations of Section 1605.2 of the IBC, ACI 318-14 5.3 or ACI 318-11 9.2, as applicable, are used in accordance with ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of ϕ must be determined in accordance with ACI 318 D.4.4. Values correspond to ductile steel elements.
3. The tabulated value of ϕ applies when the load combinations of Section 1605.2 of the IBC, ACI 318-14 5.3 or ACI 318-11 9.2, as applicable, are used in accordance with ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of ϕ must be determined in accordance with ACI 318 D.4.4. Values correspond to brittle steel elements
4. In accordance with ACI 318-14 17.4.1.2 and 17.5.1.2 or ACI 318-11 D.5.1.2 and D.6.1.2, as applicable, the calculated values for nominal tension and shear strength for ASTM A193 Grade B8/B8M Class 1 stainless steel threaded rods are based on limiting the specified tensile strength of the anchor steel to 1.9fy or 57,000 psi (393 MPa).

Steel Tension and Shear Design for Reinforcing Bars in Normal Weight Concrete (For use with load combinations taken from ACI 318-14 Section 5.3)

CODE LISTED ICC-ES ESR-2583

Design Information		Symbol	Units	Nominal Reinforcing Bar Size (Rebar)[1]							
				No. 3	No. 4	No. 5	No. 6	No. 7	No. 8	No. 9	No. 10
Rebar nominal outside diameter		d	inch (mm)	0.375 (9.5)	0.500 (12.7)	0.625 (15.9)	0.750 (19.1)	0.875 (22.2)	1.000 (25.4)	1.125 (28.7)	1.250 (32.3)
Rebar effective cross-sectional area		A_{se}	inch² (mm²)	0.110 (71.0)	0.200 (129.0)	0.310 (200.0)	0.440 (283.9)	0.600 (387.1)	0.790 (509.7)	1.000 (645.2)	1.270 (819.4)
ASTM A 615 Grade 75	Nominal strength as governed by steel strength (for a single anchor)	N_{sa}	lbf (kN)	11,000 (48.9)	20,000 (89.0)	31,000 (137.9)	44,000 (195.7)	60,000 (266.9)	79,000 (351.4)	100,000 (444.8)	127,000 (564.9)
		V_{sa}	lbf (kN)	6,600 (29.4)	12,000 (53.4)	18,600 (82.7)	26,400 (117.4)	36,000 (160.1)	47,400 (210.8)	60,000 (266.9)	76,200 (338.9)
	Reduction factor for seismic shear	$\alpha_{V,seis}$	-	0.70	0.70	0.80	0.80	0.80	0.80	0.80	0.80
	Strength reduction factor for tension[3]	ϕ	-	0.65							
	Strength reduction factor for shear[3]	ϕ	-	0.60							
ASTM A 615 Grade 60	Nominal strength as governed by steel strength (for a single anchor)	N_{sa}	lbf (kN)	9,900 (44.0)	18,000 (80.1)	27,900 (124.1)	39,600 (176.1)	54,000 (240.2)	71,100 (316.3)	90,000 (400.3)	114,300 (508.4)
		V_{sa}	lbf (kN)	5,940 (26.4)	10,800 (48.0)	16,740 (74.5)	23,760 (105.7)	32,400 (144.1)	42,660 (189.8)	54,000 (240.2)	68,580 (305.0)
	Reduction factor for seismic shear	$\alpha_{V,seis}$	-	0.70	0.70	0.80	0.80	0.80	0.80	0.80	0.80
	Strength reduction factor for tension[2]	ϕ	-	0.75							
	Strength reduction factor for shear[2]	ϕ	-	0.65							
ASTM A 706 Grade 60	Nominal strength as governed by steel strength (for a single anchor)	N_{sa}	lbf (kN)	8,800 (39.1)	16,000 (71.2)	24,800 (110.3)	35,200 (156.6)	48,000 (213.5)	63,200 (281.1)	80,000 (355.9)	101,600 (452.0)
		V_{sa}	lbf (kN)	5,280 (23.5)	9,600 (42.7)	14,880 (66.2)	21,120 (94.0)	28,800 (128.1)	37,920 (168.7)	48,000 (213.5)	60,960 (271.2)
	Reduction factor for seismic shear	$\alpha_{V,seis}$	-	0.70	0.70	0.80	0.80	0.80	0.80	0.80	0.80
	Strength reduction factor for tension[2]	ϕ	-	0.75							
	Strength reduction factor for shear[2]	ϕ	-	0.65							
ASTM A 615 Grade 40	Nominal strength as governed by steel strength (for a single anchor)	N_{sa}	lbf (kN)	6,600 (29.4)	12,000 (53.4)	18,600 (82.7)	26,400 (117.4)	In accordance with ASTM A 615, Grade 40 bars are furnished only in sizes No. 3 through No. 6			
		V_{sa}	lbf (kN)	3,960 (17.6)	7,200 (32.0)	11,160 (49.6)	15,840 (70.5)				
	Reduction factor for seismic shear	$\alpha_{V,seis}$	-	0.70	0.70	0.80	0.80				
	Strength reduction factor for tension[2]	ϕ	-	0.75							
	Strength reduction factor for shear[2]	ϕ	-	0.65							

For SI: 1 inch = 25.4 mm, 1 lbf = 4.448 N. For pound-inch units: 1 mm = 0.03937 inches, 1 N = 0.2248 lbf.

1. Values provided for reinforcing bar material types based on minimum specified strengths and calculated in accordance with ACI 318-14 Eq. 17.4.1.2 and Eq. 17.5.1.2b or ACI 318-11 Eq. (D-2) and Eq. (D-29), as applicable.
2. The tabulated value of ϕ applies when the load combinations of Section 1605.2 of the IBC, ACI 318-14 5.3 or ACI 318-11 9.2, as applicable, are used in accordance with ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of ϕ must be determined in accordance with ACI 318 D.4.4. Values correspond to ductile steel elements. In accordance with ACI 318-14 17.2.3.4.3(a)(vi) or ACI 318-11 D.3.3.4.3(a)6, as applicable, deformed reinforcing bars meeting this specification used as ductile steel elements to resist earthquake effects shall be limited to reinforcing bars satisfying the requirements of ACI 318-14 20.2.2.4 and 20.2.2.5 or ACI 318-11 21.1.5.2 (a) and (b), as applicable.
3. The tabulated value of ϕ applies when the load combinations of Section 1605.2 of the IBC, ACI 318-14 5.3 or ACI 318-11 9.2, as applicable, are used in accordance with ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of ϕ must be determined in accordance with ACI 318 D.4.4. Values correspond to brittle steel elements.

Concrete Breakout Design Information for Threaded Rod and Reinforcing Bars (For use with loads combinations taken from ACI 318-14 Section 5.3)[1]

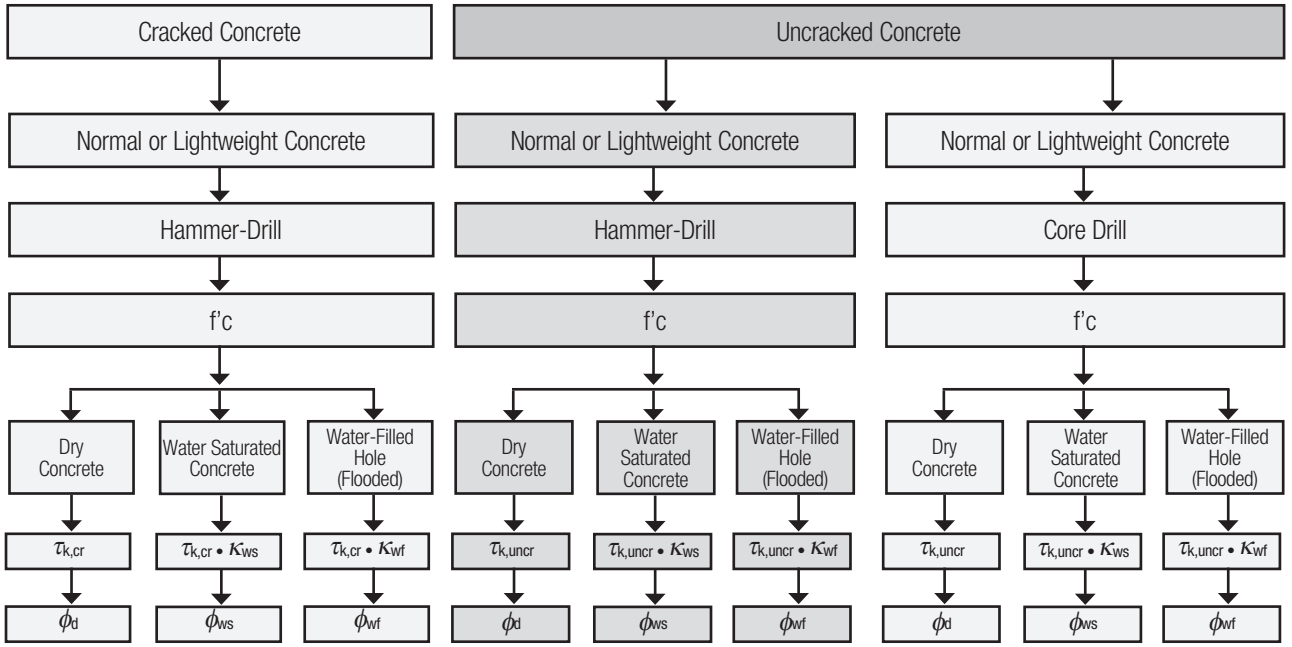
CODE LISTED
ICC-ES ESR-2583

Design Information	Symbol	Units	Nominal Rod Diameter (inch) / Reinforcing Bar Size							
			3/8 or #3	1/2 or #4	5/8 or #5	3/4 or #6	7/8 or #7	1 or #8	#9	1-1/4 or #10
Effectiveness factor for cracked concrete	$k_{c,cr}$	- (SI)	Not Applicable	17 (7.1)						
Effectiveness factor for uncracked concrete	$k_{c,uncr}$	- (SI)	24 (10.0)							
Minimum embedment	$h_{ef,min}$	inch (mm)	2-3/8 (60)	2-3/4 (70)	3-1/8 (79)	3-1/2 (89)	3-1/2 (89)	4 (102)	4-1/2 (114)	5 (127)
Maximum embedment	$h_{ef,max}$	inch (mm)	7-1/2 (191)	10 (254)	12-1/2 (318)	15 (381)	17-1/2 (445)	20 (508)	22-1/2 (572)	25 (635)
Minimum anchor spacing	s_{min}	inch (mm)	1-7/8 (48)	2-1/2 (64)	3-1/8 (79)	3-3/4 (95)	4-3/8 (111)	5 (127)	5-5/8 (143)	6-1/4 (159)
Minimum edge distance[2]	c_{min}	inch (mm)	5*d* where *d* is nominal outside diameter of the anchor							
Minimum edge distance,reduced[2]	$c_{min,red}$	inch (mm)	1-3/4 (45)	1-3/4 (45)	1-3/4 (45)	1-3/4 (45)	1-3/4 (45)	1-3/4 (45)	2-3/4 (70)	2-3/4 (70)
Minimum member thickness	h_{min}	inch (mm)	h_{ef} + 1-1/4 (h_{ef} + 30)		$h_{ef} + 2d_o$ where d_o is hole diameter;					
Critical edge distance—splitting (for uncracked concrete only)[3]	c_{ac}	inch	$c_{ac} = h_{ef} \cdot (\frac{\tau_{uncr}}{1160})^{0.4} \cdot [3.1\text{-}0.7 \frac{h}{h_{ef}}]$							
		(mm)	$c_{ac} = h_{ef} \cdot (\frac{\tau_{uncr}}{8})^{0.4} \cdot [3.1\text{-}0.7 \frac{h}{h_{ef}}]$							
Strength reduction factor for tension, concrete failure modes, Condition B[4]	ϕ	-	0.65							
Strength reduction factor for shear, concrete failure modes, Condition B[4]	ϕ	-	0.70							

For SI: 1 inch = 25.4 mm, 1 lbf = 4.448 N. For pound-inch units: 1 mm = 0.03937 inch, 1 N = 0.2248 lbf.

1. Additional setting information is described in the installation instructions.
2. For installation between the minimum edge distance, c_{min}, and the reduced minimum edge distance, $c_{min,red}$, the maximum torque applied must be reduced (multiplied) by a factor of 0.45.
3. $\tau_{k,uncr}$ need not be taken as greater than: $\tau_{k,uncr} = \frac{k_{uncr} \cdot \sqrt{h_{ef} \cdot f'c}}{\pi \cdot d}$ and $\frac{h}{h_{ef}}$ need not be taken as larger than 2.4.
4. Condition A requires supplemental reinforcement, while Condition B applies where supplemental reinforcement is not provided or where pryout governs, as set forth in ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable. The tabulated value of ϕ applies when the load combinations of Section 1605.2 of the IBC, ACI 318-14 5.3 or ACI 318-11 9.2, as applicable, are used in accordance with ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of ϕ must be determined in accordance with ACI 318 D.4.4.

FLOWCHART FOR THE ESTABLISHMENT OF DESIGN BOND STRENGTH



Bond Strength Design Information for Threaded Rods and Reinforcing Bars in Holes Drilled with a Hammer Drill and Carbide Bit (For use with load combinations taken from ACI 318-14 Section 5.3)[1]

Design Information		Symbol	Units	Nominal Rod Diameter (inch) / Reinforcing Bar Size							
				3/8 or #3	1/2 or #4	5/8 or #5	3/4 or #6	7/8 or #7	1 or #8	#9	11/4 or #10
Minimum embedment		$h_{ef,min}$	inch (mm)	2-3/8 (60)	2-3/4 (70)	3-1/8 (79)	3-1/2 (89)	3-1/2 (89)	4 (102	4-1/2 (114)	5 (127)
Maximum embedment	Dry concrete and saturated concrete[7]	$h_{ef,max}$	inch (mm)	4-1/2 (114)	10 (254)	12-1/2 (318)	15 (381)	17-1/2 (445)	20 (508)	22-1/2 (572)	25 (635)
	Water-filled hole (flooded)	$h_{ef,max}$	inch (mm)	4-1/2 (114)	6 (152)	7-1/2 (190)	9 (225)	10-1/2 (267)	12 (305)	13-1/2 (343)	15 (381)
110°F (43°C) Maximum Long-Term Service Temperature; 140°F (60°C) Maximum Short-Term Service Temperature[2,4]	Characteristic bond strength in cracked concrete[5,8]	$\tau_{k,cr}$	psi (N/mm²)	N/A	576 (4.0)	474 (3.3)	441 (3.0)	416 (2.9)	416 (2.9)	416 (2.9)	416 (2.9)
	Characteristic bond strength in uncracked concrete[5,9]	$\tau_{k,uncr}$	psi (N/mm²)	1,223 (8.4)	1,156 (8.0)	1,106 (7.6)	1,067 (7.4)	1,036 (7.1)	1,010 (7.0)	986 (6.8)	966 (6.7)
110°F (43°C) Maximum Long-Term Service Temperature; 176°F (80°C) Maximum Short-Term Service Temperature[3,4]	Characteristic bond strength in cracked concrete[5,8]	$\tau_{k,cr}$	psi (N/mm²)	N/A	455 (3.1)	374 (2.6)	349 (2.4)	329 (2.3)	329 (2.3)	329 (2.3)	329 (2.3)
	Characteristic bond strength in uncracked concrete[5,9]	$\tau_{k,uncr}$	psi (N/mm²)	966 (6.7)	913 (6.3)	874 (6.0)	843 (5.8)	819 (5.6)	798 (5.5)	779 (5.4)	763 (5.3)
Permissible installation conditions[6]	Dry concrete	ϕ_d	-	0.65	0.65	0.65	0.65	0.65	0.65	0.65	0.65
	Water-saturated concrete	ϕ_{ws}	-	0.45	0.45	0.45	0.45	0.45	0.45	0.45	0.45
		κ_{ws}		0.93	0.9	0.96	1.0	1.0	1.0	1.0	0.99
	Water-filled hole (flooded)	ϕ_{wf}	-	0.45	0.45	0.45	0.45	0.45	0.45	0.45	0.45
		κ_{wf}		0.93	0.83	0.75	0.70	0.65	0.62	0.59	0.56
Reduction factor for seismic tension		$\alpha_{N,seis}$	-	1.0							

For SI: 1 inch = 25.4 mm, 1 psi = 0.006894 MPa. For pound-inch units: 1 mm = 0.03937 inch, 1 MPa = 145.0 psi.

1. Bond strength values correspond to a normal-weight concrete compressive strength f'c = 2,500 psi (17.2 MPa). For concrete compressive strength, f'c between 2,500 psi and 8,000 psi (17.2 MPa and 55.2 MPa), the tabulated characteristic bond strength may be increased by a factor of $(f'c / 2{,}500)^{0.12}$ [For SI: $(f'c / 17.2)^{0.12}$].
2. The maximum short-term service temperature may be increased to 162°F (72°C) provided characteristic bond strengths are reduced by 10 percent. Long-term and short-term temperatures meet the requirements of Section 8.5 of ACI 355.4 and Table 8.1, Temperature Category B.
3. Long-term and short-term temperatures meet the requirements of Section 8.5 of ACI 355.4 and Table 8.1, Temperature Category A.
4. Short-term base material service temperatures are those that occur over brief intervals, e.g. as a result of diurnal cycling. Long-term base material service temperatures are roughly constant over significant periods of time.
5. Characteristic bond strengths are for sustained loads including dead and live loads.
6. Permissible installation conditions include dry concrete, water-saturated concrete and water-filled holes. Water-filled holes include applications in dry or water-saturated concrete where the drilled holes contain standing water at the time of anchor installation.
7. Maximum embedment is limited to twelve anchor diameters for horizontal and upwardly inclined installations.
8. For structures assigned to Seismic Design Categories C, D, E or F, bond strength values for cracked concrete do not require an additional reduction factor applied for seismic tension ($\alpha_{N,seis}$ = 1.0), where seismic design is applicable.
9. Bond strength values for uncracked concrete are applicable for structures assigned to Seismic Design Categories A and B only.

Bond Strength Design Information for Threaded Rods and Reinforcing Bars in Holes Drilled with a Core Drill and Diamond Core Bit (For use with load combinations taken from ACI 318-14 Section 5.3)[1]

Design Characteristic		Notation	Units	Nominal Rod Diameter (inch) / Reforcing Bar Size				
				1/2" or #4	5/8" or #5	3/4" or #6	7/8" or #7	1" or #8
Minimum embedment		$h_{ef,min}$	in. (mm)	2-3/4 (70)	3-1/8 (79)	3-1/2 (89)	3-1/2 (89)	4 (102)
Maximum embedment[7]		$h_{ef,max}$	in. (mm)	10 (54)	12-1/2 (318)	15 (381)	17-1/2 (445)	20 (508)
110°F (43°C) Maximum Long-Term Service Temperature; 140°F (60°C) Maximum Short-Term Service Temperature[2,4]	Characteristic bond strength in uncracked concrete[5,8]	$\tau_{k,uncr}$	psi (N/mm²)	1,133 (7.8)	1,075 (7.4)	1,033 (7.1)	1,022 (6.9)	975 (6.7)
110°F (43°C) Maximum Long-Term Service Temperature; 176°F (80°C) Maximum Short-Term Service Temperature[3,4]	Characteristic bond strength in uncracked concrete[5,8]	$\tau_{k,uncr}$	psi (N/mm²)	895 (6.2)	849 (5.9)	816 (5.6)	791 (5.5)	770 (5.3)
Permissible Installation Conditions[6]	Dry concrete	ϕ_d	-	0.55	0.45	0.45	0.45	0.45
	Water-saturated concrete	ϕ_{ws}	-	0.45	0.45	0.45	0.45	0.45
		κ_{ws}	-	1.0	1.0	1.0	1.0	1.0
	Water-filled hole (flooded)	ϕ_{ws}	-	0.45	0.45	0.45	0.45	0.45
		κ_{wf}	-	0.94	0.95	0.95	0.95	0.96

For SI: 1 inch = 25.4 mm, 1 psi = 0.006894 MPa. For pound-inch units: 1 mm = 0.03937 inch, 1 MPa = 145.0 psi.

