CANADIAN TECHNICAL PAGE SUPPLEMENT



GENERAL INFORMATION

POWER-STUD®+ SD2

High Performance Wedge Expansion Anchor

PRODUCT DESCRIPTION

The Power-Stud+ SD2 anchor is a fully threaded, torque-controlled, wedge expansion anchor which is designed for consistent performance in cracked and uncracked concrete. Suitable base materials include normal-weight concrete, lightweight concrete and concrete over steel deck. The anchor is manufactured with a zinc plated carbon steel body and stainless steel expansion clip for premium performance.

GENERAL APPLICATIONS AND USES

- Structural connections, i.e., beam and column anchorage
- Interior applications / low level corrosion environment
- Tension zone applications, i.e., safety-related attachments
- Seismic and wind loading
- Utility supports, e.g. pipe, strut, trapeze, bracing
- Equipment anchorage, angles, brackets and ledgers
- Barriers, guards and fencing
- Mezzanines, racking and railing

FEATURES AND BENEFITS

- + Consistent performance in high and low strength concrete
- + Nominal drill bit size is the same as the anchor diameter
- + Anchor can be installed through standard fixture holes
- + Length ID code and identifying marking stamped on head of each anchor
- + Anchor design allows for follow-up expansion after setting under tensile loading

APPROVALS AND LISTINGS

- International Code Council, Evaluation Service (ICC-ES), ESR-2502 and ELC-2502 for cracked and uncracked concrete
- Code Compliant with the 2020/2015 NBCC and 2021 IBC/IRC, 2018 IBC/IRC, 2015 IBC/IRC, and 2012 BC/IRC
- Tested in accordance with ACI 355.2/ASTM E488 and ICC-ES AC193 for use in structural concrete under the design provisions of ACI 318 (-19 or -14) Chapter 17 and CSA A23.3 (-19, -14, -04) Annex D or ACI 318 Appendix D
- Evaluated and gualified by an accredited independent testing laboratory for recognition in cracked and uncracked concrete including seismic and wind loading (Category 1 anchors)
- City of Los Angeles, LABC and LARC Supplement (within ESR-2502)
- Florida Building Code, FBC Supplement including HVHZ (within ESR-2502)
- FM Approvals (Factory Mutual) see FM Approval Guide for sizes
- Underwriters Laboratory (UL Listed) File No. EX1289 and VFXT7.EX1289, see listing for sizes

GUIDE SPECIFICATIONS

CSI Divisions: 03 16 00 - Concrete Anchors, 04 05 19.16 - Masonry Anchors and 05 05 09 - Post-Installed Concrete Anchors. Expansion anchors shall be Power-Stud+ SD2 as supplied by DEWALT, Towson, MD. Anchors shall be installed in accordance with published instructions and the Authority Having Jurisdiction.

MATERIAL SPECIFICATIONS

Anchor component	Specification
Anchor Body	Medium carbon steel
Hex nut	Carbon steel, ASTM A563, Grade A
Washer	Carbon Steel, ASTM F844; meets dimensional requirements of ANSI B18.22.2. Type A Plain
Expansion wedge (clip)	316 Stainless Steel
Plating (anchor body, nut and washer)	Zinc plating according to ASTM B633, SC1 Type III (Fe/Zn 5) Minimum plating requirements for Mild Service Condition.
See Tension Design Information table for yield and	d ultimate strengths of the anchor body.

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POWER-STUD+ SD2 ASSEMBLY

THREAD VERSION

UNC threaded stud

ANCHOR MATERIALS

 Zinc plated carbon steel body with stainless steel expansion clip, zinc plated carbon steel nut and washer

ANCHOR SIZE RANGE (TYP.)