1. Bond strength values correspond to a normal-weight concrete compressive strength f'c = 2,500 psi (17.2 MPa). For concrete compressive strength, f'c between 2,500 psi and 8,000 psi (17.2 MPa and 55.2 MPa), the tabulated characteristic bond strength may be increased by a factor of $(f'c / 2{,}500)^{0.12}$ [For SI: $(f'c / 17.2)^{0.12}$].
2. The maximum short-term service temperature may be increased to 162°F (72°C) provided characteristic bond strengths are reduced by 10 percent. Long-term and short-term temperatures meet the requirements of Section 8.5 of ACI 355.4 and Table 8.1, Temperature Category B.
3. Long-term and short-term temperatures meet the requirements of Section 8.5 of ACI 355.4 and Table 8.1, Temperature Category A.
4. Short-term base material service temperatures are those that occur over brief intervals, e.g. as a result of diurnal cycling. Long-term base material service temperatures are roughly constant over significant periods of time.
5. Characteristic bond strengths are for sustained loads including dead and live loads.
6. Permissible installation conditions include dry concrete, water-saturated concrete and water-filled holes. Water-filled holes include applications in dry or water-saturated concrete where the drilled holes contain standing water at the time of anchor installation.
7. Maximum embedment is limited to twelve anchor diameters for horizontal and upwardly inclined installations.
8. For structures assigned to Seismic Design Categories C, D, E or F, bond strength values for cracked concrete do not require an additional reduction factor applied for seismic tension ($\alpha_{N,seis}$ = 1.0), where seismic design is applicable.
9. Bond strength values for uncracked concrete are applicable for structures assigned to Seismic Design Categories A and B only.

Tension and Shear Design Strength Installed in Uncracked Concrete (Bond or Concrete Strength) Drilled with a Hammer-Drill and Carbide Bit in a Dry Hole Condition 110°F (43°C) Maximum Long-Term Service Temperature; 140°F (60°C) Maximum Short-Term Service Temperature[1,2,3,4,5,6,7,8,9]

Nominal Rod/Rebar Size (in. or #)	Embed. Depth h_{ef} (in.)	Minimum Concrete Compressive Strength									
		f'c = 2,500 psi		f'c = 3,000 psi		f'c = 4,000 psi		f'c = 6,000 psi		f'c = 8,000 psi	
		ΦNcb or ΦNa Tension (lbs.)	ΦVcb or ΦVcp Shear (lbs.)	ΦNcb or ΦNa Tension (lbs.)	ΦVcb or ΦVcp Shear (lbs.)	ΦNcb or ΦNa Tension (lbs.)	ΦVcb or ΦVcp Shear (lbs.)	ΦNcb or ΦNa Tension (lbs.)	ΦVcb or ΦVcp Shear (lbs.)	ΦNcb or ΦNa Tension (lbs.)	ΦVcb or ΦVcp Shear (lbs.)
3/8 or #3	2-3/8	2,225	2,330	2,275	2,450	2,355	2,535	2,470	2,660	2,555	2,755
	3	2,810	3,460	2,870	3,825	2,975	4,480	3,120	5,595	3,230	6,550
	4-1/2	4,215	6,320	4,310	6,985	4,460	8,175	4,680	10,085	4,845	10,435
1/2 or #4	2-3/4	3,245	3,185	3,320	3,520	3,435	4,120	3,605	5,145	3,730	6,025
	4	4,720	5,990	4,825	6,620	4,995	7,755	5,245	9,680	5,430	11,335
	6	7,080	10,915	7,240	12,065	7,495	14,125	7,865	16,945	8,145	17,540
	10	11,805	23,250	12,065	25,690	12,490	26,895	13,110	28,240	13,570	29,230
5/8 or #5	3-1/8	4,310	4,120	4,510	4,595	4,665	5,375	4,900	6,715	5,070	7,860
	5	7,060	9,175	7,215	10,140	7,465	11,870	7,840	14,825	8,115	17,355
	7-1/2	10,585	16,710	10,820	18,465	11,200	21,620	11,760	25,330	12,170	26,220
	12-1/2	17,645	35,610	18,035	38,845	18,670	40,210	19,600	42,215	20,285	43,695
3/4 or #6	3-1/2	5,105	5,015	5,480	5,700	5,735	6,790	6,000	8,480	6,195	9,925
	6	9,805	12,775	10,020	14,115	10,375	16,525	10,890	20,635	11,275	24,160
	9	14,705	23,265	15,035	25,710	15,560	30,100	16,335	35,185	16,910	36,420
	15	24,510	49,560	25,055	53,965	25,935	55,860	27,225	58,645	28,185	60,705
7/8 or #7	3-1/2	5,085	4,930	5,290	5,605	5,625	6,855	5,980	8,765	6,175	10,260
	7	12,960	15,900	13,245	17,570	13,710	20,570	14,395	25,690	14,900	30,075
	10-1/2	19,435	28,960	19,865	32,000	20,565	37,465	21,590	46,500	22,350	48,135
	17-1/2	32,395	61,700	33,110	68,185	34,275	73,820	35,985	77,500	37,245	80,225
1 or #8	4	6,240	6,115	6,685	6,945	7,110	8,495	7,645	11,045	7,895	12,930
	8	16,500	19,225	16,865	21,245	17,455	24,870	18,325	31,060	18,970	36,360
	12	24,750	35,010	25,295	38,690	26,185	45,295	27,490	56,570	28,455	61,290
	20	41,250	74,605	42,160	82,440	43,640	94,000	45,820	98,685	47,430	102,150
#9	4-1/2	7,445	7,110	8,105	8,080	8,615	9,880	9,350	13,025	9,655	15,250
	9	20,385	22,755	20,835	25,145	21,570	29,440	22,645	36,765	23,440	43,045
	13-1/2	30,580	41,450	31,255	45,805	32,355	53,630	33,965	66,970	35,160	75,730
	22-1/2	50,965	88,290	52,095	97,570	53,920	114,230	56,610	121,930	58,600	126,215
1-1/4	5	8,720	8,170	9,555	9,285	10,495	11,355	11,450	15,085	11,870	17,755
	10	24,660	26,380	25,205	29,150	26,090	34,130	27,390	42,620	28,350	49,895
	15	36,985	48,045	37,805	53,090	39,130	62,155	41,085	77,625	42,525	90,880
	25	61,645	102,380	63,005	113,140	65,220	132,460	68,470	147,480	70,875	152,660
#10	5	8,720	8,160	9,555	9,270	10,375	11,335	11,315	15,060	11,725	17,725
	10	24,660	26,425	25,205	29,200	26,090	34,190	27,390	42,695	28,350	49,985
	15	36,985	48,130	37,805	53,190	39,130	62,270	41,085	77,765	42,525	91,045
	25	61,645	102,530	63,005	113,305	65,220	132,655	68,470	147,480	70,875	152,660

□ - Concrete Breakout Strength □ - Bond Strength/Pryout Strength

1. Tabular values are provided for illustration and are applicable for single anchors installed in uncracked normal-weight concrete with minimum slab thickness, $h_a = h_{min}$, and with the following conditions:
   - c_{a1} is greater than or equal to the critical edge distance, c_{ac}
   - c_{a2} is greater than or equal to 1.5 times c_{a1}.
2. Calculations were performed according to ACI 318-14 Ch.17 and ICC-ES AC308. The load level corresponding to the failure mode listed [Concrete breakout strength, bond strength/pryout strength] must be checked against the tabulated steel strength of the corresponding threaded rod or rebar size and type, the lowest load level controls.
3. Strength reduction factors (ϕ) for concrete breakout strength are based on ACI 318-14 Section 5.3 for load combinations. Condition B was assumed.
4. Strength reduction factors (ϕ) for bond strength are determined from reliability testing and qualification in accordance with ICC-ES AC308 and are tabulated in this product information and in ESR-2583.
5. Tabular values are permitted for static loads only, seismic loading is not considered with these tables. Periodic special inspection must be performed where required by code, see ESR-2583 for applicable information.
6. For anchors subjected to tension resulting from sustained loading a supplemental check must be performed according to ACI 318-14 17.3.1.2.
7. For designs that include combined tension and shear, the interaction of tension and shear loads must be calculated in accordance with ACI 318-14 Ch.17.
8. Interpolation is not permitted to be used with the tabular values. For intermediate base material compressive strengths, please see ACI 318-14 Ch.17, ICC-ES AC308 and information included in this product supplement. For other design conditions including seismic considerations please see ACI 318-14 Ch.17 and ICC-ES AC308 and ESR-2583.
9. Long term concrete temperatures are roughly constant over significant periods of time. Short-term elevated temperatures are those that occur over brief intervals, e.g. as a result of diurnal cycling.

Tension and Shear Design Strength Installed in Cracked Concrete (Bond or Concrete Strength) Drilled with a Hammer-Drill and Carbide Bit in a Dry Hole Condition 110°F (43°C) Maximum Long-Term Service Temperature; 140°F (60°C) Maximum Short-Term Service Temperature[1,2,3,4,5,6,7,8,9]

Nominal Rod/Rebar Size (in. or #)	Embed. Depth h_{ef} (in.)	Minimum Concrete Compressive Strength									
		f'c = 2,500 psi		f'c = 3,000 psi		f'c = 4,000 psi		f'c = 6,000 psi		f'c = 8,000 psi	
		ΦN_{cb} or ΦN_a Tension (lbs.)	ΦV_{cb} or ΦV_{cp} Shear (lbs.)	ΦN_{cb} or ΦN_a Tension (lbs.)	ΦV_{cb} or ΦV_{cp} Shear (lbs.)	ΦN_{cb} or ΦN_a Tension (lbs.)	ΦV_{cb} or ΦV_{cp} Shear (lbs.)	ΦN_{cb} or ΦN_a Tension (lbs.)	ΦV_{cb} or ΦV_{cp} Shear (lbs.)	ΦN_{cb} or ΦN_a Tension (lbs.)	ΦV_{cb} or ΦV_{cp} Shear (lbs.)
1/2 or #4	2-3/4	1,615	2,275	1,655	2,515	1,710	2,945	1,795	3,675	1,860	4,005
	4	2,350	4,280	2,405	4,730	2,490	5,360	2,615	5,630	2,705	5,825
	6	3,530	7,600	3,605	7,770	3,735	8,040	3,920	8,440	4,055	8,740
	10	5,880	12,665	6,010	12,945	6,220	13,400	6,535	14,070	6,760	14,565
5/8 or #5	3-1/8	1,890	2,940	1,930	3,280	2,000	3,840	2,100	4,525	2,175	4,680
	5	3,025	6,515	3,090	6,660	3,200	6,895	3,360	7,235	3,480	7,490
	7-1/2	4,535	9,770	4,640	9,990	4,800	10,340	5,040	10,855	5,215	11,235
	12-1/2	7,560	16,285	7,730	16,645	8,000	17,230	8,400	18,090	8,695	18,725
3/4 or #6	3-1/2	2,175	3,580	2,265	4,070	2,370	4,850	2,480	5,340	2,560	5,515
	6	4,050	8,730	4,140	8,920	4,290	9,235	4,500	9,695	4,660	10,035
	9	6,080	13,090	6,215	13,380	6,430	13,850	6,750	14,545	6,990	15,055
	15	10,130	21,820	10,355	22,305	10,720	23,085	11,255	24,240	11,650	25,090
7/8 or #7	3-1/2	2,045	3,525	2,125	4,000	2,260	4,865	2,400	5,170	2,480	5,340
	7	5,205	11,205	5,320	11,455	5,505	11,855	5,780	12,450	5,980	12,885
	10-1/2	7,805	16,810	7,975	17,180	8,255	17,785	8,670	18,670	8,975	19,330
	17-1/2	13,010	28,015	13,295	28,635	13,760	29,640	14,450	31,120	14,955	32,215
1 or #8	4	2,650	4,365	2,755	4,960	2,930	6,065	3,150	6,780	3,250	7,005
	8	6,795	13,730	6,945	14,960	7,190	15,485	7,550	16,260	7,815	16,830
	12	10,195	21,955	10,420	22,440	10,785	23,230	11,325	24,390	11,720	25,245
	20	16,990	36,595	17,365	37,405	17,975	38,715	18,870	40,645	19,535	42,075
#9	4-1/2	3,290	5,080	3,420	5,770	3,635	7,060	3,945	8,495	4,075	8,775
	9	8,600	16,255	8,790	17,960	9,100	19,600	9,555	20,575	9,890	21,300
	13-1/2	12,900	27,790	13,185	28,405	13,650	29,400	14,330	30,865	14,835	31,950
	22-1/2	21,505	46,315	21,980	47,340	22,750	49,000	23,885	51,445	24,725	53,250
1-1/4	5	4,090	5,835	4,250	6,630	4,520	8,110	4,930	10,620	5,110	11,010
	10	10,620	18,840	10,855	20,820	11,235	24,200	11,795	25,405	12,210	26,295
	15	15,930	34,305	16,280	35,065	16,850	36,295	17,690	38,105	18,315	39,445
	25	26,545	57,175	27,135	58,440	28,085	60,495	29,485	63,510	30,525	65,740
#10	5	4,045	5,830	4,205	6,620	4,465	8,100	4,870	10,495	5,050	10,880
	10	10,620	18,875	10,855	20,860	11,235	24,200	11,795	25,405	12,210	26,295
	15	15,930	34,305	16,280	35,065	16,850	36,295	17,690	38,105	18,315	39,445
	25	26,545	57,175	27,135	58,440	28,085	60,495	29,485	63,510	30,525	65,740

☐ - Concrete Breakout Strength ☐ - Bond Strength/Pryout Strength

1. Tabular values are provided for illustration and are applicable for single anchors installed in cracked normal-weight concrete with minimum slab thickness, $h_a = h_{min}$, and with the following conditions:
   - c_{a1} is greater than or equal to the critical edge distance, c_{ac}
   - c_{a2} is greater than or equal to 1.5 times c_{a1}.
2. Calculations were performed according to ACI 318-14 Ch.17 and ICC-ES AC308. The load level corresponding to the failure mode listed [Concrete breakout strength, bond strength/pryout strength] must be checked against the tabulated steel strength of the corresponding threaded rod or rebar size and type, the lowest load level controls.
3. Strength reduction factors (ϕ) for concrete breakout strength are based on ACI 318-14 Section 5.3 for load combinations. Condition B was assumed.
4. Strength reduction factors (ϕ) for bond strength are determined from reliability testing and qualification in accordance with ICC-ES AC308 and are tabulated in this product information and in ESR-2583.
5. Tabular values are permitted for static loads only, seismic loading is not considered with these tables. Periodic special inspection must be performed where required by code, see ESR-2583 for applicable information.
6. For anchors subjected to tension resulting from sustained loading a supplemental check must be performed according to ACI 318-14 17.3.1.2.
7. For designs that include combined tension and shear, the interaction of tension and shear loads must be calculated in accordance with ACI 318-14 Ch.17.
8. Interpolation is not permitted to be used with the tabular values. For intermediate base material compressive strengths, please see ACI 318-14 Ch.17, ICC-ES AC308 and information included in this product supplement. For other design conditions including seismic considerations please see ACI 318-14 Ch.17 and ICC-ES AC308 and ESR-2583.
9. Long term concrete temperatures are roughly constant over significant periods of time. Short-term elevated temperatures are those that occur over brief intervals, e.g. as a result of diurnal cycling.

Tension and Shear Design Strength Installed in Uncracked Concrete (Bond or Concrete Strength) Drilled with a Core-Drill and Diamond Core Bit in a Dry Hole Condition 110°F (43°C) Maximum Long-Term Service Temperature; 140°F (60°C) Maximum Short-Term Service Temperature[1,2,3,4,5,6,7,8,9]

Nominal Rod/Rebar Size (in. or #)	Embed. Depth h_{ef} (in.)	Minimum Concrete Compressive Strength									
		f'c = 2,500 psi		f'c = 3,000 psi		f'c = 4,000 psi		f'c = 6,000 psi		f'c = 8,000 psi	
		ΦN_{cb} or ΦN_a Tension (lbs.)	ΦV_{cb} or ΦV_{cp} Shear (lbs.)	ΦN_{cb} or ΦN_a Tension (lbs.)	ΦV_{cb} or ΦV_{cp} Shear (lbs.)	ΦN_{cb} or ΦN_a Tension (lbs.)	ΦV_{cb} or ΦV_{cp} Shear (lbs.)	ΦN_{cb} or ΦN_a Tension (lbs.)	ΦV_{cb} or ΦV_{cp} Shear (lbs.)	ΦN_{cb} or ΦN_a Tension (lbs.)	ΦV_{cb} or ΦV_{cp} Shear (lbs.)
1/2 or #4	2-3/4	2,690	3,160	2,750	3,490	2,850	4,085	2,990	5,105	3,095	5,975
	4	3,915	5,945	4,000	6,570	4,145	7,690	4,350	9,605	4,500	11,245
	6	5,875	10,830	6,005	11,965	6,215	14,010	6,525	16,605	6,755	17,190
	10	9,790	23,065	10,005	25,465	10,355	26,360	10,875	27,675	11,255	28,650
5/8 or #5	3-1/8	2,970	4,110	3,035	4,540	3,140	5,320	3,295	6,640	3,410	7,775
	5	4,750	9,090	4,855	10,045	5,025	11,760	5,275	14,685	5,460	16,990
	7-1/2	7,125	16,555	7,280	18,290	7,535	21,415	7,915	24,620	8,190	25,485
	12-1/2	11,875	35,260	12,135	37,755	12,560	39,080	13,190	41,030	13,650	42,470
3/4 or #6	3-1/2	3,570	5,015	3,720	5,700	3,855	6,700	4,030	8,370	4,160	9,800
	6	6,570	12,610	6,715	13,935	6,955	16,310	7,300	20,370	7,555	23,510
	9	9,855	22,965	10,075	25,375	10,430	29,710	10,950	34,065	11,335	35,260
	15	16,430	48,925	16,795	52,245	17,380	54,080	18,250	56,775	18,890	58,770
7/8 or #7	3-1/2	3,445	4,930	3,580	5,605	3,810	6,855	4,015	8,645	4,145	10,125
	7	8,675	15,690	8,870	17,340	9,180	20,300	9,635	25,350	9,975	29,675
	10-1/2	13,015	28,575	13,300	31,580	13,770	36,970	14,455	44,975	14,965	46,555
	17-1/2	21,690	60,885	22,170	67,280	22,950	71,400	24,095	74,960	24,940	77,590
1 or #8	4	4,350	6,115	4,520	6,945	4,810	8,495	5,120	10,890	5,290	12,745
	8	11,025	18,955	11,270	20,945	11,665	24,520	12,250	30,625	12,680	35,855
	12	16,540	34,520	16,905	38,150	17,500	44,665	18,375	55,775	19,020	59,165
	20	27,565	73,560	28,175	81,285	29,165	90,740	30,620	95,265	31,695	98,610

□ - Concrete Breakout Strength □ - Bond Strength/Pryout Strength

1. Tabular values are provided for illustration and are applicable for single anchors installed in uncracked normal-weight concrete with minimum slab thickness, $h_a = h_{min}$, and with the following conditions:
   - c_{a1} is greater than or equal to the critical edge distance, c_{ac}
   - c_{a2} is greater than or equal to 1.5 times c_{a1}.
2. Calculations were performed according to ACI 318-14 Ch.17 and ICC-ES AC308. The load level corresponding to the failure mode listed [Concrete breakout strength, bond strength/pryout strength] must be checked against the tabulated steel strength of the corresponding threaded rod or rebar size and type, the lowest load level controls.
3. Strength reduction factors (ϕ) for concrete breakout strength are based on ACI 318-14 Section 5.3 for load combinations. Condition B was assumed.
4. Strength reduction factors (ϕ) for bond strength are determined from reliability testing and qualification in accordance with ICC-ES AC308 and are tabulated in this product information and in ESR-2583.
5. Tabular values are permitted for static loads only, seismic loading is not considered with these tables. Periodic special inspection must be performed where required by code, see ESR-2583 for applicable information.
6. For anchors subjected to tension resulting from sustained loading a supplemental check must be performed according to ACI 318-14 17.3.1.2.
7. For designs that include combined tension and shear, the interaction of tension and shear loads must be calculated in accordance with ACI 318-14 Ch.17.
8. Interpolation is not permitted to be used with the tabular values. For intermediate base material compressive strengths, please see ACI 318-14 Ch.17, ICC-ES AC308 and information included in this product supplement. For other design conditions including seismic considerations please see ACI 318-14 Ch.17 and ICC-ES AC308 and ESR-2583.
9. Long term concrete temperatures are roughly constant over significant periods of time. Short-term elevated temperatures are those that occur over brief intervals, e.g. as a result of diurnal cycling.

Tension Design of Steel Elements (Steel Strength)[1,2]

Steel Elements - Threaded Rod and Reinforcing Bar										
Nominal Rod/Rebar Size (in. or No.)	ASTM A36 and ASTM F1554 Grade 36	ASTM F1554 Grade 55	ASTM A193 Grade B7 and ASTM F1554 Grade 105	ASTM F593 CW Stainless (Types 304 and 316)	ASTM A193 Grade B8/ B8M, Class 1 Stainless (Types 304 and 316)	ASTM A193 Grade B8/ B8M2, Class 2B Stainless (Types 304 and 316)	ASTM A615 Grade 75 Rebar	ASTM A615 Grade 60 Rebar	ASTM A706 Grade 60 Rebar	ASTM A615 Grade 40 Rebar
	ϕN_{sa} Tension (lbs.)	ϕN_{sa} Tension (lbs.)	ϕN_{sa} Tension (lbs.)	ϕN_{sa} Tension (lbs.)	ϕN_{sa} Tension (lbs.)	ϕN_{sa} Tension (lbs.)	ϕN_{sa} Tension (lbs.)	ϕN_{sa} Tension (lbs.)	ϕN_{sa} Tension (lbs.)	ϕN_{sa} Tension (lbs.)
3/8 or #3	3,370	4,360	7,265	5,040	3,315	5,525	7,150	7,425	6,600	4,950
1/2 or #4	6,175	7,980	13,300	9,225	6,070	10,110	13,000	13,500	12,000	9,000
5/8 or #5	9,835	12,715	21,190	14,690	9,660	16,105	20,150	20,925	18,600	13,950
3/4 or #6	14,550	18,815	31,360	18,480	14,300	23,830	28,600	29,700	26,400	19,800
7/8 or #7	20,085	25,970	43,285	25,510	19,735	32,895	39,000	40,500	36,000	-
1 or #8	26,350	34,070	56,785	33,465	25,895	43,160	51,350	53,325	47,400	-
#9	-	-	-	-	-	-	65,000	67,500	60,000	-
1-1/4 or #10	42,160	54,510	90,850	53,540	41,430	69,050	82,550	85,725	76,200	-

■ - Steel Strength

1. Steel tensile design strength according to ACI 318-14 Ch. 17, $\phi N_{sa} = \phi \bullet A_{se,N} \bullet f_{uta}$
2. The tabulated steel design strength in tension must be checked against the bond strength/concrete capacity design strength to determine the controlling failure mode, the lowest load level controls.

Shear Design of Steel Elements (Steel Strength)[1,2]

Steel Elements - Threaded Rod and Reinforcing Bar										
Nominal Rod/Rebar Size (in. or No.)	ASTM A36 and ASTM F1554 Grade 36	ASTM F1554 Grade 55	ASTM A193 Grade B7 and ASTM F1554 Grade 105	ASTM F593 CW Stainless (Types 304 and 316)	ASTM A193 Grade B8/ B8M, Class 1 Stainless (Types 304 and 316)	ASTM A193 Grade B8/ B8M2, Class 2B Stainless (Types 304 and 316)	ASTM A615 Grade 75 Rebar	ASTM A615 Grade 60 Rebar	ASTM A706 Grade 60 Rebar	ASTM A615 Grade 40 Rebar
	ϕV_{sa} Tension (lbs.)	ϕV_{sa} Tension (lbs.)	ϕV_{sa} Tension (lbs.)	ϕV_{sa} Tension (lbs.)	ϕV_{sa} Tension (lbs.)	ϕV_{sa} Tension (lbs.)	ϕV_{sa} Tension (lbs.)	ϕV_{sa} Tension (lbs.)	ϕV_{sa} Tension (lbs.)	ϕV_{sa} Tension (lbs.)
3/8 or #3	1,755	2,265	3,775	2,790	1,725	2,870	3,960	3,860	3,430	2,575
1/2 or #4	3,210	4,150	6,915	5,110	3,155	5,255	7,200	7,020	6,240	4,680
5/8 or #5	5,115	6,610	11,020	8,135	5,025	8,375	11,160	10,880	9,670	7,255
3/4 or #6	7,565	9,785	16,305	10,235	7,435	12,390	15,840	15,445	13,730	10,295
7/8 or #7	10,445	13,505	22,505	14,130	10,265	17,105	21,600	21,060	18,720	-
1 or #8	13,700	17,715	29,525	18,535	13,465	22,445	28,440	27,730	24,650	-
#9	-	-	-	-	-	-	36,000	35,100	31,200	-
1-1/4 or #10	21,920	28,345	47,240	29,655	21,545	35,905	45,720	44,575	39,625	-

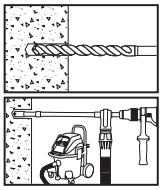
■ - Steel Strength

1. Steel shear design strength according to ACI 318-14 Ch. 17, $\phi V_{sa} = \phi \bullet 0.60 \bullet A_{se,N} \bullet f_{uta}$
2. The tabulated steel design strength in shear must be checked against the bond strength/concrete capacity design strength to determine the controlling failure mode, the lowest load level controls.

INSTALLATION INSTRUCTIONS (SOLID BASE MATERIALS)

HAMMER DRILLING

DRILLING

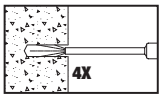


1- Drill a hole into the base material with rotary hammer drill (i.e. percussion drill) and a carbide drill bit to the size and embedment required by the selected steel hardware element (reference installation specifications for threaded rod and reinforcing bar). The tolerances of the carbide drill bits, including hollow bits, must meet ANSI Standard B212.15.

- Precaution: Use suitable eye and skin protection. Avoid inhalation of dust during drilling and/or removal.
- Note! In case of standing water in the drilled hole (flooded hole condition), all the water has to be removed from the hole (e.g. vacuum, compressed air, etc.) prior to cleaning.

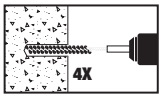
Drilling in dry base materials is recommended when using hollow drill bits (vacuum must be on).

HOLE CLEANING (BLOW 4X, BRUSH 4X, BLOW 4X)



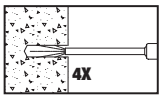
2a- Starting from the bottom or back of the drilled anchor hole, blow the hole clean using a compressed air nozzle (min. 90 psi) or a hand pump (supplied by DEWALT) a minimum of four times (4x).

- Use a compressed air nozzle (min. 90 psi) or a hand pump (min. volume 25 fl. oz.) for anchor rod 3/8" to 3/4" diameter or reinforcing bar (rebar) sizes #3 to #6.
- Use a compressed air nozzle (min. 90 psi) for anchor rod 7/8" to 1-1/4" diameter and rebar sizes #7 to #10. A hand pump shall not be used with these anchor sizes.



2b- Determine wire brush diameter (reference hole cleaning equipment selection table) and attach the brush with adaptor to a rotary drill tool or battery screw gun. Brush the hole with the selected wire brush a minimum of four times (4x). A brush extension (supplied by DEWALT, Cat. #08282) should be used for holes drilled deeper than the listed brush length.

- The wire brush diameter must be checked periodically during use. The brush should resist insertion into the drilled hole and come into contact with the sides of the drilled hole. If not the brush is too small and must be replaced.



2c- Finally, blow the hole clean again a minimum of four times (4x).

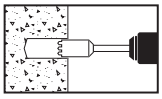
- Use a compressed air nozzle (min. 90 psi) or a hand pump (min. volume 25 fl. oz.) for anchor rod 3/8" to 3/4" diameter or reinforcing bar (rebar) sizes #3 to #6.
- Use a compressed air nozzle (min. 90 psi) for anchor rod 7/8" to 1-1/4" diameter and rebar sizes #7 to #10. A hand pump shall not be used with these anchor sizes.

When finished the hole should be clean and free of dust, debris, ice, grease, oil or other foreign material.

NEXT GO TO STEP 3.

CORE DRILLING

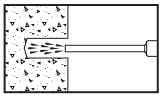
DRILLING



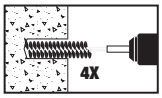
1- Drill a hole into the base material with a core drill tool to the size and embedment required by the selected steel hardware element (reference installation table). The tolerances of the carbide drill bit must meet ANSI Standard B212.15.

- Precaution: Use suitable eye and skin protection. Avoid inhalation of dust during drilling and/or removal.

HOLE CLEANING (RINSE, BRUSH 4X, RINSE, BLOW 4X, BRUSH 4X, BLOW 4X)

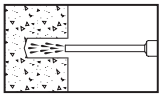


2a- Starting from the bottom or back of the drilled anchor hole, rinse/flush the hole clean with water (water line pressure) until clear water comes out.



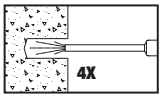
2b- Determine brush diameter (see installation table) for drilled hole and attach the brush with adaptor to a rotary drill tool or battery screw gun. Brush the hole with the selected wire brush a minimum of four times (4x). A brush extension (supplied by DEWALT) must be used for holes drilled deeper than the listed brush length.

- The wire brush diameter must be checked periodically during use The brush should resist insertion into the drilled hole and come into contact with the sides of the drilled hole. If not the brush is too small and must be replaced.

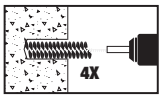


2c- Repeat Step 2a again by rinse/flush the hole clean with water.

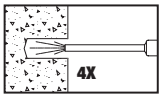
Following this remove all standing water completely (e.g. vacuum, compressed air, etc.) prior to further cleaning. To attain a dried borehole a DEWALT compressed air nozzle is recommended.



2d- Starting from the bottom or back of the drilled anchor hole, blow the hole clean (free of noticeable dust) a minimum if four times (4x). Use a compressed air nozzle (min. 90 psi) for all sizes of anchor rod and reinforcing bar (rebar)



2e- Repeat Step 2b again by brushing the hole with a wire brush a minimum of four times (4x).



2f- Repeat Step 2d again by blowing the hole clean a minimum of four times (4x).

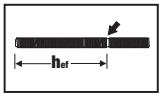
When finished the hole should be clean and free of dust, debris, ice, grease, oil or other foreign material.

NEXT GO TO STEP 3.

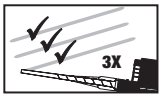
PREPARING

3- Check adhesive expiration date on cartridge label. Do not use expired product. Review Safety Data Sheet (SDS) before use. Cartridge temperature must be between 41°F - 95°F (5°C - 35°C) when in use; for downward applications only the adhesive temperature may be up to 104°F (40°C). Consideration should be given to the reduced gel time of the adhesive in warm temperatures.

- Attach a supplied mixing nozzle to the cartridge. Unless otherwise noted do not modify the mixer in any way and make sure the mixing element is inside the nozzle. Load the cartridge into the correct dispensing tool.
- A new mixing nozzle must be used for every working interruption longer than the published working times (reference gel time and curing time table) as well as for new cartridges.
- Note: Always use a new mixing nozzle with new cartridge of adhesive and also for all work interruptions exceeding the published gel (working) time of the adhesive.



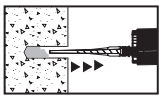
4- Prior to inserting the anchor rod or rebar into the filled bore hole, the position of the embedment depth has to be marked on the anchor. Verify anchor element is straight and free of surface damage.



5- For new cartridges and nozzles: prior to dispensing into the anchor hole, squeeze out separately a minimum three full strokes of the mixed adhesive. Discard non-uniform adhesives until the adhesive is a consistent **RED** color.

- Review and note the published working and cure times (reference gel time and curing time table) prior to injection of the mixed adhesive into the cleaned anchor hole.

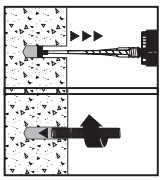
INSTALLATION



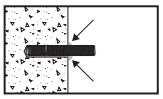
6- Fill the cleaned hole approximately two-thirds full with mixed adhesive starting from the bottom or back of the anchor hole. If the bottom or back of the anchor hole is not reached with the mixing nozzle only, a plastic extension tube must be used with the mixing nozzle (see reference tables for installation). Slowly withdraw the mixing nozzle as the hole fills to avoid creating air pockets or voids.

- Piston plugs (see adhesive piston plug table) must be used with and attached to the mixing nozzle and extension tube for horizontal and overhead installations with anchor rod from 5/8" to 1-1/4" diameter and rebar size #5 to #10. Insert piston plug to the back of the drilled hole and inject as described in the method above. During installation the piston plug will be naturally extruded from the drilled hole by the adhesive pressure.
- **Attention!** Do not install anchors overhead without proper training and installation hardware provided by the DEWALT. Contact DEWALT for details prior to use.

WITH PISTON PLUG:

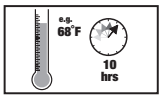


7- The anchor should be free of dirt, grease, oil or other foreign material. Push clean threaded rod or reinforcing bar into the anchor hole while turning slightly to ensure positive distribution of the adhesive until the embedment depth is reached. Air pockets are present when the threaded rod or rebar springs or air pockets burst during installation. In case of air pockets: remove rod or rebar, let the adhesive harden, re-drill the hole and repeat the complete installation.



8- Be sure that the anchor is fully seated at the bottom of the hole to the specified embedment. Adhesive must completely fill the annular gap between the anchor and the base material. Protect the anchor element threads from fouling with adhesive. For all installations the rebar must be restrained from movement throughout the specified curing period (as necessary) where necessary through the use of temporary wedges, external supports, or other methods. Minor adjustments to the position of the anchor element may be performed during the gel (working) time only.

CURING AND LOADING



9- Allow the adhesive anchor to cure to the specified full curing time prior to applying any load (reference gel time and curing time table).

- Do not disturb, torque or load the anchor until it is fully cured.

10- After full curing of the adhesive anchor, a fixture can be installed to the anchor and tightened up to the maximum torque (reference gel time and curing table) by using a calibrated torque wrench.

- Take care not to exceed the maximum torque for the selected anchor.

REFERENCE TABLES FOR INSTALLATION

Gel (working) Time and Curing Table

Temperature of Base Material		Gel (working) Time	Full Curing Time
°F	°C		
41	5	180 minutes	50 hours
50	10	120 minutes	30 hours
68	20	30 minutes	10 hours
86	30	20 minutes	6 hours
95	35	15 minutes	5 hours
104	40	12 minutes	4 hours

Cartridge adhesive temperature must be between 41°F - 95°F (5°C - 35°C) when in use; for downward applications only the cartridge adhesive temperature may be up to 104°F (40°C).

Hole Cleaning Equipment Selection Table for PE1000+[1,2,3]

Threaded rod diameter (inch)	Rebar size (no.)	ANSI drill bit diameter (inch)	Core drill bit diameter (inch)	Brush length (inches)	Steel wire brush (Cat. #)	Blowout tool	Number of cleaning actions
3/8	#3	7/16	7/16	6-3/4	08284	Hand-pump or compressed air nozzle (min. 90 psi)	4x blowing 4x brushing 4x blowing
1/2	-	9/16	9/16	6-3/4	08285		
-	#4	5/8	5/8	6-3/4	08275		
5/8	#5	11/16	11/16	7-7/8	08286		
		3/4	3/4	7-7/8	08278		
3/4	#6	7/8	7/8	7-7/8	08287		
7/8	#7	1	1	11-7/8	08288	Compressed air nozzle only (min. 90 psi)	
1	#8	1-1/8	1-1/8	11-7/8	08289		
		1-1/4	1-1/4	11-7/8	08274		
1-1/4	#9	1-3/8	1-3/8	11-7/8	08290		
-	#10	1-1/2	1-1/2	11-7/8	08291		

1. An SDS-plus adaptor (Cat. #08283) or Jacobs chuck style adaptor (Cat. #08296) is required to attach a steel wire brush to the drill tool.
2. For any case, it must be possible for the steel anchor element to be inserted into the cleaned hole without resistance.
3. A brush extension (Cat. #08282) must be used with a steel wire brush for holes drilled deeper than the listed brush length.

Piston Plugs for Adhesive Anchors[1,2]

Plug Size (inch)	ANSI Drill Bit Diameter (inch)	Plastic Plug (Cat. #)	Piston Plug
11/16	11/16	08258	
3/4	3/4	08259	
7/8	7/8	08300	
1	1	08301	
1-1/8	1-1/8	08303	
1-1/4	1-1/4	08307	
1-3/8	1-3/8	08305	
1-1/2	1-1/2	08309	

1. Overhead and horizontal installations require the use of piston plugs where one is tabulated together with the anchor size.
2. A plastic extension tube (Cat. #08281 or Cat. #08297) or equivalent approved by DEWALT must be used with piston plugs.

PERMISSIBLE INSTALLATION CONDITIONS (ADHESIVE)

Dry Concrete: cured concrete that, at the time of adhesive anchor installation, has not been exposed to water for the preceding 14 days.
Water-Saturated Concrete (wet): cured concrete that, at the time of adhesive anchor installation, has been exposed to water over a sufficient length of time to have the maximum possible amount of absorbed water into the concrete pore structure to a depth equal to the anchor embedment depth.
Water-Filled Holes (flooded): cured concrete that is water-saturated and where the drilled hole contains standing water at the time of anchor installation.

ORDERING INFORMATION

PE1000+ Cartridge System

Cat No.	Description	Std. Ctn.	Pallet
0500SD	PE1000+ 13 fl. oz. dual cartridge	12	540
0502SD	PE1000+ 19.5 fl. oz. dual cartridge	12	540

One PE1000+ mixing nozzle is packaged with each cartridge.
PE1000+ mixing nozzles must be used to ensure complete and proper mixing of the adhesive.

Extra Mixing Nozzles

Cat No.	Description	Std. Ctn.	Pallet
08294	Extra mixing nozzle (with an 8" extension) for PE1000+	2	24
08281	Mixing nozzle extension, 8" long	2	24
08297	Mixing nozzle extension, 20" long	1	12

Dispensing Tools for Injection Adhesive

Cat No.	Description	Std. Ctn.	Std. Carton
08298	13 fl. oz. and 19.5 fl. oz. Manual Tool	1	6
08497SD	19.5 fl. oz. Pneumatic tool	1	-
DCE593D1	13 fl. oz. and 19.5 fl. oz. 20 v Battery powered dispensing tool	1	-

Hole Cleaning Tools and Accessories

Cat No.	Description	Std. Box
08284	Wire brush for 7/16" or 1/2" ANSI hole, 6-3/4" length	1
08285	Wire brush for 9/16" ANSI hole, 6-3/4" length	1
08275	Wire brush for 5/8" ANSI hole, 6-3/4" length	1
08286	Wire brush for 11/16" ANSI hole, 7-7/8" length	1
08278	Wire brush for 3/4" ANSI hole, 7-7/8" length	1
08287	Wire brush for 7/8" ANSI hole, 7-7/8" length	1
08288	Wire brush for 1" ANSI hole, 11-7/8" length	1
08289	Wire brush for 1-1/8" ANSI hole, 11-7/8" length	1
08274	Wire brush for 1-1/4" ANSI hole, 11-7/8" length	1
08290	Wire brush for 1-3/8" ANSI hole, 11-7/8" length	1
08291	Wire brush for 1-1/2" ANSI hole, 11-7/8" length	1
08283	SDS-plus adapter for steel brushes	1
08296	Standard drill adapter for steel brushes (e.g. Jacobs Chuck)	1
08282	Steel brush extension, 12" length	1
08280	Hand pump/dust blower (25 fl. oz. cylinder volume)	1
08292	Air compressor nozzle with extension, 18" length	1
52073	Adhesive cleaning kit, includes 4 wire brushes (08284, 08285, 08286, 08287), steel brush extension (08282), SDS-plus adapter (08283), standard drill adapter (08296), hand pump/dust blower (08280), gloves and safety glasses	1

Adhesive Piston Plugs

Cat No.	Description	ANSI Drill Bit Dia.	Std. Bag
08258	11/16" Plug	11/16"	10
08259	3/4" Plug	3/4"	10
08300	7/8" Plug	7/8"	10
08301	1" Plug	1"	10
08303	1-1/8" Plug	1-1/8"	10
08307	1-1/4" Plug	1-1/4"	10
08305	1-3/8" Plug	1-3/8"	10
08309	1-1/2" Plug	1-1/2"	10

ICC EVALUATION SERVICE
In Cooperation with Innovation RESEARCH LABS

Most Widely Accepted and Trusted

ICC-ES Evaluation Report

ICC-ES | (800) 423-6587 | (562) 699-0543 | www.icc-es.org

ESR-2583

Reissued 12/2018
This report is subject to renewal 12/2019.
.