• 3/8" through 3/4" diameters

SUITABLE BASE MATERIALS

- Normal-weight concrete
- · Lightweight concrete
- Concrete over steel deck
- Grouted-filled concrete masonry (CMU)



CODE LISTED ICC-FS FSB-2502 CONCRETE

CODE LISTED C-FS FI C-2502 CONCRETE





CHANICAL

ANCHO

INSTALLATION SPECIFICATIONS

Power-Stud+ SD2 Anchor Installation Table

Anchor Property/	Natalian	Unite					Nomin	al Anchor	Size			
Setting Information	Notation	Units	3/8"		1/	2"			5/8"		3/	4"
Outside diameter of anchor	da	mm (in.)	9.5 (0.375)		12 (0.5	2.7 500)			15.9 (0.625)		19 (0.7).1 '50)
Minimum diameter of hole clearance in fixture	dh	mm (in.)	11.1 (7/16)		14 (9/	l.3 16)			17.5 (11/16)		20 (13).6 /16)
Nominal drill bit diameter	d _{bit}	in.	3/8 ANSI	1/2 ANSI			5/8 ANSI			3/4 ANSI		
Minimum nominal embedment depth ²	h _{nom}	mm	60	64 83		98	124		114	146		
Effective embedment	h _{ef}	mm	51	5	1	8	3	83	108		95	127
Minimum concrete member thickness	hmin	mm	102	114	152	146	146	146	165	203	178	254
Critical edge distance	Cac	mm	165	20)3	25	54	203	400	254	305	305
Minimum edge distance	Cmin	mm	64	102	70	102	70	108	10)8	127	114
Minimum spacing distance	Smin	mm	89	152	152	102	152	108	10)8	152	152
Minimum hole depth	h₀	mm	67	70		102		108	13	33	127	159
Minimum overall anchor length ³	lanch	mm	76	9	5	11	14	121	15	52	159	178
Installation torque	Tinst	N-m	27	54		81			14	49		
Torque wrench / socket size	-	in.	9/16	3/4		15/16			1-1	1/8		
Nut height	-	in.	21/64		7/	16		35/64			4-1	/64

For SI: 1 inch = 25.4 mm, 1 ft-lbf = 1.356 N-m.

1. The information presented in this table is to be used in conjunction with the design criteria of CSA A23.3 (-19, -14, -04) Annex D, as applicable.

2. The embedment depth, hnom, is measured from the outside surface of the concrete member to the embedded end of the anchor prior to tightening.

3. The listed minimum overall anchor length is based on anchor sizes commercially available at the time of publication compared with the requirements to achieve the minimum nominal embedment depth and possible fixture attachment.

Anchor Setting Information for Installation on the Top of Concrete-Filled Steel Deck Assemblies³⁴

Anchor Property/	Matalian			Nominal Ancl	hor Size (inch)			
Setting Information	Notation	Units	3	/8"	1/	2"		
Nominal drill bit diameter (ANSI)	d _{bit}	in.	3/8	ANSI	1/2	ANSI		
Minimum nominal embedment depth ¹	h _{nom}	mm (in.)	(2-	60 -3/8)	6 (2-	64 1/2)		
Effective embedment	h _{ef}	mm (in.)	(2	51 00)	5 (2.	51 00)		
Minimum concrete member thickness ²	h _{min,deck}	mm (in.)	(2-	64 -1/2)	64 (2-1/2)			
Critical edge distance for topside of concrete- filled steel deck assemblies with minimum topping thickness (uncracked concrete only)	Cac,deck,top	mm (in.)	2	203 (8)	22	29 9)		
Minimum edge distance	Cmin,deck,top	mm (in.)	102 (4)	70 (2-3/4)	102 (4)	203 (8)		
Minimum spacing distance	Smin,deck,top	mm (in.)	89 (3-1/2)	152 (6)	203 (8)	102 (4)		
Minimum hole depth	h₀	mm (in.)	(2-	64 -1/2)	6 (2-	64 1/2)		
Installation torque	Tinst	N-m (ftlb.)		27 20)	5 (4	64 0)		
Torque wrench socket size	-	in.	9	/16	3/4			
Nut height	eight -			1/64	7/	16		
Washer O.D.	-	in.	1:	3/16	1-1/16			

For SI: 1 inch = 25.4 mm, 1 ft-lbf = 1.356 N-m.