DIVISION: 03 00 00—CONCRETE
SECTION: 03 16 00—CONCRETE ANCHORS
DIVISION: 05 00 00—METALS
SECTION: 05 05 19—POST-INSTALLED CONCRETE ANCHORS

REPORT HOLDER:

DEWALT

EVALUATION SUBJECT:

PE1000+® EPOXY ADHESIVE ANCHOR SYSTEM IN CRACKED AND UNCRACKED CONCRETE (DEWALT / POWERS)

"2014 Recipient of Prestigious Western States Seismic Policy Council (WSSPC) Award in Excellence"

A Subsidiary of ICC INTERNATIONAL CODE COUNCIL®

ICC-ES Evaluation Report

ESR-2583

Reissued December 2018

This report is subject to renewal December 2019.

www.icc-es.org | (800) 423-6587 | (562) 699-0543 *A Subsidiary of the International Code Council®*

DIVISION: 03 00 00—CONCRETE
Section: 03 16 00—Concrete Anchors

DIVISION: 05 00 00—METALS
Section: 05 05 19—Post-Installed Concrete Anchors

REPORT HOLDER:

DEWALT

ADDITIONAL LISTEE:

POWERS FASTENERS

EVALUATION SUBJECT:

PE1000+® EPOXY ADHESIVE ANCHOR SYSTEM IN CRACKED AND UNCRACKED CONCRETE (DEWALT / POWERS)

1.0 EVALUATION SCOPE

Compliance with the following codes:

- 2018, 2015, 2012, and 2009 *International Building Code*® (IBC)
- 2018, 2015, 2012, and 2009 *International Residential Code*® (IRC)

Property evaluated:

- Structural

2.0 USES

2.1 General:

The PE1000+ epoxy adhesive anchors are used to resist static, wind or earthquake (IBC Seismic Design Categories A through F) tension and shear loads in cracked and uncracked normal-weight concrete or lightweight concrete with $^{1}/_{2}$-, $^{5}/_{8}$-, $^{3}/_{4}$-, $^{7}/_{8}$-, 1-, and $1^{1}/_{4}$-inch-diameter (12.7, 15.9, 19.1, 22.2, 25.4 and 31.8 mm) threaded steel rods and No. 4 through No. 10 steel reinforcing bars in hammer-drilled holes.

The anchors are used to resist static, wind or earthquake (IBC Seismic Design Categories A and B only) tension and shear loads in uncracked normal-weight concrete or lightweight concrete only with $^{3}/_{8}$-inch-diameter (9.5 mm) threaded steel rods and No. 3 steel reinforcing bars in hammer-drilled holes, and uncracked normal-weight or lightweight concrete only with $^{1}/_{2}$-, $^{5}/_{8}$-, $^{3}/_{4}$-, $^{7}/_{8}$- and 1-inch-diameter (12.7, 15.9, 19.1, 22.2 and 25.4 mm) threaded steel rods and No. 4 through No. 8 steel reinforcing bars in core drilled holes. Use is limited to normal-weight or lightweight concrete with a specified compressive strength, f'_c, of 2,500 psi to 8,500 psi (17.2 MPa to 58.6 MPa).

The anchor system complies with anchors as described in Section 1901.3 of the 2018 and 2015 IBC, Section 1909 of the 2012 IBC and is an alternative to cast-in-place and post-installed anchors described in Section 1908 of the 2012 IBC, and Sections 1911 and 1912 of the 2009IBC. The anchor systems may also be used where an engineered design is submitted in accordance with Section R301.1.3 of the IRC.

3.0 DESCRIPTION

3.1 General:

The PE1000+ Epoxy Adhesive Anchor System is comprised of a two-component epoxy adhesive provided in cartridges, static mixing nozzles, dispensing tools, hole cleaning equipment and adhesive injection accessories.

Product names for the report holder and the additional listee is presented in the following table of this report.

COMPANY NAME	PRODUCT NAME
DEWALT	PE1000+®
	(Pure150-PRO outside North America)
Powers Fasteners	PE1000+®
	(Pure150-PRO outside North America)

PE1000+ epoxy adhesive may be used with continuously threaded steel rods or deformed steel reinforcing bars. The primary components of the PE1000+ Epoxy Adhesive Anchor System, including the epoxy adhesive cartridge, static mixing nozzle, the nozzle extension tube, dispensing tool and typical steel anchor elements, are shown in Figure 2 of this report. Manufacturer's printed installation instructions (MPII) and parameters, as included with each adhesive unit package, are replicated in Figure 3 of this report.

3.2 Materials:

3.2.1 PE1000+ Epoxy Adhesive: PE1000+ epoxy adhesive is an injectable two-component epoxy. The two components are separated by means of a labelled dual-cylinder cartridge. The two components combine and react when dispensed through a static mixing nozzle, supplied by DEWALT, which is attached to the cartridge. A nozzle extension tube is also packaged with the cartridge. The PE1000+ epoxy adhesive is available in 13-ounce (385 mL) and, 19.5-ounce (585 mL) cartridges. Each cartridge label is marked with the adhesive expiration date. The shelf life, as indicated by the expiration date, applies to an unopened cartridge when stored in accordance with Figure 3.

3.2.2 Hole Cleaning Equipment: Hole cleaning equipment is comprised of steel wire brushes and an air pump supplied by DEWALT, and a compressed air nozzle. The equipment is shown in Figure 3 of this report.

3.2.3 Dispensers: PE1000+ epoxy adhesive must be dispensed with manual, pneumatic dispensers, or electric powered dispensers supplied by DEWALT.

3.2.4 Steel Anchor Elements:

3.2.4.1 Threaded Steel Rods: Threaded steel rods must be clean and continuously threaded (all-thread) in diameters as described in Table 4 and Figure 3 of this report. The embedded portions of threaded rods must be clean, straight, and free of mill scale, rust and other coatings (other than zinc) that may impair the bond with adhesive. Specifications for grades of threaded rod, including the mechanical properties and corresponding nuts and washers, are listed in Table 2 of this report. Carbon steel threaded rods may be furnished with a minimum 0.0002-inch-thick (0.005 mm) zinc electroplated coating complying with ASTM B633, SC1; or a minimum 0.0021-inch-thick (0.053 mm) mechanically deposited zinc coating complying with ASTM B695, Class 55; or a hot-dip galvanized zinc coating complying with ASTM A153, Class C or D. The stainless steel threaded rods must comply with Table 2 of this report. Steel grades and material types (carbon, stainless) of the washers and nuts must be matched to the threaded rods. Threaded steel rods must be straight and free of indentations or other defects along their length. The embedded end may be either flat cut or cut on the bias to a chisel point.

3.2.4.2 Steel Reinforcing Bars: Steel reinforcing bars are deformed reinforcing bars (rebars) as described in Table 1 of this report. Table 5 and Figure 3 of this report summarize reinforcing bar size ranges. The embedded portions of reinforcing bars must be clean, straight, and free of mill scale, rust and other coatings (other than zinc) that may impair the bond with the adhesive. Reinforcing bars must not be bent after installation, except as set forth in ACI 318-14 Section 26.6.3.1 (b) or ACI 318-11 Section 7.3.2, as applicable, with the additional condition that the bars must be bent cold, and heating of the reinforcing bars to facilitate field bending is not permitted.

3.2.4.3 Ductility: In accordance with ACI 318-14 2.3 or ACI 318-11 D.1, as applicable, in order for a steel anchor element to be considered ductile, the tested elongation must be at least 14 percent and the reduction of area must be at least 30 percent. Steel elements with a tested elongation of less than 14 percent or a reduction of area less than 30 percent, or both, are considered brittle. Values for various steel materials are provided in Tables 2 and 3 of this report. Where values are nonconforming or unstated, the steel element must be considered brittle.

3.3 Concrete:

Normal-weight and lightweight concrete must comply with Sections 1903 and 1905 of the IBC, as applicable. The specified compressive strength of the concrete must be from 2,500 psi to 8,500 psi (17.2 MPa to 58.6 MPa).

4.0 DESIGN AND INSTALLATION

4.1 Strength Design:

4.1.1 General: The design strength of anchors under the 2018 and 2015 IBC, as well as the 2018 and 2015 IRC must be determined in accordance with ACI 318-14 and this report. The design strength of anchors under the 2012 and 2009 IBC, as well the 2012 and 2009 IRC, must be determined in accordance with ACI 318-11 and this report.

The strength design of anchors must comply with ACI 318-14 17.3.1 or ACI 318-11 D.4.1, as applicable, except as required in ACI 318-14 17.2.3 or ACI 318-11 D.3.3, as applicable.

Design parameters are provided in Table 4 through Table 8. Strength reduction factors, ϕ, as given in ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable, must be used for load combinations calculated in accordance with Section 1605.2 of the IBC, ACI 318-14 5.3, or ACI 318-11 9.2, as applicable. Strength reduction factors, ϕ, as described in ACI 318-11 D.4.4 must be used for load combinations calculated in accordance with ACI 318-11 Appendix C.

4.1.2 Static Steel Strength in Tension: The nominal static steel strength of a single anchor in tension, N_{sa}, in accordance with ACI 318-14 17.4.1.2 or ACI 318-11 D.5.1.2, as applicable, and the associated strength reduction factors, ϕ, in accordance with ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable, are given in Tables 4 and 5 of this report for the anchor element types included in this report. See Table 1 for design use and table index.

4.1.3 Static Concrete Breakout Strength in Tension: The nominal static concrete breakout strength of a single anchor or group of anchors in tension, N_{cb} or N_{cbg}, must be calculated in accordance with ACI 318-14 17.4.2 or ACI 318-11 D.5.2, as applicable, with the following addition:

The basic concrete breakout strength of a single anchor in tension, N_b, must be calculated in accordance with ACI 318-14 17.4.2.2 or ACI 318-11 D.5.2.2, as applicable, using the selected values of $k_{c,cr}$ and $k_{c,uncr}$ as provided in the tables of this report. Where analysis indicates no cracking in accordance with ACI 318-14 17.4.2.6 or ACI 318-11 D.5.2.6, as applicable, N_b must be calculated using $k_{c,uncr}$ and $\Psi_{c,N}$ = 1.0. See Table 6 for additional design information. See ACI 318-14 17.2.6 or ACI 318-11 D.3.6, as applicable, for modification factor, λ_a, for lightweight concrete. The value of f'_c used for calculation must be limited to 8,000 psi (55 MPa) in accordance with ACI 318-14 17.2.7 or ACI 318-11 D.3.7, as applicable. Additional information for the determination of nominal bond strength in tension is given in Section 4.1.4 of this report.

4.1.4 Static Bond Strength in Tension: The nominal static bond strength of a single adhesive anchor or group of adhesive anchors in tension, N_a or N_{ag}, must be calculated in accordance with ACI 318-14 17.4.5 or ACI 318-11 D.5.5, as applicable. Bond strength values ($\tau_{k,cr}$, $\tau_{k,uncr}$) are a function of concrete compressive strength (f'_c), concrete state (cracked, uncracked), concrete type (normal weight, lightweight), drilling method (hammer-drill, core drilling) and installation conditions (dry concrete, water-saturated concrete, water-filled holes). Bond strength values must further be modified with the factor κ_{nn} for cases where holes are drilled in water-saturated concrete (κ_{ws}) or where the holes are water-filled at the time of anchor installation (κ_{wf}). Special inspection level is qualified as periodic for all anchors except as noted in Section 4.4 of this report. The selection of continuous special inspection level with an onsite proof loading program is not necessary and does not provide a benefit of a lower anchor category or an increase in the associated strength reduction factors for design).

The following table summarizes the requirements:

CONCRETE STATE	CONCRETE TYPE	DRILLING METHOD	BOND STRENGTH	CONCRETE COMPRESSIVE STRENGTH	PERMISSIBLE INSTALLATION CONDITIONS	ASSOCIATED STRENGTH REDUCTION FACTORS
Cracked	Normal weight or lightweight	Hammer-drill	$\tau_{k,cr}$	f'_c	Dry concrete	ϕ_d
					Water-saturated concrete	$\phi_{ws} \cdot \kappa_{ws}$
					Water-filled hole (flooded)	$\phi_{wf} \cdot \kappa_{wf}$
Uncracked	Normal weight or lightweight	Hammer-drill	$\tau_{k,uncr}$	f'_c	Dry concrete	ϕ_d
					Water-saturated concrete	$\phi_{ws} \cdot \kappa_{ws}$
					Water-filled hole (flooded)	$\phi_{wf} \cdot \kappa_{wf}$
Uncracked	Normal weight or lightweight	Core-drill	$\tau_{k,uncr}$	f'_c	Dry concrete	ϕ_d
					Water-saturated concrete	$\phi_{ws} \cdot \kappa_{ws}$
					Water-filled hole (flooded)	$\phi_{wf} \cdot \kappa_{wf}$

The bond strength values in Table 7 for hammer-drilled holes and in Table 8 for core drilled holes, correspond to concrete compressive strength f'_c equal to 2,500 psi (17.2 MPa) in normal weight concrete. See ACI 318-14 17.2.6 or ACI 318-11 D.3.6, as applicable, for modification factor, λ_a, for lightweight concrete. For concrete compressive strength, f'_c between 2,500 psi and 8,000 psi (17.2 MPa and 55.2 MPa), the tabulated characteristic bond strength may be increased by a factor of $(f'_c / 2{,}500)^{0.12}$ [For **SI:** $(f'_c / 17.2)^{0.12}$]. Where applicable, the modified bond strength values, including κ_{ws} or κ_{wf}, must be used in lieu of $\tau_{k,cr}$ and $\tau_{k,uncr}$ in Equations (D-21) and (D-22). The resulting nominal bond strength must be multiplied by the associated strength reduction factor ϕ_d, ϕ_{ws} or ϕ_{wf}, as applicable.

Figure 1 of this report presents a bond strength design selection flowchart. Strength reduction factors for determination of the bond strength are given in Tables 7 and 8 of this report. See Table 1. Adjustments to the bond strength may also be made for increased concrete compressive strength as noted in the footnotes to the corresponding tables.

4.1.5 Static Steel Strength in Shear: The nominal static steel strength of a single anchor in shear as governed by the steel, V_{sa}, in accordance with ACI 318-14 17.5.1.2 or ACI 318-11 D.6.1.2, as applicable, and strength reduction factors, ϕ, in accordance with ACI 318-14 17.2.3 or ACI 318-11 D.4.3, as applicable, are given in Tables 4 and 5 of this report for the anchor element types included in this report. See Table 1 for index of design tables.

4.1.6 Static Concrete Breakout Strength in Shear: The nominal concrete breakout strength of a single anchor or group of anchors in shear, V_{cb} or V_{cbg}, must be calculated in accordance with ACI 318-14 17.5.2 or ACI 318-11 D.6.2, as applicable, based on information given in Table 6 of this report. The basic concrete breakout strength in shear of a single anchor in cracked concrete, V_b, must be calculated in accordance with ACI 318-14 17.5.2.2 or ACI 318-11 D.6.2.2, as applicable, using the value of d given in Tables 4 and 5 of this report in lieu of d_a. In addition, h_{ef} must be substituted for ℓ_e. In no case shall ℓ_e exceed 8d. See ACI 318-14 17.2.6 or ACI 318-11 D.3.6, as applicable, for modification factor, λ_a, for lightweight concrete. The value of f'_c must be limited to a maximum of 8,000 psi (55.2 MPa), in accordance with ACI 318-14 17.2.7 or ACI 318-11 D.3.7.

4.1.7 Static Concrete Pryout Strength in Shear: The nominal static pryout strength of a single anchor or group of anchors in shear, V_{cp} or V_{cpg}, shall be calculated in accordance with ACI 318-14 17.5.3 or ACI 318-11 D.6.3, as applicable.

4.1.8 Interaction of Tensile and Shear Forces: For designs that include combined tension and shear, the interaction of tension and shear loads must be calculated in accordance with ACI 318-14 17.6 or ACI 318-11 D.7, as applicable.

4.1.9 Minimum Member Thickness, h_{min}, Anchor Spacing, s_{min}, Edge Distance, c_{min}: In lieu of ACI 318-14 17.7.1 and 17.7.3 or ACI 318-11 D.8.1 and D.8.3, as applicable, values of s_{min} and c_{min} described in this report must be observed for anchor design and installation. The minimum member thicknesses, h_{min}, described in this report must be observed for anchor design and installation. For adhesive anchors that will remain untorqued, ACI 318-14 17.7.4 or ACI 318-11 D.8.4, as applicable, applies.

For anchors that will be torqued during installation, the maximum torque, T_{max}, must be reduced for edge distances less than 5 anchor diameters (5d). T_{max} is subject to the edge distance, c_{min}, and anchor spacing, s_{min}, and shall comply with the following requirements:

MAXIMUM TORQUE SUBJECT TO EDGE DISTANCE			
NOMINAL ANCHOR SIZE, d	MIN. EDGE DISTANCE, c_{min}	MIN. ANCHOR SPACING, s_{min}	MAXIMUM TORQUE, T_{max}
all sizes	5d	5d	T_{max}
$^3/_8$ in. to 1 in. (9.5 mm to 25.4 mm)	1.75 in. (45 mm)	5d	$0.45 \cdot T_{max}$
$1^1/_4$ in. (31.8 mm)	2.75 in. (70 mm)	5d	$0.45 \cdot T_{max}$

For values of T_{max}, see Table 9 and Figure 3 of this report.

4.1.10 Critical Edge Distance c_{ac} and $\psi_{cp,Na}$: The modification factor $\psi_{cp,Na}$, must be determined in accordance with ACI 318-14 17.4.5.5 or ACI 318-11 D.5.5.5, as applicable, except as noted below:

For all cases where $c_{Na}/c_{ac}<1.0$, $\psi_{cp,Na}$ determined from ACI 318-14 Eq. 17.4.5.5b or ACI 318-11 Eq. D-27, as applicable, need not be taken less than c_{Na}/c_{ac}. For all other cases, $\psi_{cp,Na}$ shall be taken as 1.0.

The critical edge distance, c_{ac} must be calculated according to Eq. 17.4.5.5c for ACI 318-14 or Eq. D-27a for ACI 318-11, in lieu of ACI 318-14 17.7.6 or ACI 318-11 D.8.6, as applicable.

$$c_{ac} = h_{ef} \cdot \left(\frac{\tau_{k,uncr}}{1160}\right)^{0.4} \cdot \left[3.1 - 0.7\frac{h}{h_{ef}}\right]$$

(Eq. 17.4.5.5c for ACI 318-14 or Eq. D-27a for ACI 318-11)

where

$\left[\frac{h}{h_{ef}}\right]$ need not be taken as larger than 2.4; and where

$\tau_{k,uncr}$ = the characteristic bond strength stated in the tables of this report whereby $\tau_{k,uncr}$ need not be taken as larger than:

$$\tau_{k,uncr} = \frac{k_{uncr}\sqrt{h_{ef} f_c'}}{\pi \cdot d_a} \quad \text{Eq. (4-1)}$$

4.1.11 Design Strength in Seismic Design Categories C, D, E and F: In structures assigned to Seismic Design Category C, D, E or F under the IBC or IRC, anchors must be designed in accordance with ACI 318-14 17.2.3 or ACI 318-11 D.3.3, as applicable.

The nominal steel shear strength, V_{sa}, must be adjusted by $\alpha_{V,seis}$ as given in Tables 4 and 5 for the corresponding anchor steel. The nominal bond strength $\tau_{k,cr}$ need not be adjusted by $\alpha_{N,seis}$ since $\alpha_{N,seis}$ = 1.0.

As an exception to ACI 318-11 D.3.3.4.2: Anchors designed to resist wall out-of-plane forces with design strengths equal to or greater than the force determined in accordance with ASCE 7 Equation 12.11-1 or 12.14-10 shall be deemed to satisfy ACI 318-11 D.3.3.4.3(d).

Under ACI 318-11 D.3.3.4.3(d), in lieu of requiring the anchor design tensile strength to satisfy the tensile strength requirements of ACI 318-11 D.4.1.1, the anchor design tensile strength shall be calculated from ACI 318-11 D.3.3.4.4.

The following exceptions apply to ACI 318-11 D.3.3.5.2:

1. For the calculation of the in-plane shear strength of anchor bolts attaching wood sill plates of bearing or non-bearing walls of light-frame wood structures to foundations or foundation stem walls, the in-plane shear strength in accordance with ACI 318-11 D.6.2 and D.6.3 need not be computed and ACI 318-11 D.3.3.5.3 need not apply provided all of the following are satisfied:
   1.1. The allowable in-plane shear strength of the anchor is determined in accordance with AF&PA NDS Table 11E for lateral design values parallel to grain.
   1.2. The maximum anchor nominal diameter is $^5/_8$ inch (16 mm).
   1.3. Anchor bolts are embedded into concrete a minimum of 7 inches (178 mm).
   1.4. Anchor bolts are located a minimum of $1^3/_4$ inches (45 mm) from the edge of the concrete parallel to the length of the wood sill plate.
   1.5. Anchor bolts are located a minimum of 15 anchor diameters from the edge of the concrete perpendicular to the length of the wood sill plate.
   1.6. The sill plate is 2-inch or 3-inch nominal thickness.
2. For the calculation of the in-plane shear strength of anchor bolts attaching cold-formed steel track of bearing or non-bearing walls of light-frame construction to foundations or foundation stem walls, the in-plane shear strength in accordance with ACI 318-11 D.6.2 and D.6.3 need not be computed and ACI 318-11 D.3.3.5.3 need not apply provided all of the following are satisfied:
   2.1. The maximum nominal anchor diameter is $^5/_8$ inch (16 mm).
   2.2. Anchors are embedded into concrete a minimum of 7 inches (178 mm).
   2.3. Anchors are located a minimum of $1^3/_4$ inches (45 mm) from the edge of the concrete parallel to the length of the track.
   2.4. Anchors are located a minimum of 15 anchor diameters from the edge of the concrete perpendicular to the length of the track.
   2.5. The track is 33 to 68 mil designation thickness.