1. The embedment depth, hnom, is measured from the outside surface of the concrete member to the embedded end of the anchor prior to tightening.

The anchors may be installed in the topside of concrete-filled steel deck floor and roof assemblies provided the concrete thickness above the upper flute meets the minimum thicknesses specified in this table. Minimum concrete member thickness refers to the concrete thickness above the upper flute (topping thickness). See Installation Detail D.

For all other anchor diameters and embedment depths, refer to the installation table for applicable values of hmin, Cmin and Smin.

4. Design capacities shall be based on calculations according to values in Tension Design Information and Shear Design Information tables.



MECHANICAL ANCHORS

POWER-STUD®+ SD2 High Performance Wedge Expansion Anchor

Power-Stud+ SD2 Anchor Detail





Interpolation of Minimum Edge Distance and Anchor Spacing



This interpolation applies to the cases when two sets of minimum edge distances, $c_{\text{min},}$ and minimum spacing distances, s_{min} , are given for a selected anchor diameter effective embedment depth, h_{ef} , and corresponding minimum member thickness, h_{min} .

Power-Stud+ SD2 Anchor Assembly

Head Marking



Legend

- Letter Code = Length Identification Mark
- +' Symbol = Strength Design Compliant Anchor

Number Code 2 = Carbon Steel Body and Stainless Steel Expansion Clip



Length Identification

- J																
Mark	A	B	C	D	E	F	G	H	I	J	K	L	М	N	0	Р
From	1-1/2"	2"	2-1/2"	3"	3-1/2"	4"	4-1/2"	5"	5-1/2"	6"	6-1/2"	7"	7-1/2"	8"	8-1/2"	9"
Up to but not including	2"	2-1/2"	3"	3-1/2"	4"	4-1/2"	5"	5-1/2"	6"	6-1/2"	7"	7-1/2"	8"	8-1/2"	9"	9-1/2"
Length identification r	nark indicat	tos ovorall l	onath of ar	nchor												

Step 3

Length identification mark indicates overall length of ancho

INSTALLATION INSTRUCTIONS

Installation Instructions for Power-Stud+ SD2



Step 1

Using the proper drill bit size, drill a hole into the base material to the required depth. The tolerances of the drill bit used should meet the requirements of ANSI Standard B212.15.

Step 2

Remove dust and debris from the hole during drilling, (e.g. dust extractor, hollow bit) or following drilling (e.g. suction, forced air) to extract loose particles created by drilling.

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Position the washer on the anchor and thread on the nut. If installing through

a fixture, drive the anchor through the

is driven to the minimum required embedment depth, h_{nom} .

fixture into the hole. Be sure the anchor

Step 4

Tighten the anchor with a torque wrench by applying the required installation torque, T_{inst}.

CHANICAL ANCHORS

Performance Wedge Expansion Anchor

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- REV. D

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Installation Detail A: Power-Stud+ SD2 Installed in the Soffit of Concrete over Steel Deck Floor and Roof Assemblies (see dimensional profile requirements)¹



Installation Detail B: Power-Stud+ SD2 Installed in the Soffit of Concrete Over Steel Deck Floor and Roof Assemblies (see dimensional profile requirements)¹



Installation Detail C: Power-Stud+ SD2 Installed in the Soffit of Concrete over Steel Deck Floor and Roof Assemblies (See Dimensional Profile Requirements)^{1,2}



Installation Detail D: Installation Detail for Anchors in the Top of Concrete Over Steel Deck Floor and Roof Assemblies (see dimensional profile requirements)¹²