   Allowable in-plane shear strength of exempt anchors, parallel to the edge of concrete shall be permitted to be determined in accordance with AISI S100 Section E3.3.1.
3. In light-frame construction, bearing or nonbearing walls, shear strength of concrete anchors less than or equal to 1 inch [25 mm] in diameter attaching a sill plate or track to foundation or foundation stem wall need not satisfy ACI 318-11 D.3.3.5.3(a) through (c) when the design strength of the anchors is determined in accordance with ACI 318-11 D.6.2.1(c).

4.2 Allowable Stress Design (ASD):

4.2.1 General: For anchors designed using load combinations in accordance with IBC Section 1605.3 (Allowable Stress Design) loads must be established using the equations below:

$$T_{allowable,ASD} = \phi N_n / \alpha \quad \text{(Eq. 4-2)}$$

and

$$V_{allowable,ASD} = \phi V_n / \alpha \quad \text{(Eq. 4-3)}$$

where

$T_{allowable,ASD}$ = Allowable tension load (lbf or kN).

$V_{allowable,ASD}$ = Allowable shear load (lbf or kN).

ϕN_n = Lowest design strength of an anchor or anchor group in tension as determined in accordance with ACI 318-14 Chapter 17 or ACI 318 (-11, -08) Appendix D, as applicable, and 2018 and 2015 IBC Section 1905.1.8, 2012 IBC Errata Section 1905.1.9, 2009 IBC Section 1908.1.9 and Section 4.1 of this report, as applicable (lbf or kN).

ϕV_n = Lowest design strength of an anchor or anchor group in shear as determined in accordance with ACI 318-14 Chapter 17 or ACI 318 (-11, -08) Appendix D, as applicable, and 2018 and 2015 IBC Section 1905.1.8, 2012 IBC Errata Section 1905.1.9, 2009 IBC Section 1908.1.9 and Section 4.1 of this report, as applicable (lbf or kN).

α = Conversion factor calculated as a weighted average of the load factors for the controlling load combination. In addition, α must include all applicable factors to account for non-ductile failure modes and required over-strength.

The requirements for member thickness, edge distance and spacing, described in this report must apply.

4.2.2 Interaction of Tensile and Shear Forces: Interaction must be calculated in accordance with ACI 318-14 17.6 or ACI 318 (-11,-08) D.7, as applicable, as follows:

For shear loads $V \le 0.2\ V_{allowable,ASD}$, the full allowable load in tension shall be permitted.

For tension loads $T \le 0.2\ T_{allowable,ASD}$, the full allowable load in shear shall be permitted.

For all other cases:

$$\frac{T}{T_{allowable,ASD}} + \frac{V}{V_{allowable,ASD}} \le 1.2 \quad \text{Eq. (4-4)}$$

4.3 Installation:

Installation parameters are provided in Table 9 of this report. Installation must be in accordance with ACI 318-14 17.8.1 and 17.8.2 or ACI 318-11 D.9.1 and D.9.2, as applicable. Anchor locations must comply with this report and the plans and specifications approved by the code

official. Installation of the PE1000+ Epoxy Adhesive Anchor System must conform to the Manufacturer's printed installation instructions (MPII) included in each unit package and as reproduced in Figure 3 of this report.

The adhesive anchor system may be used for upwardly inclined orientation applications (e.g. overhead). Upwardly inclined and horizontal orientation applications are to be installed using piston plugs for the $^5/_8$-inch through $1^1/_4$-inch diameter threaded steel rods and No. 5 through No. 10 steel reinforcing bars, installed in the specified hole diameter, and attached to the mixing nozzle and extension tube supplied by DEWALT as described in Figure 6 in this report. Upwardly inclined and horizontal orientation installation for the $^3/_8$-inch and $^1/_2$-inch diameter threaded steel rods, and No. 3 and No. 4 steel reinforcing bars may be injected directly to the end of the hole using a mixing nozzle with a hole depth $h_0 \leq 10$" (250 mm).

Installation of anchors in horizontal or upwardly inclined (overhead) orientations shall be fully restrained from movement throughout the specified curing period through the use of temporary wedges, external supports, or other methods. Where temporary restraint devices are used, their use shall not result in impairment of the anchor shear resistance.

4.4 Special Inspection:

Periodic special inspection must be performed where required in accordance with Section 1705.1.1 and Table 1705.3 of the 2018, 2015 and 2012 IBC, 1704.4 and 1704.15 of the 2009 IBC, and this report. The special inspector must be on the jobsite initially during anchor installation to verify the anchor type, adhesive identification and expiration date, anchor dimensions, concrete type, concrete compressive strength, hole dimensions, hole cleaning procedures, anchor spacing, edge distances, concrete thickness, anchor embedment, tightening torque and adherence to the manufacturer's printed installation instructions (MPII). The special inspector must verify the initial installations of each type and size of adhesive anchor by construction personnel on the site. Subsequent installations of the same anchor type and size by the same construction personnel are permitted to be performed in the absence of the special inspector. Any change in the anchor product being installed or the personnel performing the installation requires an initial inspection. For ongoing installations over an extended period, the special inspector must make regular inspections to confirm correct handling and installation of the product.

Continuous special inspection of adhesive anchors installed in horizontal or upwardly inclined orientations to resist sustained tension loads must be performed in accordance with ACI 318-14 17.8.2.4, 26.7.1(h) and 26.13.3.2(c) or ACI 318-11 D.9.2.4, as applicable.

Under the IBC, additional requirements as set forth in Sections 1705, 1706 or 1707 must be observed, where applicable.

4.5 Compliance with NSF/ANSI Standard 61:

The PE1000+ Epoxy Adhesive Anchor System complies with the requirements of NSF/ANSI Standard 61, as referenced in Section 605 of the 2018, 2015, 2012, and 2009 *International Plumbing Code*® (IPC), and is certified for use as an anchoring adhesive for installing threaded rods less than or equal to 1.3 inches (33 mm) in diameter in concrete for water treatment applications.

5.0 CONDITIONS OF USE

The PE1000+ Epoxy Adhesive Anchor System described in this report complies with or is a suitable alternative to what is specified in, those codes listed in Section 1.0 of this report, subject to the following conditions:

5.1 PE1000+ epoxy adhesive anchors must be installed in accordance with this report and the manufacturer's printed installation instructions (MPII) as attached to each cartridge and reproduced in Figure 3 of this report.

5.2 The anchors described in this report must be installed in cracked or uncracked normal-weight or lightweight concrete having a specified compressive strength, f'_c = 2,500 psi to 8,500 psi (17.2 MPa to 58.6 MPa).

5.3 The values of f'_c used for calculation purposes must not exceed 8,000 psi (55.2 MPa). Anchors must be installed in concrete base materials in holes predrilled in accordance with the installation instructions provided in Figure 3 of this report.

5.4 Loads applied to the anchors must be adjusted in accordance with Section 1605.2 of the IBC for strength design and in accordance with Section 1605.3 of the IBC for allowable stress design.

5.5 PE1000+ epoxy adhesive anchors are recognized for use to resist short and long-term loads, including wind and earthquake, subject to the conditions of this report.

5.6 In structures assigned to Seismic Design Categories C, D, E, and F under the IBC or IRC, anchor strength must be adjusted in accordance with Section 4.1.11 of this report.

5.7 Strength design values must be established in accordance with Section 4.1 of this report.

5.8 Allowable stress design values must be established in accordance with Section 4.2 of this report.

5.9 Minimum anchor spacing and edge distance, as well as minimum member thickness, must comply with the values given in this report.

5.10 The maximum embedment is limited to 12 diameters for horizontal and upwardly inclined installations and for installations in water-filled (flooded) holes with a carbide drill bit.

5.11 Prior to anchor installation, calculations and details demonstrating compliance with this report must be submitted to the code official. The calculations and details must be prepared by a registered design professional where required by the statutes of the jurisdiction in which the project is to be constructed.

5.12 Anchors are not permitted to support fire-resistive construction. Where not otherwise prohibited by the code, PE1000+ epoxy adhesive anchors are permitted for installation in fire-resistive construction provided that at least one of the following conditions is fulfilled:

- Anchors are used to resist wind or seismic forces only.
- Anchors that support gravity load–bearing structural elements are within a fire-resistive envelope or a fire-resistive membrane, are protected by approved fire-resistive materials, or have been evaluated for resistance to fire exposure in accordance with recognized standards.
- Anchors are used to support non-structural elements.

5.13 Since an ICC-ES acceptance criteria for evaluating data to determine the performance of adhesive

anchors subjected to fatigue or shock loading is unavailable at this time, the use of these anchors under such conditions is beyond the scope of this report.

5.14 Use of zinc-plated carbon steel threaded rods or steel reinforcing bars is limited to dry, interior locations.

5.15 Use of hot-dipped galvanized carbon steel and stainless steel rods is permitted for exterior exposure or damp environments.

5.16 Steel anchoring materials in contact with preservative-treated wood and fire-retardant-treated wood must be of zinc-coated carbon steel or stainless steel. The minimum coating weights for zinc-coated steel must comply with ASTM A153.

5.17 Periodic special inspection must be provided in accordance with Section 4.3 of this report. Continuous special inspection for anchors installed in horizontal or upwardly inclined orientations to resist sustained tension loads must be provided in accordance with Section 4.3 of this report.

5.18 Installation of anchors in horizontal or upwardly inclined orientations to resist sustained tension loads must be performed by personnel certified by an applicable certification program in accordance with ACI 318-14 17.8.2.2 or 17.8.2.3 or ACI 318-11 D.9.2.2 or D.9.2.3, as applicable.

5.19 PE1000+ epoxy adhesive is manufactured under an approved quality control program with inspections by ICC-ES.

6.0 EVIDENCE SUBMITTED

Data in accordance with the ICC-ES Acceptance Criteria for Post-installed Adhesive Anchors in Concrete (AC308), dated October 2017 (editorially revised March 2018), which incorporates requirements in ACI 355.4-11 for use in cracked and uncracked concrete; including, but not limited to, tests under freeze/thaw conditions, tests under sustained load, tests for installation direction, tests at elevated temperatures, tests for resistance to alkalinity, tests for resistance to sulfur and tests for seismic tension and shear.

7.0 IDENTIFICATION

7.1 PE1000+ epoxy adhesive and additional listee product name described in Section 3.1 of this report are identified by packaging labeled with the lot number; expiration date; company name; and evaluation report number (ESR-2583). Threaded rods, nuts, washers and deformed reinforcing bars are standard steel anchor elements and must conform to applicable national specifications as set forth in Tables 2 and 3 of this report.

7.2 The report holder's contact information is the following:

DEWALT
701 EAST JOPPA ROAD
TOWSON, MARYLAND 21286
(800) 524-3244
www.DEWALT.com
anchors@DEWALT.com

7.3 The Additional Listee's contact information is the following:

POWERS FASTENERS
701 EAST JOPPA ROAD
TOWSON, MARYLAND 21286
(800) 524-3244
www.powers.com
engineering@powers.com

The DEWALT drilling systems shown below collect and remove dust with a HEPA dust extractor during the hole drilling operation in dry base materials using hammer-drills (see step 1 of the manufacturer's published installation instructions).

FIGURE A—EXAMPLES OF DEWALT DUST REMOVAL DRILLING SYSTEMS WITH HEPA DUST EXTRACTORS FOR ILLUSTRATION

TABLE 1—DESIGN USE AND TABLE INDEX FOR POST-INSTALLED ADHESIVE ANCHORS

DESIGN STRENGTH[1]			THREADED ROD (FRACTIONAL)[5]	DEFORMED REINFORCING BAR[6]
Steel	N_{sa}, V_{sa}		Table 4	Table 5
Concrete	N_{cb}, N_{cbg}, V_{cb}, V_{cbg}, V_{cp}, V_{cpg}		Table 6	Table 6
Bond[2]	N_a, N_{ag}	Hammer-drilled holes	Table 7	Table 7
		Diamond cored holes	Table 8	Table 8

CONCRETE TYPE	CONCRETE STATE	THREADED ROD DIAMETER (inch)	REINFORCING BAR SIZE (No.)	DRILLING METHOD	MINIMUM EMBEDMENT	MAXIMUM EMBEDMENT	SEISMIC DESIGN CATEGORIES[3]
Normal-weight and lightweight	Cracked	$^1/_2$, $^5/_8$, $^3/_4$, $^7/_8$, 1 and $1^1/_4$	4, 5, 6, 7, 8, 9, 10	Hammer-drill	See Table 7	See Table 7	A through F
	Uncracked	$^3/_8$, $^1/_2$, $^5/_8$, $^3/_4$, $^7/_8$, 1 and $1^1/_4$	3, 4, 5, 6, 7, 8, 9, 10	Hammer-drill	See Table 7	See Table 7	A and B
	Uncracked	$^1/_2$, $^5/_8$, $^3/_4$, $^7/_8$ and 1	4, 5, 6, 7, 8	Core drill	See Table 8	See Table 8	A and B

For **SI:** 1 inch = 25.4 mm. For **pound-inch** units: 1 mm = 0.03937 inch.

[1]Reference ACI 318-14 17.3.1.1 or ACI 318-11 D.4.1.1, as applicable. The controlling strength is decisive from all appropriate failure modes (i.e. steel, concrete, bond) and design assumptions.
[2]See Section 4.1.4 of this report for bond strength determination of post-installed adhesive anchors.
[3]See Section 4.1.11 for requirements for seismic design where applicable.
[4]Hammer-drill, i.e. rotary impact drills or rock drills with a carbide bit (including hollow bits).
[5]Anchors with $^1/_2$-, $^5/_8$-, $^3/_4$-, $^7/_8$- 1- and $1^1/_4$-inch-diameter (12.7, 15.9, 19.1, 22.2, 25.4 and 31.8 mm) threaded steel rods and No. 4 through No. 10 steel reinforcing bars may be installed in normal-weight concrete that is cracked or that may be expected to crack during the service life of the anchor when installed in hammer-drilled holes. Anchors with $^3/_8$-inch-diameter (9.5 mm) threaded steel rods and No. 3 steel reinforcing bars are limited to installation in uncracked concrete when installed in hammer-drilled holes.
[6]Anchors with $^1/_2$-, $^5/_8$-, $^3/_4$-, $^7/_8$- and 1-inch-diameter (12.7, 15.9, 19.1, 22.2 and 25.4 mm) threaded steel rods and No. 4 through No. 8 steel reinforcing bars are limited to installation in uncracked concrete when installed in core drilled holes.

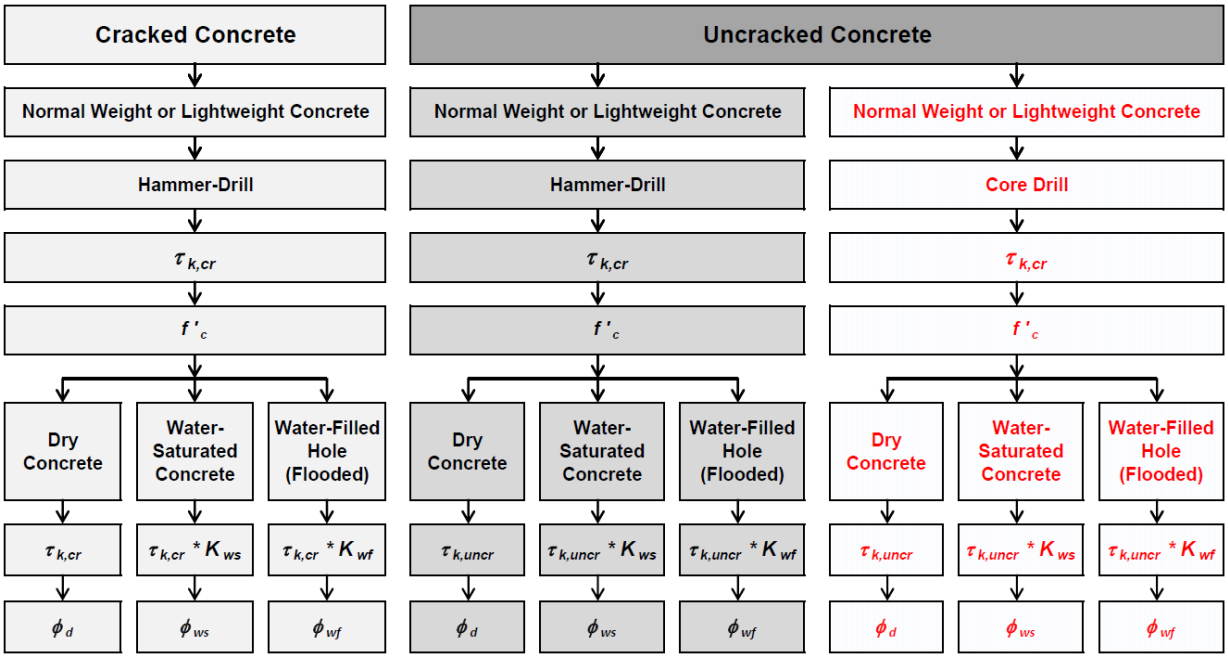


FIGURE 1—FLOW CHART FOR THE ESTABLISHMENT OF DESIGN BOND STRENGTH

TABLE 2—FRACTIONAL THREADED CARBON AND STAINLESS STEEL ROD MATERIALS[1]

THREADED ROD SPECIFICATION		UNITS	MIN. SPECIFIED ULTIMATE STRENGTH, f_{uta}	MIN. SPECIFIED YIELD STRENGTH 0.2 PERCENT OFFSET, f_{ya}	$\frac{f_{uta}}{f_{ya}}$	ELONGATION MINIMUM PERCENT[7]	REDUCTION OF AREA MINIMUM PERCENT	NUT SPECIFICATION[8]
Carbon Steel	ASTM A36[2] and F1554[3] Grade 36	psi (MPa)	58,000 (400)	36,000 (248)	1.61	23	40[9]	ASTM A194 / A563 Grade A
	ASTM F1554[3] Grade 55	psi (MPa)	75,000 (517)	55,000 (380)	1.36	23	40	
	ASTM F1554[3] Grade 105	psi (MPa)	125,000 (862)	105,000 (724)	1.19	15	45	ASTM A194 / A563 Grade DH
	ASTM A193[4] Grade B7	psi (MPa)	125,000 (860)	105,000 (720)	1.19	16	50	
Stainless Steel (Types 304 and 316)	ASTM F593[5] CW1 ($^3/_8$ to $^5/_8$ inch dia.)	psi (MPa)	100,000 (690)	65,000 (450)	1.54	20	-[10]	ASTM F594 Alloy Group 1, 2 or 3
	ASTM F593[5] CW2 ($^3/_4$ to $1^1/_4$ inch dia.	psi (MPa)	85,000 (590)	45,000 (310)	1.89	25	-[10]	
	ASTM A193[6] Grade B8/B8M, Class 1	psi (MPa)	75,000 (517)	30,000 (207)	2.50	30	50	ASTM F594 Alloy Group 1, 2 or 3
	ASTM A193[6] Grade B8/B8M2, Class 2B	psi (MPa)	95,000 (655)	75,000 (517)	1.27	25	40	

For **SI:** 1 inch = 25.4 mm, 1 psi = 0.006897 MPa. For **pound-inch** units: 1 mm = 0.03937 inch, 1 MPa = 145.0 psi.

[1]Adhesive must be used with continuously threaded carbon or stainless steels (all-thread) that have thread characteristics comparable with ANSI B1.1 UNC Coarse Thread Series. Tabulated values correspond to anchor diameters included in this report. See Section 3.2.4.3 of this report for ductility of steel anchor elements.
[2]*Standard Specification for Carbon Structural Steel.*
[3]*Standard Specification for Anchor Bolts, Steel, 36, 55, and 105-ksi Yield Strength.*
[4]*Standard Specification for Alloy-Steel and Stainless Steel Bolting Materials for High Temperature or High Pressure Service and Other Special Purpose Applications.*
[5]*Standard Specification for Stainless Steel Bolts, Hex Cap Screws, and Studs.*
[6]*Standard Standard Specification for Alloy-Steel and Stainless Steel Bolting for High Temperature or High Pressure Service and Other Special Purpose Applications.*
[7]Based on 2-inch (50 mm) gauge length except ASTM A193, which are based on a gauge length of 4d.
[8]Nuts of other grades and style having specified proof load stress greater than the specified grade and style are also suitable. Nuts must have specified proof load stresses equal to or greater than the minimum tensile strength of the specified threaded rod. Material types of the nuts and washers must be matched to the threaded rods.
[9]Minimum percent reduction of area reported in ASTM A36 is 50 percent.
[10]Minimum percent reduction of area not reported in the referenced ASTM standard.

TABLE 3—SPECIFICATIONS AND PHYSICAL PROPERTIES OF COMMON STEEL REINFORCING BARS[1]

REINFORCING SPECIFICATION	UNITS	MINIMUM SPECIFIED ULTIMATE STRENGTH, f_{uta}	MINIMUM SPECIFIED YIELD STRENGTH, f_{ya}
ASTM A615[2], A767[4], Grade 60	psi (MPa)	90,000 (620)	60,000 (414)
ASTM A706[3], A767[4], Grade 60	psi (MPa)	80,000 (550)	60,000 (414)
ASTM A615[2], A767[4], Grade 40	psi (MPa)	60,000 (415)	40,000 (275)

For **SI:** 1 psi = 0.006897 MPa. For **pound-inch** units: 1 MPa = 145.0 psi.

[1]Adhesive must be used with specified deformed reinforcing bars. Tabulated values correspond to bar sizes included in this report.
[2]*Standard Specification for Deformed and Plain Carbon-Steel Bars for Concrete Reinforcement.* Grade 40 and Grade 60 bars furnished to specification are considered ductile elements. In accordance with ACI 318-14 17.2.3.4.3(a)vi or ACI 318-11 D.3.3.4.3(a)6, as applicable, deformed reinforcing bars meeting this specification used as ductile steel elements to resist earthquake effects shall be limited to reinforcing bars satisfying the requirements of ACI 318-14 20.2.2.4 and 20.2.2.5 or ACI 318-11 21.1.5.2(a) and (b), as applicable. Grade 75 bars furnished to specification are considered brittle elements unless evidence is otherwise shown to the satisfaction of the registered design professional and code official in accordance with Section 3.2.4.3 of this report.
[3]*Standard Specification for Low-Alloy Steel Deformed and Plain Bars for Concrete Reinforcement.* Bars furnished to specification are considered ductile elements.
[4]*Standard Specification for Zinc-Coated (Galvanized) Steel Bars for Concrete Reinforcement.* Bars furnished to specification are considered brittle elements unless evidence is otherwise shown to the satisfaction of the registered design professional and code official in accordance with Section 3.2.4.3 of this report.

TABLE 4—STEEL DESIGN INFORMATION FOR FRACTIONAL THREADED ROD

DESIGN INFORMATION		SYMBOL	UNITS	NOMINAL ROD DIAMETER (inch)[1]						
				3/8	1/2	5/8	3/4	7/8	1	1 1/4
Threaded rod nominal outside diameter		d	inch (mm)	0.375 (9.5)	0.500 (12.7)	0.625 (15.9)	0.750 (19.1)	0.875 (22.2)	1.000 (25.4)	1.250 (31.8)
Threaded rod effective cross-sectional area		A_{se}	inch² (mm²)	0.0775 (50)	0.1419 (92)	0.2260 (146)	0.3345 (216)	0.4617 (298)	0.6057 (391)	0.9691 (625)
ASTM A36 and F1554, Grade 36	Nominal strength as governed by steel strength (for a single anchor)	N_{sa}	lbf (kN)	4,495 (20.0)	8,230 (36.6)	13,110 (58.3)	19,400 (86.3)	26,780 (119.1)	35,130 (156.3)	56,210 (250.0)
		V_{sa}	lbf (kN)	2,695 (12.0)	4,940 (22.0)	7,860 (35.0)	11,640 (51.8)	16,070 (71.4)	21,080 (93.8)	33,725 (150.0)
	Reduction factor for seismic shear	$\alpha_{V,seis}$	-	Not applicable	0.85	0.85	0.85	0.85	0.80	0.80
	Strength reduction factor for tension[2]	ϕ	-	0.75						
	Strength reduction factor for shear[2]	ϕ	-	0.65						
ASTM F1554, Grade 55	Nominal strength as governed by steel strength (for a single anchor)	N_{sa}	lbf (kN)	5,810 (25.9)	10,640 (47.3)	16,950 (75.4)	25,085 (111.6)	34,625 (154.0)	45,425 (202.0)	72,680 (323.3)
		V_{sa}	Lbf (kN)	3,485 (15.5)	6,385 (28.4)	10,170 (45.2)	15,050 (67.0)	20,775 (92.4)	27,255 (121.2)	43,610 (194.0)
	Reduction factor for seismic shear	$\alpha_{V,seis}$	-	Not applicable	0.85	0.85	0.85	0.85	0.80	0.80
	Strength reduction factor for tension[2]	ϕ	-	0.75						
	Strength reduction factor for shear[2]	ϕ	-	0.65						
ASTM A193 Grade B7 and F1554, Grade 105	Nominal strength as governed by steel strength (for a single anchor)	N_{sa}	Lbf (kN)	9,685 (43.1)	17,735 (78.9)	28,250 (125.7)	41,810 (186.0)	57,710 (256.7)	75,710 (336.8)	121,135 (538.8)
		V_{sa}	Lbf (kN)	5,815 (25.9)	10,640 (7.3)	16,950 (75.4)	25,085 (111.6)	34,625 (154.0)	45,425 (202.1)	72,680 (323.3)
	Reduction factor for seismic shear	$\alpha_{V,seis}$	-	Not applicable	0.85	0.85	0.85	0.85	0.80	0.80
	Strength reduction factor for tension[2]	ϕ	-	0.75						
	Strength reduction factor for shear[2]	ϕ	-	0.65						
ASTM F593 CW Stainless (Types 304 and 316)	Nominal strength as governed by steel strength (for a single anchor)	N_{sa}	Lbf (kN)	7,750 (34.5)	14,190 (63.1)	22,600 (100.5)	28,430 (126.5)	39,245 (174.6)	51,485 (229.0)	82,370 (366.4)
		V_{sa}	Lbf (kN)	4,650 (20.7)	8,515 (37.9)	13,560 (60.3)	17,060 (75.9)	23,545 (104.7)	30,890 (137.4)	49,425 (219.8)
	Reduction factor for seismic shear	$\alpha_{V,seis}$	-	Not applicable	0.85	0.85	0.85	0.85	0.80	0.80
	Strength reduction factor for tension[3]	ϕ	-	0.65						
	Strength reduction factor for shear[3]	ϕ	-	0.60						
ASTM A193 Grade B8/B8M, Class 1 Stainless (Types 304 and 316)	Nominal strength as governed by steel strength (for a single anchor)[4]	N_{sa}	Lbf (kN)	4,420 (19.7)	8,090 (36.0)	12,880 (57.3)	19,065 (84.8)	26,315 (117.1)	34,525 (153.6)	55,240 (245.7)
		V_{sa}	Lbf (kN)	2,650 (11.8)	4,855 (21.6)	7,730 (34.4)	11,440 (50.9)	15,790 (70.2)	20715 (92.1)	33,145 (147.4)
	Reduction factor for seismic shear	$\alpha_{V,seis}$	-	Not applicable	0.85	0.85	0.85	0.85	0.80	0.80
	Strength reduction factor for tension[2]	ϕ	-	0.75						
	Strength reduction factor for shear[2]	ϕ	-	0.65						
ASTM A193 Grade B8/B8M2, Class 2B Stainless (Types 304 and 316)	Nominal strength as governed by steel strength (for a single anchor)	N_{sa}	Lbf (kN)	7,365 (32.8)	13,480 (60.0)	21,470 (95.5)	31,775 (141.3)	43,860 (195.1)	57,545 (256.0)	92,065 (409.5)
		V_{sa}	Lbf (kN)	4,420 (19.7)	8,085 (36.0)	12,880 (57.3)	19,065 (84.8)	26,315 (117.1)	34,525 (153.6)	55,240 (245.7)
	Reduction factor for seismic shear	$\alpha_{V,seis}$	-	Not applicable	0.85	0.85	0.85	0.85	0.80	0.80
	Strength reduction factor for tension[2]	ϕ	-	0.75						
	Strength reduction factor for shear[2]	ϕ	-	0.65						

For **SI:** 1 inch = 25.4 mm, 1 lbf = 4.448 N. For **pound-inch** units: 1 mm = 0.03937 inches, 1 N = 0.2248 lbf.

[1]Values provided for fractional steel element material types are based on minimum specified strengths and calculated in accordance with ACI 318-14 Eq. 17.4.1.2 and Eq. 17.5.1.2b or ACI 318-11 Eq. D-2 and Eq. D-29, as applicable, except where noted. Nuts must be appropriate for the rod, as listed in Table 2 of this report.
[2]The tabulated value of ϕ applies when the load combinations of Section 1605.2 of the IBC, ACI 318-14 5.3 or ACI 318-11 9.2, as applicable are used in accordance with ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of ϕ must be determined in accordance with ACI 318-11 D.4.4. Values correspond to ductile steel elements.
[3]The tabulated value of ϕ applies when the load combinations of Section 1605.2 of the IBC, ACI 318-14 5.3 or ACI 318-11 9.2, as applicable are used in accordance with ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of ϕ must be determined in accordance with ACI 318-11 D.4.4. Values correspond to brittle steel elements.
[4]In accordance with ACI 318-14 26.12.3.1(a) and 26.11.1.2(c) or ACI 318-11 D.5.1.2 and D.6.1.2, as applicable the calculated values for nominal tension and shear strength for ASTM A193 Grade B8/B8M Class 1 stainless steel threaded rods are based on limiting the specified tensile strength of the anchor steel to $1.9f_y$ or 57,000 psi (393 MPa).

TABLE 5—STEEL DESIGN INFORMATION FOR REINFORCING BARS

DESIGN INFORMATION		SYMBOL	UNITS	NOMINAL REINFORCING BAR SIZE (REBAR)[1]							
				No. 3	No. 4	No. 5	No. 6	No. 7	No. 8	No. 9	No. 10
Rebar nominal outside diameter		d	inch (mm)	0.375 (9.5)	0.500 (12.7)	0.625 (15.9)	0.750 (19.1)	0.875 (22.2)	1.000 (25.4)	1.125 (28.7)	1.250 (32.3)
Rebar effective cross-sectional area		A_{se}	inch^2 (mm^2)	0.110 (71)	0.200 (129)	0.310 (200)	0.440 (284)	0.600 (387)	0.790 (510)	1.000 (645)	1.270 (819)
ASTM A615, Grade 75	Nominal strength as governed by steel strength (for a single anchor)	N_{sa}	lbf (kN)	11,000 (48.9)	20,000 (89.0)	31,000 (137.9)	44,000 (195.7)	60,000 (266.9)	79,000 (351.4)	100,000 (444.8)	127,000 (564.9)
		V_{sa}	lbf (kN)	6,600 (29.4)	12,000 (53.4)	18,600 (82.7)	26,400 (117.4)	36,000 (160.1)	47,400 (210.8)	60,000 (266.9)	76,200 (338.9)
	Reduction factor for seismic shear	$\alpha_{V,seis}$	-	Not applicable	0.70	0.70	0.70	0.70	0.70	0.70	0.70
	Strength reduction factor for tension[3]	ϕ	-	0.65							
	Strength reduction factor for shear[3]	ϕ	-	0.60							
ASTM A615, Grade 60	Nominal strength as governed by steel strength (for a single anchor)	N_{sa}	lbf (kN)	9,900 (44.0)	18,000 (80.1)	27,900 (124.1)	39,600 (176.1)	54,000 (240.2)	71,100 (316.3)	90,000 (400.3)	114,300 (508.4)
		V_{sa}	lbf (kN)	5,940 (26.4)	10,800 (48.0)	16,740 (74.5)	23,760 (105.7)	32,400 (144.1)	42,660 (189.8)	54,000 (240.2)	68,580 (305.0)
	Reduction factor for seismic shear	$\alpha_{V,seis}$	-	Not applicable	0.70	0.70	0.70	0.70	0.70	0.70	0.70
	Strength reduction factor for tension[2]	ϕ	-	0.75							
	Strength reduction factor for shear[2]	ϕ	-	0.65							
ASTM A706, Grade 60	Nominal strength as governed by steel strength (for a single anchor)	N_{sa}	lbf (kN)	8,800 (39.1)	16,000 (71.2)	24,800 (110.3)	35,200 (156.6)	48,000 (213.5)	63,200 (281.1)	80,000 (355.9)	101,600 (452.0)
		V_{sa}	lbf (kN)	5,280 (23.5)	9,600 (42.7)	14,880 (66.2)	21,120 (94.0)	28,800 (128.1)	37,920 (168.7)	48,000 (213.5)	60,960 (271.2)
	Reduction factor for seismic shear	$\alpha_{V,seis}$	-	Not applicable	0.70	0.70	0.70	0.70	0.70	0.70	0.70
	Strength reduction factor for tension[2]	ϕ	-	0.75							
	Strength reduction factor for shear[2]	ϕ	-	0.65							
ASTM A615, Grade 40	Nominal strength as governed by steel strength (for a single anchor)	N_{sa}	lbf (kN)	6,600 (29.4)	12,000 (53.4)	18,600 (82.7)	26,400 (117.4)	In accordance with ASTM A615, Grade 40 bars are furnished only in sizes No. 3 through No. 6			
		V_{sa}	lbf (kN)	3,960 (17.6)	7,200 (32.0)	11,160 (49.6)	15,840 (70.5)				
	Reduction factor for seismic shear	$\alpha_{V,seis}$	-	Not applicable	0.70	0.70	0.70				
	Strength reduction factor for tension[2]	ϕ	-	0.75							
	Strength reduction factor for shear[2]	ϕ	-	0.65							

For **SI:** 1 inch = 25.4 mm, 1 lbf = 4.448 N. For **pound-inch** units: 1 mm = 0.03937 inches, 1 N = 0.2248 lbf.

[1]Values provided for reinforcing bar material types based on minimum specified strengths and calculated in accordance with ACI 318-14 Eq. 17.4.1.2 and Eq. 17.5.1.2b or ACI 318-11 Eq. D-2 and Eq. D-29, as applicable.

[2]The tabulated value of ϕ applies when the load combinations of Section 1605.2 of the IBC, ACI 318-14 5.3 or ACI 318-11 Section 9.2 are used in accordance with ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of ϕ must be determined in accordance with ACI 318 D.4.4. Values correspond to ductile steel elements. In accordance with ACI 318-14 17.2.3.4.3(a)6 or ACI 318-11 D.3.3.4.3 (a) 6, as applicable, deformed reinforcing bars meeting this specification used as ductile steel elements to resist earthquake effects shall be limited to reinforcing bars bars satisfying the requirements of ACI 318-14 20.2.2.4 and 20.2.2.5 or ACI 318-11 21.1.5.2 (a) and (b) as applicable.

[3]The tabulated value of ϕ applies when the load combinations of Section 1605.2 of the IBC, ACI 318-14 5.3 or ACI 318-11 9.2 are used in accordance with ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of ϕ must be determined in accordance with ACI 318-11 D.4.4. Values correspond to brittle steel elements.

TABLE 6—CONCRETE BREAKOUT AND PRYOUT DESIGN INFORMATION FOR FRACTIONAL THREADED ROD AND REINFORCING BARS IN HOLES DRILLED WITH A HAMMER DRILL AND CARBIDE BIT OR A CORE DRILL AND DIAMOND CORE BIT[1]

DESIGN INFORMATION	SYMBOL	UNITS	NOMINAL ROD DIAMETER (inch) / REINFORCING BAR SIZE							
			$^3/_8$ or #3	$^1/_2$ or #4	$^5/_8$ or #5	$^3/_4$ or #6	$^7/_8$ or #7	1 or #8	#9	$1^1/_4$ or #10
Effectiveness factor for cracked concrete	$k_{c,cr}$	- (SI)	Not Applicable	17 (7.1)						
Effectiveness factor for uncracked concrete	$k_{c,uncr}$	- (SI)	24 (10.0)							
Minimum embedment	$h_{ef,min}$	inch (mm)	$2^3/_8$ (60)	$2^3/_4$ (70)	$3^1/_8$ (79)	$3^1/_2$ (89)	$3^1/_2$ (89)	4 (102)	$4^1/_2$ (114)	5 (127)
Maximum embedment	$h_{ef,max}$	inch (mm)	$4^1/_2$ (114)	10 (254)	$12^1/_2$ (318)	15 (381)	$17^1/_2$ (445)	20 (508)	$22^1/_2$ (572)	25 (635)
Minimum anchor spacing	s_{min}	inch (mm)	$1^7/_8$ (48)	$2^1/_2$ (64)	$3^1/_8$ (79)	$3^3/_4$ (95)	$4^3/_8$ (111)	5 (127)	$5^5/_8$ (143)	$6^1/_4$ (159)
Minimum edge distance	c_{min}	inch (mm)	5d where d is nominal outside diameter of the anchor; see Section 4.1.9 of this report for design with reduced minimum edge distances (with reduced torque)							
Minimum member thickness	h_{min}	inch (mm)	$h_{ef} + 1^1/_4$ (h_{ef} + 30)		$h_{ef} + 2d_o$ where d_o is hole diameter; for installation parameters see Table 9 of this report					
Critical edge distance—splitting (for uncracked concrete only)	c_{ac}	inch (mm)	See Section 4.1.10 of this report							
Strength reduction factor for tension, concrete failure modes, Condition B[2]	ϕ	-	0.65							
Strength reduction factor for shear, concrete failure modes, Condition B[2]	ϕ	-	0.70							

For **SI:** 1 inch = 25.4 mm, 1 lbf = 4.448 N. For **pound-inch** units: 1 mm = 0.03937 inch, 1 N = 0.2248 lbf.

[1]Additional setting information is described in the installation instructions, Figure 3 of this report.
[2]Condition A requires supplemental reinforcement, while Condition B applies where supplemental reinforcement is not provided or where pryout governs, as set forth in ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable. The tabulated value of ϕ applies when the load combinations of Section 1605.2 of the IBC, ACI 318-14 5.3 or ACI 318-11 9.2 are used in accordance with ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of ϕ must be determined in accordance with ACI 318-11 D.4.4.

TABLE 7—BOND STRENGTH DESIGN INFORMATION FOR FRACTIONAL THREADED RODS AND REINFORCING BARS IN HOLES DRILLED WITH A HAMMER DRILL AND CARBIDE BIT[1]

DESIGN INFORMATION		SYMBOL	UNITS	NOMINAL ROD DIAMETER (inch) / REINFORCING BAR SIZE							
				3/8 or #3	1/2 or #4	5/8 or #5	3/4 or #6	7/8 or #7	1 or #8	#9	1 1/4 or #10
Minimum embedment		$h_{ef,min}$	inch (mm)	2 3/8 (60)	2 3/4 (70)	3 1/8 (79)	3 1/2 (89)	3 1/2 (89)	4 (102)	4 1/2 (114)	5 (127)
Maximum embedment	Dry concrete and saturated concrete[7]	$h_{ef,max}$	inch (mm)	4 1/2 (114)	10 (254)	12 1/2 (318)	15 (381)	17 1/2 (445)	20 (508)	22 1/2 (572)	25 (635)
	Water-filled hole (flooded)	$h_{ef,max}$	inch (mm)	4 1/2 (114)	6 (152)	7 1/2 (190)	9 (225)	10 1/2 (267)	12 (305)	13 1/2 (343)	15 (381)
110°F (43°C) Maximum long-term service temperature; 140°F (60°C) maximum short-term service temperature[2,4]	Characteristic bond strength in cracked concrete[5,8]	$\tau_{k,cr}$	psi (N/mm^2)	Not applicable	576 (4.0)	474 (3.3)	441 (3.0)	416 (2.9)	416 (2.9)	416 (2.9)	416 (2.9)
	Characteristic bond strength in cracked concrete, short-term loads only[8]	$\tau_{k,cr}$	psi (N/mm^2)	Not applicable	979 (6.8)	806 (5.6)	750 (5.2)	707 (4.9)	707 (4.9)	707 (4.9)	707 (4.9)
	Characteristic bond strength in uncracked concrete[5,9]	$\tau_{k,uncr}$	psi (N/mm^2)	1,223 (8.4)	1,156 (8.0)	1,106 (7.6)	1,067 (7.4)	1,036 (7.1)	1,010 (7.0)	986 (6.8)	966 (6.7)
	Characteristic bond strength in uncracked concrete, short-term loads only[9]	$\tau_{k,uncr}$	psi (N/mm^2)	2,079 (14.3)	1,965 (13.5)	1,880 (13.0)	1,814 (12.5)	1,761 (12.1)	1,717 (11.8)	1,676 (11.6)	1,642 (11.3)
110°F (43°C) Maximum long-term service temperature; 176°F (80°C) maximum short-term service temperature[3,4]	Characteristic bond strength in cracked concrete[5,8]	$\tau_{k,cr}$	psi (N/mm^2)	Not applicable	455 (3.1)	374 (2.6)	349 (2.4)	329 (2.3)	329 (2.3)	329 (2.3)	329 (2.3)
	Characteristic bond strength in cracked concrete, short-term loads only[8]	$\tau_{k,cr}$	psi (N/mm^2)	Not applicable	757 (5.2)	636 (4.4)	593 (4.1)	559 (3.9)	559 (3.9)	559 (3.9)	559 (3.9)
	Characteristic bond strength in uncracked concrete[5,9]	$\tau_{k,uncr}$	psi (N/mm^2)	966 (6.7)	913 (6.3)	874 (6.0)	843 (5.8)	819 (5.6)	798 (5.5)	779 (5.4)	763 (5.3)
	Characteristic bond strength in uncracked concrete, short-term loads only[9]	$\tau_{k,uncr}$	psi (N/mm^2)	1,642 (11.3)	1,552 (10.7)	1,486 (10.2)	1,433 (9.9)	1,392 (9.6)	1,357 (9.4)	1,324 (9.1)	1,297 (8.9)
Permissible installation conditions[6]	Dry concrete	ϕ_d	-	0.65	0.65	0.65	0.65	0.65	0.65	0.65	0.65
	Water-saturated concrete	ϕ_{ws}	-	0.45	0.45	0.45	0.45	0.45	0.45	0.45	0.45
		κ_{ws}	-	0.93	0.9	0.96	1.0	1.0	1.0	1.0	0.99
	Water-filled hole (flooded)	ϕ_{wf}	-	0.45	0.45	0.45	0.45	0.45	0.45	0.45	0.45
		κ_{wf}	-	0.93	0.83	0.75	0.70	0.65	0.62	0.59	0.56
Reduction factor for seismic tension		$\alpha_{N,seis}$	-	1.0							

For **SI:** 1 inch = 25.4 mm, 1 psi = 0.006894 MPa. For **pound-inch** units: 1 mm = 0.03937 inch, 1 MPa = 145.0 psi.