1-800-4 **DeWALT**



LIMIT STATES INFORMATION

teristic	Notation	Units			Nominal A	Anchor Diamo	eter (inch)			
	1.0 er 0		3/8	1.	2	5/	/8	3/	4	
	1,2 or 3	- IFFL STRENG	TH IN TENSIO	 DN ⁴						
h (neck)	fy	N/mm²	662	58	36	58	36	48	33	
sile strength (neck)	f _{uta}	N/mm²	827	7:	31	73	31	62	20	
ck)	Ase, N	mm²	35.6	65	i.0	10-	4.5	153.2		
	N _{sa}	kN	29.5	46	6.5	58	3.2	94.4		
tance factor for	$\phi_{ m s}$	-				0.85				
r steel strength ³	R	-				0.80				
	CONCRET	E BREAKOUT	STRENGTH II	N TENSION [®]						
	hef	mm	51	51	83	83	108	95	127	
ed concrete	Kucr	-	10	1	0	1	0	10		
concrete	k _{cr}	-	7		7	7	7	7		
and uncracked concrete6	$\psi_{ ext{c,N}}$	-	1.0 See note 6	1 See r	.0 lote 6	1. See r	.0 iote 6	1. See n	0 ote 6	
ncrete	$\phi_{ m c}$	-				0.65				
	Cac	mm		See note 6						
or tension, concrete failure	R	-				1.0				
	PUI	LLOUT STREN	GTH IN TENS	IONº						
cracked concrete	N _{p,uncr}	kN	12.3	See note 8	29.4	See note 8	See note 8	See note 8	See note 8	
cracked concrete	N _{p,cr}	kN	9.6	See note 8	19.5	See note 8	See note 8	See note 8	35.1	
ncrete	ϕ_{c}	-				0.65				
or tension, pullout	R	-				1.0				
PULL	OUT STRENG	TH IN TENSIO	N FOR SEISM	NIC APPLICAT	IONS					
seismic (17.2 MPa) ^{7,10}	N _{p,eq}	kN	9.6	See note 8	19.5	See note 8	See note 8	See note 8	35.1	
ncrete	$\phi_{ m c}$	-	0.65							
or pullout strength,	R					1.0				
Uncracked concrete	β	kN/mm	1517	12	58	99	98	747		
Cracked concrete	β	kN/mm	87	1(00	113		126		
	teristic teristic teristic teristic turce turce turce turce turce turce turce turce turce turce turce turce turce turce turce turce turce tur	teristicNotation1,2 or 3 $1,2$ or 3h (neck)fysile strength (neck)futak)Ase, NLance factor for ϕ_s r steel strength3RCONCRETTad concretekuarconcretekuarconcretekarund uncracked concrete ⁶ $\psi_{c,N}$ ncrete ϕ_c caccacor tension, concrete failureRuracked concreteNp.uncrrracked concreteNp.uncrrracked concreteNp.orncrete ϕ_c or tension, pulloutRPULLOUT STRENGseismic (17.2 MPa) ^{7,10} Np.eqncrete ϕ_c or pullout strength,RUncracked concrete β Cracked concrete β	teristicNotationUnits1,2 or 3-STEEL STRENGh (neck)fyN/mm²sile strength (neck)futaN/mm²sile strength (neck)futaN/mm²k)Ase, Nmm²k)Ase, Nmm²r steel strength³R-r steel strength³R-concretekucr-concretekucr-concretekcr-ind uncracked concrete ⁶ $\psi_{c,N}$ -or tension, concrete failureR-racked concreteNp,orrkNracked concreteNp,orrkNracked concreteNp,orkNiracked concrete ϕ_{b} -or tension, pulloutR-ceismic (17.2 MPa)7.10Np,eqkNncrete ϕ_{b} -or pullout strength,R-Uncracked concrete β kN/mm	teristicNotationUnits3/81,2 or 3-1STEEL STRENGTH IN TENSIONh (neck)fyN/mm²662sile strength (neck)futaN/mm²827ik)Ase, Nmm²35.6NsaKN29.5tance factor for ϕ_{c} -r steel strength³R-CONCRETE BREAKOUT STRENGTH INConcretekar-net concretekwar-net uncracked concrete [®] $\psi_{c,N}$ -or tension, concrete failureR-racked