[1]Bond strength values correspond to concrete compressive strength f'_c = 2,500 psi. For concrete compressive strength, f'_c between 2,500 psi and 8,000 psi, the tabulated characteristic bond strength may be increased by a factor of $(f'_c / 2{,}500)^{0.12}$ [For **SI:** $(f'_c / 17.2)^{0.12}$]. See Section 4.1.5 of this report.
[2]The maximum short- term service temperature may be increased to 162°F (72°C) provided characteristic bond strengths are reduced by 10 percent. Long-term and short-term temperatures meet the requirements of Section 8.5 of ACI 355.4 and Table 8.1, Temperature Category B.
[3]Long-term and short-term temperatures meet the requirements of Section 8.5 of ACI 355.4 and Table 8.1, Temperature Category A.
[4]Short-term elevated concrete temperatures are those that occur over brief intervals, e.g. as a result of diurnal cycling. Long-term concrete temperatures are roughly constant over significant periods of time.
[5]Characteristic bond strengths are for sustained loads including dead and live loads.
[6]Permissible installation conditions include dry concrete, water-saturated concrete and water-filled holes. Water-filled holes include applications in dry or water-saturated concrete where the drilled holes contain standing water at the time of anchor installation. For installation instructions see Figure 3 of this report.
[7]Maximum embedment is limited to twelve anchor diameters for horizontal and upwardly inclined installations.
[8]For structures assigned to Seismic Design Categories C, D, E or F, bond strength values for cracked concrete do not require an additional reduction factor applied ($\alpha_{N,seis}$ = 1.0) where seismic design is applicable. See Section 4.1.11 of this report for requirements for seismic design.
[9]Bond strength values for uncracked concrete are applicable for structures assigned to Seismic Design Categories A and B only.

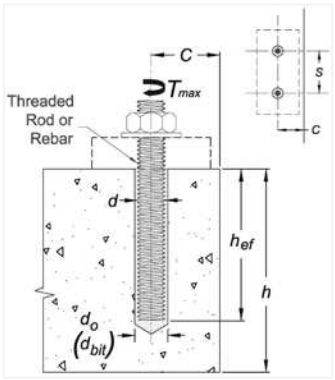
TABLE 8—BOND STRENGTH DESIGN INFORMATION FOR FRACTIONAL THREADED RODS AND REINFORCING BARS IN HOLES DRILLED WITH A CORE DRILL AND DIAMOND CORE BIT[1]

DESIGN INFORMATION		SYMBOL	UNITS	NOMINAL ROD DIAMETER (inch) / REINFORCING BAR SIZE				
				1/2 or #4	5/8 or #5	3/4 or #6	7/8 or #7	1 or #8
Minimum embedment		$h_{ef,min}$	inch (mm)	2 3/4 (70)	3 1/8 (79)	3 1/2 (89)	3 1/2 (89)	4 (102)
Maximum embedment[7]		$h_{ef,max}$	inch (mm)	10 (254)	12 1/2 (318)	15 (381)	17 1/2 (445)	20 (508)
110°F (43°C) Maximum long-term service temperature;	Characteristic bond strength in uncracked concrete[5,8]	$\tau_{k,uncr}$	psi (N/mm^2)	1,133 (7.8)	1,075 (7.4)	1,033 (7.1)	1,002 (6.9)	975 (6.7)
140°F (60°C) maximum short-term service temperature[2,4]	Characteristic bond strength in uncracked concrete, short-term loads only[8]	$\tau_{k,uncr}$	psi (N/mm^2)	1,892 (13.0)	1,795 (12.4)	1,725 (11.9)	1,673 (11.5)	1,628 (11.2)
110°F (43°C) Maximum long-term service temperature;	Characteristic bond strength in uncracked concrete[5,8]	$\tau_{k,uncr}$	psi (N/mm^2)	895 (6.2)	849 (5.9)	816 (5.6)	791 (5.5)	770 (5.3)
176°F (80°C) maximum short-term service temperature[3,4]	Characteristic bond strength in uncracked concrete, short-term loads only[8]	$\tau_{k,uncr}$	psi (N/mm^2)	1,495 (10.3)	1,418 (9.8)	1,363 (9.4)	1,321 (9.1)	1,286 (8.9)
Permissible installation conditions[6]	Dry concrete	ϕ_d	-	0.55	0.45	0.45	0.45	0.45
	Water-saturated concrete	ϕ_{ws}	-	0.45	0.45	0.45	0.45	0.45
		κ_{ws}	-	1.0	1.0	1.0	1.0	1.0
	Water-filled hole (flooded)	ϕ_{wf}	-	0.45	0.45	0.45	0.45	0.45
		κ_{wf}	-	0.94	0.95	0.95	0.95	0.96

For **SI:** 1 inch = 25.4 mm, 1 psi = 0.006894 MPa. For **pound-inch** units: 1 mm = 0.03937 inch, 1 MPa = 145.0 psi.

[1]Bond strength values correspond to concrete compressive strength f'_c = 2,500 psi. For concrete compressive strength, f'_c between 2,500 psi and 8,000 psi, the tabulated characteristic bond strength may be increased by a factor of $(f'_c / 2{,}500)^{0.12}$ [For **SI:** $(f'_c / 17.2)^{0.12}$]. See Section 4.1.5 of this report.
[2]The maximum short- term service temperature may be increased to 162°F (72°C) for this temperature category provided characteristic bond strengths are reduced by 10 percent. Long-term and short-term temperatures meet the requirements of Section 8.5 of ACI 355.4 and Table 8.1, Temperature Category B.
[3]Long-term and short-term temperatures meet the requirements of Section 8.5 of ACI 355.4 and Table 8.1, Temperature Category A.
[4]Short-term elevated concrete temperatures are those that occur over brief intervals, e.g. as a result of diurnal cycling. Long-term concrete temperatures are roughly constant over significant periods of time.
[5]Characteristic bond strengths are for sustained loads including dead and live loads.
[6]Permissible installation conditions include dry concrete, water-saturated concrete and water-filled holes. Water-filled holes include applications in dry or water-saturated concrete where the drilled holes contain standing water at the time of anchor installation. For installation instructions see Figure 3 of this report.
[7]Maximum embedment for dry concrete and saturated concrete is limited to twelve anchor diameters for horizontal and upwardly inclined installations.
[8]Bond strength values for uncracked concrete are applicable for structures assigned to Seismic Design Categories A and B only.

TABLE 9—INSTALLATION PARAMETERS FOR FRACTIONAL THREADED ROD AND REINFORCING BARS



PARAMETER	SYMBOL	UNITS	NOMINAL ROD DIAMETER (inch) / REINFORCING BAR SIZE									
			3/8 or #3	1/2	#4	5/8 or #5	3/4 or #6	7/8 or #7	1 or #8	#9	1 1/4	#10
Threaded rod outside diameter	d	inch (mm)	0.375 (9.5)	0.500 (12.7)	N/A[1]	0.625 (15.9)	0.750 (19.1)	0.875 (22.2)	1.000 (25.4)	N/A[1]	1.250 (31.8)	N/A[1]
Rebar nominal outside diameter	d	inch (mm)	0.375 (9.5)	N/A[1]	0.500 (12.7)	0.625 (15.9)	0.750 (19.1)	0.875 (22.2)	1.000 (25.4)	1.125 (28.7)	N/A[1]	1.250 (31.8)
Carbide drill bit nominal size	d_o (d_{bit})	inch	7/16	9/16	5/8	11/16 or 3/4	7/8	1	1 1/8	1 3/8	1 3/8	1 1/2
Diamond core bit nominal size	d_o (d_{bit})	inch	N/A[1]	5/8		3/4	7/8	1	1 1/8	N/A[1]	N/A[1]	N/A[1]
Minimum embedment	$h_{ef,min}$	inch (mm)	2 3/8 (60)	2 3/4 (70)		3 1/8 (79)	3 1/2 (89)	3 1/2 (89)	4 (102)	4 1/2 (114)	5 (127)	5 (127)
Maximum embedment[2]	$h_{ef,max}$	inch (mm)	4 1/2 (114)	10 (254)		12 1/2 (318)	15 (381)	17 1/2 (445)	20 (508)	22 1/2 (572)	25 (635)	25 (635)
Max. torque	T_{max}	ft-lbs	15	33		60	105	125	165	200	280	280
Max. torque[3] (A36/Grade 36 rod)	T_{max}	ft-lbs	10	25		50	90	125	165	N/A[1]	280	N/A[1]
Max. torque[4] (Class 1 SS rod)	T_{max}	ft-lbs	5	20		40	60	100	165	N/A[1]	280	N/A[1]
Minimum anchor spacing	s_{min}	inch (mm)	1 7/8 (48)	2 1/2 (64)		3 1/8 (79)	3 3/4 (95)	4 3/8 (111)	5 (127)	5 5/8 (143)	6 1/4 (159)	6 1/4 (159)
Minimum edge distance	c_{min}	inch (mm)	5d where d is nominal outside diameter of the anchor; see Section 4.1.9 of this report for design with reduced minimum edge distances (with reduced torque):									
			1 3/4 (45)	1 3/4 (45)		1 3/4 (45)	1 3/4 (45)	1 3/4 (45)	1 3/4 (45)	2 3/4 (70)	2 3/4 (70)	2 3/4 (70)
Minimum member thickness	h_{min}	inch (mm)	h_{ef} + 1 1/4 (h_{ef} + 30)			h_{ef} + 2d_o						

For **SI:** 1 inch = 25.4 mm, 1 ft-lbf = 1.356 N-m. For **pound-inch** units: 1 mm = 0.03937 inch, 1 N-m = 0.7375 ft-lbf.

[1]N/A = Not Applicable.
[2]The maximum embedment is limited to 12 diameters for horizontal and upwardly inclined installations and for installations in water-filled (flooded) holes with a carbide drill bit.
[3]These values apply to ASTM A36 / F1554 Grade 36 carbon steel threaded rods only.
[4]These values apply to ASTM A193 Grade B8/B8M (Class 1) stainless steel threaded rods only.

TABLE 10—EXAMPLE OF PE1000+ EPOXY ADHESIVE ANCHOR ALLOWABLE STRESS DESIGN (ASD) VALUES FOR ILLUSTRATIVE PURPOSES[1,2,3,4,6,9,10,13,14,16]

NOMINAL ANCHOR ROD DIAMETER OR REBAR SIZE d (inch) / (No.)	EFFECTIVE EMBED.[5] h_{ef} (inches)	CONCRETE STRENGTH[12] f'_c (psi)	EFFECTIVE-NESS FACTOR FOR UNCRACKED CONCRETE k_{uncr}	CHARACTERISTIC BOND STRENGTH $\tau_{k,uncr}$ (psi)		NOMINAL STRENGTH IN TENSION N_n (pounds)		STRENGTH REDUCTON FACTOR ϕ[15]		ALLOWABLE TENSION LOAD[11] $\phi N_n / \alpha$ (pounds)	
				110°F LT, 140°F ST[7]	110°F LT, 176°F ST[8]	110°F LT, 140°F ST[7]	110°F LT, 176°F ST[8]	110°F LT, 140°F ST[7]	110°F LT, 176°F ST[8]	110°F LT, 140°F ST[7]	110°F LT, 176°F ST[8]
ASTM A193 Grade B7 Threaded Rod											
3/8	2 3/8	2,500	24	1,223	966	3,422	2,703	0.65 (bond)	0.65 (bond)	1,505	1,185
	4 1/2	2,500	24	1,223	966	6,484	5,121	0.65 (bond)	0.65 (bond)	2,850	2,250
1/2	2 3/4	2,500	24	1,156	913	4,994	3,944	0.65 (bond)	0.65 (bond)	2,195	1,735
	10	2,500	24	1,156	913	18,158	14,341	0.65 (bond)	0.65 (bond)	7,975	6,295
5/8	3 1/8	2,500	24	1,106	874	6,629	5,363	0.65 (conc)	0.65 (bond)	2,910	2,355
	12 1/2	2,500	24	1,106	874	27,145	21,451	0.65 (bond)	0.65 (bond)	11,920	9,420
3/4	3 1/2	2,500	24	1,067	843	7,857	6,952	0.65 (conc)	0.65 (bond)	3,450	3,055
	15	2,500	24	1,067	843	37,711	29,794	0.65 (bond)	0.65 (bond)	16,560	13,085
7/8	3 1/2	2,500	24	1,036	819	7,857	7,857	0.65 (conc)	0.65 (conc)	3,450	3,450
	17 1/2	2,500	24	1,036	819	49,837	39,399	0.65 (bond)	0.65 (bond)	21,890	17,305
1	4	2,500	24	1,010	798	9,600	9,600	0.65 (conc)	0.65 (conc)	4,215	4,215
	20	2,500	24	1,010	798	78,408	50,140	0.65 (bond)	0.65 (bond)	27,870	22,020
1 1/4	5	2,500	24	966	763	13,416	13,416	0.65 (conc)	0.65 (conc)	5,890	5,890
	25	2,500	24	966	763	94,837	74,907	0.65 (bond)	0.65 (bond)	41,650	32,900
ASTM A706 Grade 60 Reinforcing Bar											
3	2 3/8	2,500	24	1,223	966	3,428	2,703	0.65 (bond)	0.65 (bond)	1,505	1,185
	4 1/2	2,500	24	1,223	966	6,484	5,121	0.65 (bond)	0.65 (bond)	2,850	2,250
4	2 3/4	2,500	24	1,156	913	4,972	3,944	0.65 (bond)	0.65 (bond)	2,195	1,735
	10	2,500	24	1,156	913	18,158	14,341	0.65 (bond)	0.65 (bond)	7,975	6,295
5	3 1/8	2,500	24	1,106	874	6,629	5,363	0.65 (conc)	0.65 (bond)	2,910	2,355
	12 1/2	2,500	24	1,106	874	27,145	21,451	0.65 (bond)	0.65 (bond)	11,920	9,420
6	3 1/2	2,500	24	1,067	843	7,857	6,952	0.65 (conc)	0.65 (bond)	3,450	3,055
	15	2,500	24	1,067	843	37,711	29,794	0.65 (bond)	0.65 (bond)	16,560	13,085
7	3 1/2	2,500	24	1,036	819	7,857	7,857	0.65 (conc)	0.65 (conc)	3,450	3,450
	17 1/2	2,500	24	1,036	819	49,837	39,399	0.65 (bond)	0.65 (bond)	21,890	17,305
8	4	2,500	24	1,010	798	9,600	9,600	0.65 (conc)	0.65 (conc)	4,215	4,215
	20	2,500	24	1,010	798	78,408	50,140	0.65 (bond)	0.65 (bond)	27,870	22,020
9	4 1/2	2,500	24	966	779	11,455	11,455	0.65 (conc)	0.65 (conc)	5,030	5,030
	22 1/2	2,500	24	966	779	78,408	61,947	0.65 (bond)	0.65 (bond)	41,650	27,205
10	5	2,500	24	966	763	13,416	13,416	0.65 (conc)	0.65 (conc)	5,890	5,890
	25	2,500	24	966	763	94,837	74,907	0.65 (bond)	0.65 (bond)	41,650	32,900

For **SI:** 1 inch = 25.4 mm, 1 lbf = 4.448 N, 1 psi = 0.006894 MPa. For **pound-inch** units: 1 mm = 0.03937 inch, 1 N = 0.2248 lbf, 1 MPa = 145.0 psi.

[1]Single anchor with static tension load only; ASTM A193 Grade B7 threaded rod and ASTM A706 Grade 60 reinforcing bar.
[2]Vertical downward installation direction.
[3]Special inspection interval = Periodic.
[4]Installation temperature = 41°F (5°C) to 104°F (40°C) for base material; 41°F (5°C) to 95°F (35°C) for cartridge adhesive.
[5]Embedment = $h_{ef,min}$ and $h_{ef,max}$ for each diameter.
[6]Concrete determined to remain uncracked for the life of the anchorage.
[7]Long-term service temperature = 110°F (43°C), short-term service temperature = 140°F (60°C).
[8]Long-term service temperature = 110°F (43°C), short-term service temperature = 176°F (80°C).
[9]Load combinations are based on ACI 318-14 5.3 or ACI 318-11 9.2, as applicable with no seismic loading considered.
[10]Thirty percent (30%) dead load and seventy percent (70%) live load; controlling load combination 1.2D + 1.6L.
[11]Calculation of weighted average for the conversion factor, α = 1.2(0.3) + 1.6(0.7) = 1.48.
[12]f'_c = 2,500 psi compressive strength (normal-weight concrete).
[13]$c_{a1} = c_{a2} \geq c_{ac}$.
[14]$h \geq h_{min}$.
[15]Strength reduction factor from controlling nominal strength in tension [i.e. steel, concrete (conc), bond] decisive from design assumptions.
[16]Hammer-drilled holes in dry concrete.

FIGURE 2—PE1000+ EPOXY ADHESIVE ANCHOR SYSTEM INCLUDING TYPICAL STEEL ANCHOR ELEMENTS

PE1000+
Instruction Card

DESCRIPTION:
PE1000+ is an easy dispensing, high strength, 100% solids epoxy anchoring adhesive which is formulated for use in anchoring applications by trained professionals. Please refer to manufacturer's published installation instructions and SDS for additional detailed information.

PRECAUTION:
Safety glasses and dust masks should be used when drilling holes into concrete, stone and masonry. Wear gloves and safety glasses when handling and dispensing adhesive. Do not sand the adhesive and create silica dust which could be inhaled. Avoid skin and eye contact. use a NIOSH-approved chemical mask to avoid respiratory discomfort if working indoors or in a confined area, or if sensitive to adhesive odors. Wash hands or other affected body parts with soap and water if skin contact occurs. Flush eyes with plenty of water and seek immediate medical attention if eye contact occurs. Move to fresh air if adhesive odor begins to cause discomfort.

IMPORTANT! Before using, read and review Safety Data Sheet (SDS).
This product contains crystalline silica and as supplied does not pose a dust hazard. IARC classifies crystalline silica (quartz sand) as a Group I carcinogen based upon evidence among workers in industries where there has been long-term and chronic exposure (via inhalation) to silica dust; e.g. mining, quarry, stone crushing, refractory brick and pottery workers. This product does not pose a dust hazard; therefore, this classification is not relevant. However, if reacted (fully cured) product is further processed (e.g. sanded, drilled) be sure to wear proper respiratory and eye protection to avoid health risk.

HANDLING AND STORAGE:
Store in a cool, dry, well ventilated area at temperatures between 32°F (0°C) and 95°F (35°C). Do not freeze. Keep away from excessive heat and flame. Keep partially used containers closed when not in use. Protect from damage. Store away from heat and light.

Note expiration date on product label before use. Do not use expired product. Cartridge temperature must be between 41°F - 95°F (5°C - 35°C) when in use except as noted. Partially used cartridges may be stored with hardened adhesive in the attached mixing nozzle. **Note:** if the cartridge is reused, attach a new mixing nozzle and discard the initial quantity of the anchor adhesive as described in the setting instructions (steps #3 and #5).

DEWALT
701 East Joppa Road
Towson, MD 21286 U.S.A.
anchors@DEWALT.com
www.DEWALT.com
P: (800) 524-3244 **[N]**

[V.] Adhesive piston plugs

Threaded rod diameter (inch)	Rebar size (no.)	Drill bit diameter (inch)	Plug Size (inch)	Plastic Plug (Cat. #)	Horizontal and overhead installations[1,2]
5/8	#5	11/16	11/16	08258	
		3/4	3/4	08259	
3/4	#6	7/8	7/8	08300	
7/8	#7	1	1	08301	
1	#8	1 1/8	1 1/8	08303	
1 1/4	#9	1 3/8	1 3/8	08305	
-	#10	1 1/2	1 1/2	08309	

[1]A plastic extension tube (Cat# 08281) or equivalent approved by DEWALT must be used with piston plugs.

[2]All listed overhead anchor installations require piston plugs; horizontal installations with embedments greater than 8 inches require piston plugs.

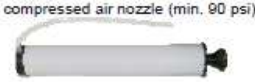
[I.] Hole cleaning tools - wire brushes and air blowers

Threaded rod diameter (inch)	Rebar size (No.)	Drill bit size[1] (inch)	Core bit diameter (inch)	Brush length (inches)	Steel wire brush (Cat. #)
3/8	#3	7/16	7/16	6 3/4	08284
1/2	-	9/16	9/16	6 3/4	08285
-	#4	5/8	5/8	6 3/4	08275
5/8	#5	11/16	11/16	7 7/8	08286
		3/4	3/4	7 7/8	08278
3/4	#6	7/8	7/8	7 7/8	08287
7/8	#7	1	1	11 7/8	08288
1	#8	1 1/8	1 1/8	11 7/8	08289
		1 1/4	1 1/4	11 7/8	08274
1 1/4	#9	1 3/8	1 3/8	11 7/8	08290
-	#10	1 1/2	1 1/2	11 7/8	08291

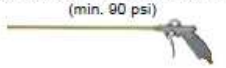
A brush extension (Cat. #08282) must be used with a steel wire brush for holes drilled deeper than the listed brush length.

Air blowers

Hand pump (volume 25 fl. oz.), Cat #8280 or compressed air nozzle (min. 90 psi)



Compressed air nozzle only, Cat #8292 (min. 90 psi)



[1]For installations with 5/8-inch threaded rod and #5 rebar size, the preferred ANSI drill bit diameter is 3/4-inch. If an 11/16-inch ANSI drill bit is used the user must check before injecting the adhesive to verify that the steel anchor element can be inserted into the cleaned hole without resistance.

[II.] Gel (working) times and curing times

Temperature of base material		Gel (working) time	Full curing time
41°F	5°C	180 minutes	50 hours
50°F	10°C	120 minutes	30 hours
68°F	20°C	30 minutes	10 hours
86°F	30°C	20 minutes	6 hours
95°F	35°C	15 minutes	5 hours
104°F	40°C	12 minutes	4 hours

Linear interpolation for intermediate base material temperatures is possible.

[III.] Installation parameters - Specifications for installation of threaded rods and reinforcing bars

Anchor property / Setting information	Threaded rod (inch) / reinforcing bar size (rebar)									
	3/8 or #3	1/2	#4	5/8 or #5	3/4 or #6	7/8 or #7	1 or #8	#9	1 1/4	#10
d = Threaded rod outside diameter (in.)	0.375	0.500	-	0.625	0.750	0.875	1.000	-	1.250	-
d = Nominal rebar diameter (in.)	0.375	-	0.500	0.625	0.750	0.875	1.000	1.125	-	1.250
d_o (d_{bit}) = Nominal ANSI drill bit size (in.)	7/16	9/16	5/8	11/16 or 3/4	7/8	1	1 1/8	1 3/8	1 3/8	1 1/2
d_o (d_{bit}) = Nominal diamond core bit size (in.)	See Table I									
$h_{ef,min}$ = Minimum embedment (inches)	2 3/8	2 3/4		3 1/8	3 1/2	3 1/2	4	4 1/2	5	5
$h_{ef,max}$ = Maximum embedment (inches)	4 1/2	10		12 1/2	15	17 1/2	20	22 1/2	25	25
s_{min} = Minimum spacing (inches)	1 7/8	2 1/2		3 1/8	3 3/4	4 3/8	5	5 5/8	6 1/4	6 1/4
c_{min} = Minimum edge distance (inches)	1 3/4	1 3/4		1 3/4	1 3/4	1 3/4	1 3/4	2 3/4	2 3/4	2 3/4
h_{min} = Minimum member thickness (inches)	h_{ef} + 1 1/4			h_{ef} + $2d_o$						
T_{max} = Maximum torque (ft.-lb.) after full curing	15	33		60	105	125	165	165	280	280
$T_{max,A36}$ = Maximum torque (ft.-lb.) for A36/Grade 36 rod	10	25		50	90	125	165	-	280	-
$T_{max,C1}$ = Maximum torque (ft.-lb.) for Grade B8/B8M Class 1 rod	5	20		40	60	100	165	-	280	-

For installations between the minimum edge distance and 5d, the tabulated maximum torque must be reduced (multiplied) by a factor of 0.45.
The maximum embedment is 12d for installations in water-filled (flooded) holes drilled with a carbide drill bit and all horizontal and vertically inclined installations.

[IV.] PE1000+ epoxy adhesive anchor system selection table

Injection tool	Plastic cartridge system	Extra mixing nozzle
13 fl.oz. manual dispenser Cat. #08295	PE1000+, 13 fl.oz. dual cartridge w/mixing nozzle and extension tube Cat. #0500SD	Mixing nozzle and extension tube Cat. #08294 or 08609
19.5 fl.oz. manual dispenser Cat. #08298	PE1000+, 19.5 fl.oz. dual cartridge w/mixing nozzle and extension tube Cat. #0502SD	Mixing nozzle and extension tube Cat. #08294 or 08609
13 & 19.5 fl.oz. pneumatic dispenser Cat. #8497SD	PE1000+, 13 fl.oz. dual cartridge w/mixing nozzle and extension tube Cat. #0500SD or	Mixing nozzle and extension tube Cat. #08294 or 08609
13 & 19.5 fl.oz. cordless battery dispenser Cat. #DCE593D1	PE1000+, 19.5 fl.oz. dual cartridge w/mixing nozzle and extension tube Cat. #0502SD	

A plastic extension tube (Cat# 08281 or 8297) or flexible extension hose (Cat.# PFC1640600) or equivalent approved by DEWALT must be used if the bottom or back of the anchor hole is not reached with the mixing nozzle only.

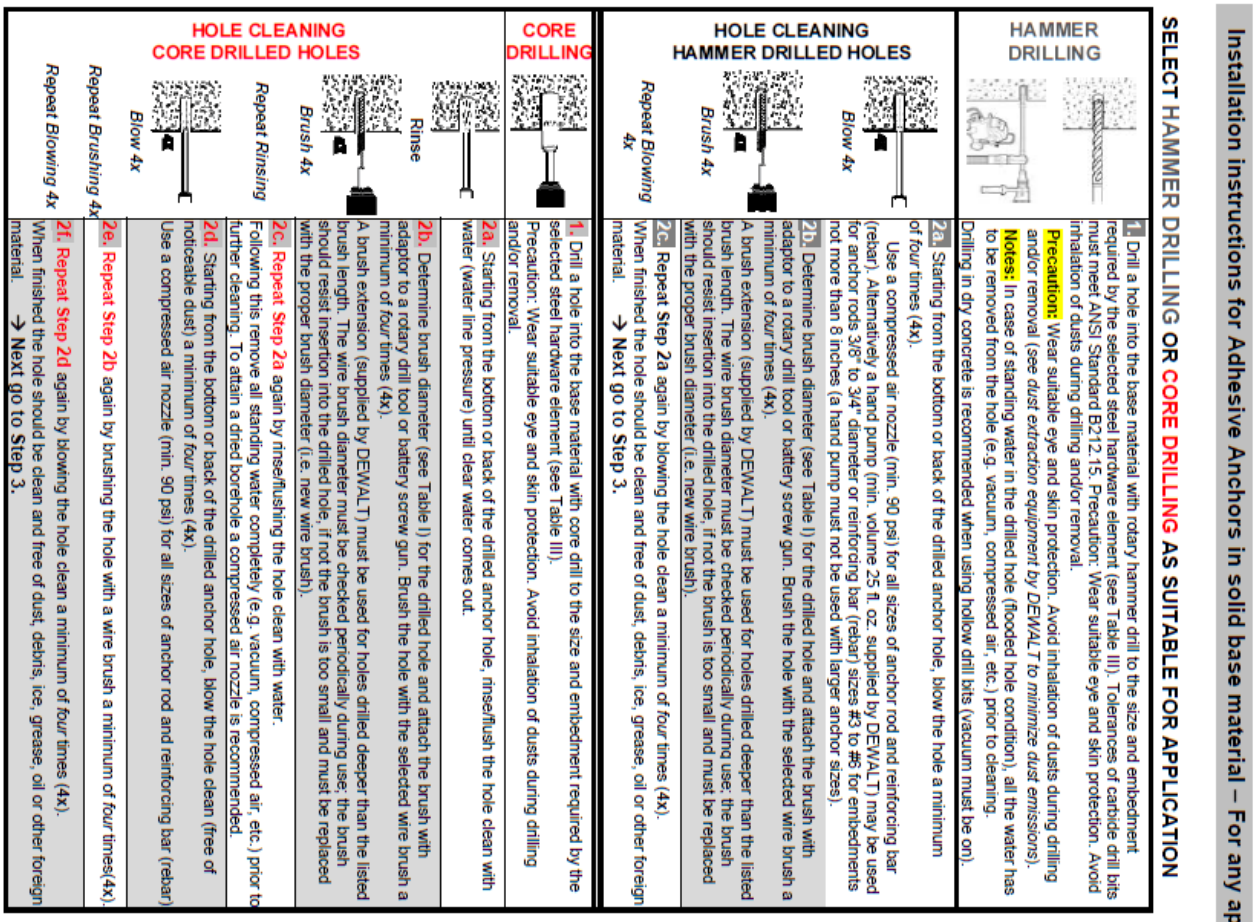
FIGURE 3—MANUFACTURER'S PUBLISHED INSTALLATION INSTRUCTIONS (MPII)

PE1000+ Instruction Card (continued)

Installation instructions for Adhesive Anchors in solid base material – For any application not covered by this document please contact DEWALT

SELECT HAMMER DRILLING OR CORE DRILLING AS SUITABLE FOR APPLICATION

FOLLOW STEPS #1 THROUGH #10 FOR RECOMMENDED INSTALLATION



1. Drill a hole into the base material with rotary hammer drill to the size and embedment required by the selected steel hardware element (see Table III). Tolerances of carbide drill bits must meet ANSI Standard B212.15. Precaution: Wear suitable eye and skin protection. Avoid inhalation of dusts during drilling and/or removal.

Precaution: Wear suitable eye and skin protection. Avoid inhalation of dusts during drilling and/or removal (*see dust extraction equipment by DEWALT to minimize dust emissions*).

Notes: In case of standing water in the drilled hole (flooded hole condition), all the water has to be removed from the hole (e.g. vacuum, compressed air, etc.) prior to cleaning.

Drilling in dry concrete is recommended when using hollow drill bits (vacuum must be on).

2a. Starting from the bottom or back of the drilled anchor hole, blow the hole a minimum of *four* times (4x).

Use a compressed air nozzle (min. 90 psi) for all sizes of anchor rod and reinforcing bar (rebar). Alternatively a hand pump (min. volume 25 fl. oz. supplied by DEWALT) may be used for anchor rods 3/8" to 3/4" diameter or reinforcing bar (rebar) sizes #3 to #6 for embedments not more than 8 inches (a hand pump must not be used with larger anchor sizes).

2b. Determine brush diameter (see Table I) for the drilled hole and attach the brush with adaptor to a rotary drill tool or battery screw gun. Brush the hole with the selected wire brush a minimum of *four* times (4x).

A brush extension (supplied by DEWALT) must be used for holes drilled deeper than the listed brush length. The wire brush diameter must be checked periodically during use; the brush should resist insertion into the drilled hole, if not the brush is too small and must be replaced with the proper brush diameter (i.e. new wire brush).

2c. Repeat Step 2a again by blowing the hole clean a minimum of *four* times (4x).

When finished the hole should be clean and free of dust, debris, ice, grease, oil or other foreign material. → Next go to Step 3.

1. Drill a hole into the base material with core drill to the size and embedment required by the selected steel hardware element (see Table III).

Precaution: Wear suitable eye and skin protection. Avoid inhalation of dusts during drilling and/or removal.

2a. Starting from the bottom or back of the drilled anchor hole, rinse/flush the hole clean with water (water line pressure) until clear water comes out.

2b. Determine brush diameter (see Table I) for the drilled hole and attach the brush with adaptor to a rotary drill tool or battery screw gun. Brush the hole with the selected wire brush a minimum of *four* times (4x).

A brush extension (supplied by DEWALT) must be used for holes drilled deeper than the listed brush length. The wire brush diameter must be checked periodically during use; the brush should resist insertion into the drilled hole, if not the brush is too small and must be replaced with the proper brush diameter (i.e. new wire brush).

2c. Repeat Step 2a again by rinse/flushing the hole clean with water.

Following this remove all standing water completely (e.g. vacuum, compressed air, etc.) prior to further cleaning. To attain a dried borehole a compressed air nozzle is recommended.

2d. Starting from the bottom or back of the drilled anchor hole, blow the hole clean (free of noticeable dust) a minimum of *four* times (4x).

Use a compressed air nozzle (min. 90 psi) for all sizes of anchor rod and reinforcing bar (rebar)

2e. Repeat Step 2b again by brushing the hole with a wire brush a minimum of *four* times(4x).

2f. Repeat Step 2d again by blowing the hole clean a minimum of *four* times (4x).

When finished the hole should be clean and free of dust, debris, ice, grease, oil or other foreign material. → Next go to Step 3.

3. Check adhesive expiration date on cartridge label. Do not use expired product. Review Safety Data Sheet (SDS) before use. Cartridge temperature must be between 41°F - 95°F (5°C - 35°C) when in use; for downward applications only the adhesive temperature may be up to 104°F (40°C). Review published working and cure times. Consideration should be given to reduced gel (working) time of the adhesive in warm temperatures. For the permitted range of the base material temperature see Table II.

Attach a supplied mixing nozzle to the cartridge. Do not modify the mixer in any way and make sure the mixing element is inside the nozzle. Load the cartridge into the correct dispensing tool.

Note: *Always use a new mixing nozzle with new cartridges of adhesive and also for all work interruptions exceeding the published gel (working) time of the adhesive.*

4. Prior to inserting the anchor rod or rebar into the filled hole, the position of the embedment depth has to be marked on the anchor. Verify anchor element is straight and free of surface damage.

5. Adhesive must be properly mixed to achieve published properties. Prior to dispensing adhesive into the drilled hole, separately dispense min. three full strokes of adhesive through the mixing nozzle until the adhesive is a consistent *red* color. Review and note the published working and cure times (see Table II) prior to injection of the mixed adhesive into the cleaned anchor hole.

6. Fill the cleaned hole approximately two-thirds full with mixed adhesive starting from the bottom or back of the anchor hole. If the bottom or back of the anchor hole is not reached with the mixing nozzle only, a plastic extension tube must be used (see Table IV). Slowly withdraw the mixing nozzle as the hole fills to avoid creating air pockets or voids.

Note: Piston plugs (see Table V) must be used with and attached to mixing nozzle and extension tube for overhead and horizontal installations with anchor rods 5/8" to 1-1/4" and rebar sizes #5 to #10. Insert piston plug to the back of the drilled hole and inject as described in the method above. During installation the piston plug will be naturally extruded from the drilled hole by the adhesive pressure.

Attention! *Do not install anchors overhead without proper training and installation hardware provided by DEWALT; contact DEWALT prior to use.*

7. The anchor should be free of dirt, grease, oil or other foreign material. Push clean threaded rod or reinforcing bar into the anchor hole while turning slightly to ensure positive distribution of the adhesive until the embedment depth is reached. Observe the gel (working) time.

8. Ensure that the anchor element is installed to the specified embedment depth. Adhesive must completely fill the annular gap at the concrete surface. Following installation of the anchor element, remove excess adhesive. Protect the anchor element threads from fouling with adhesive.

For all installations the anchor element must be restrained from movement throughout the specified curing period (where necessary) through the use of temporary wedges, external supports, or other methods. Minor adjustments to the position of the anchor element may be performed during the gel time only.

9. Allow the adhesive anchor to cure to the specified full curing time prior to applying any load (see Table II).

Do not disturb, torque or load the anchor until it is fully cured.

10. After full curing of the adhesive anchor, a fixture can be installed to the anchor and tightened up to the maximum torque (shown in Table III) by using a calibrated torque wrench.

Note: *Take care not to exceed the maximum torque for the selected anchor.*

FIGURE 3—MANUFACTURER'S PUBLISHED INSTALLATION INSTRUCTIONS (MPII) (continued)

ICC-ES Evaluation Report

ESR-2583 FBC Supplement

Reissued December 2018

This report is subject to renewal December 2019.

www.icc-es.org | (800) 423-6587 | (562) 699-0543 *A Subsidiary of the International Code Council®*

DIVISION: 03 00 00—CONCRETE
Section: 03 16 00—Concrete Anchors

DIVISION: 05 00 00—METALS
Section: 05 05 19—Post-Installed Concrete Anchors

REPORT HOLDER:

DEWALT

EVALUATION SUBJECT:

PE1000+® EPOXY ADHESIVE ANCHOR SYSTEM IN CRACKED AND UNCRACKED CONCRETE (DEWALT / POWERS)

1.0 REPORT PURPOSE AND SCOPE

Purpose:

The purpose of this evaluation report supplement is to indicate that the PE1000+® Epoxy Adhesive Anchor System in Cracked and Uncracked Concrete, recognized in ICC-ES master evaluation report ESR-2583, has also been evaluated for compliance with the codes noted below.

Applicable code editions:

- 2017 *Florida Building Code—Building*
- 2017 *Florida Building Code—Residential*

2.0 CONCLUSIONS

The PE1000+® Epoxy Adhesive Anchor System in Cracked and Uncracked Concrete**,** described in Sections 2.0 through 7.0 of the master evaluation report ESR-2583, complies with the *Florida Building Code—Building* and the *Florida Building Code—Residential*, provided the design and installation are in accordance with the 2015 *International Building Code®* (IBC) provisions noted in the master report.

Use of the PE1000+® Epoxy Adhesive Anchor System with stainless steel threaded rod materials and reinforcing bars has also been found to be in compliance with the High-Velocity Hurricane Zone provisions of the *Florida Building Code—Building* and the *Florida Building Code—Residential.*

For products falling under Florida Rule 9N-3, verification that the report holder's quality-assurance program is audited by a quality-assurance entity approved by the Florida Building Commission for the type of inspections being conducted is the responsibility of an approved validation entity (or the code official, when the report holder does not possess an approval by the Commission).

This supplement expires concurrently with the master report, reissued December 2018.

The Public Health and Safety Organization

NSF Product and Service Listings

These NSF Official Listings are current as of **Monday, September 15, 2014** at 12:15 a.m. Eastern Time. Please contact NSF International to confirm the status of any Listing, report errors, or make suggestions.

Alert: NSF is concerned about fraudulent downloading and manipulation of website text. Always confirm this information by clicking on the below link for the most accurate information:
http://info.nsf.org/Certified/PwsComponents/Listings.asp?Company=4M370&Standard=061&

NSF/ANSI 61
Drinking Water System Components - Health Effects

NOTE: Unless otherwise indicated for Materials, Certification is only for the Water Contact Material shown in the Listing. Click here for a list of Abbreviations used in these Listings.

Powers Fasteners, Inc.
2 Powers Lane
Brewster, NY 10509
United States
914-235-6300

Facility : # 1 Germany

Joining and Sealing Materials

Trade Designation	Size	Water Contact Temp	Water Contact Material
Adhesives			
AC100+ Gold	[1]	D. HOT	VE
AC100-PRO	[1]	D. HOT	VE
PE 1000+	[1]	D. HOT	EPOXY
PF PRO	[1]	D. HOT	EPOXY
PURE150-PRO	[1]	D. HOT	EPOXY
V12	[1]	D. HOT	VE

[1] Certified or use as an anchoring adhesive for installing thread rods (less than or equal to 1.3 inches in diameter) in concrete or masonry for water treatment applications.