concreteNp,unerKNstracked concreteNp,unerKNstracked concrete ϕ_{c} -or tension, pulloutR-racked concrete ϕ_{c} -or tension, pulloutR-pullout strength,R-seismic (17.2 MPa) ^{7.10} Np,eqKNNucracked concrete ϕ_{c} -pullout strength,R-uncracked concrete β kN/mmKN1517Cracked concrete β kN/mmKN1517	teristicNotationUnits $3/8$ 1/11,2 or 3-1-STEEL STRENGTH IN TENSION'h (neck)fyN/mm²66258sile strength (neck)fataN/mm²82773k)A _{ee, N} mm²35.6665NsakN29.546tance factor for ϕ_{b} -r steel strength²R-CONCRETE BREAKOUT STRENGTH IN TENSION'net el strength²R-hermm51513d concretekwar-100nd uncracked concrete ^a ψ_{cN} - $\frac{1.0}{Nee}$ cacemm5151or rension, concrete failureR-racked concreteN _{pouter} KN12.3sceen racked concreteN _{pouter} KN9.6sceen racked concrete ϕ_{e} racked concrete ϕ_{e} seismic (17.2 MPa) ^{1,10} N _{poeq} KN9.6See note 8ncrete ϕ_{e} or pullout strength,Runcracked concrete β kN/mm87uncracked concrete β kN/mm<	teristic Notation Units $3/8$ $1/2$ 1,2 or 3 - 1 1 STEEL STRENCTH IN TENSION' h (neck) f _y N/mm² 662 586 sile strength (neck) f _{uta} N/mm² 827 731 ik) A _{m,N} mm² 35.6 65.0 acce factor for ϕ_k - Nata kN 29.5 46.5 tance factor for ϕ_k - CONCRETE BREAKOUT STRENGTH IN TENSION* 83 ad concrete kwcr - 10 10 concrete kwcr - 7 7 ind uncracked concrete [#] $\psi_{c,M}$ - See note 6 1.0 See note 6 rcrete ϕ_k - 1.2.3 See note 6 racked concrete $N_{p,wa}$ kN 1	teristic Notation Units Nominal Anchor Diam 1,2 or 3 - 1 1 1 STEL STRENTH IN TENSION' h (neck) fr N/mm² 662 586 586 sile strength (neck) fum N/mm² 827 731 73 k) Aue, N mm² 35.6 65.0 100 Naa kN 29.5 46.5 586 tance factor for ϕ_h - 0.85 51 concrete kur - 0.80 83 83 ad concrete kur - 10 10 1 concrete kur - 10 10 1 concrete kur - Nome See note 6 See r ncrete ϕ_h - 1.0 1.0 See note 6 See r retesion, concrete failure R - 1.0 See note 6 See r racked concrete	Instruction Units Nominal Anchor Diameter (inclus) 1,2 or 3 - 1 1 1 1,2 or 3 - 1 1 1 1 STELE STRENETH IN TENSION" Test STRENETH IN TENSION" h (neck) f _µ N/mm² 827 731 731 k) A _m , N mm² 35.6 65.0 104.5 ance factor for φ _h - 0.85 586.2 concrete BEAKOUT STRENCTH IN TENSION" OLOBO CONCRETE BREAKOUT STRENCTH IN TENSION" OLOBO <td colspan="4</td> <td>Notation Units Nominal Anchor Diameter (inch) 3/8 1/2 5/8 3/4 1,2 or 3 - 1</td>	Notation Units Nominal Anchor Diameter (inch) 3/8 1/2 5/8 3/4 1,2 or 3 - 1	

For SI: 1 inch = 25.4 mm, 1 ft-lbf = 1.356 N-m, 1 ksi = 6.895 N/mm², 1 lbf = 0.0044 kN.

1. The data in this table is intended to be used with the design provisions of CSA A23.3 (-19, -14, -04) Annex D, as applicable; for anchors resisting seismic load combinations the additional requirements CSA A23.3 (-19, -14, -04) D.4.3, as applicable, must apply

2. Installation must comply with published instructions and details.

3. All values of R for use with the load combinations of Division B, Part 4, Section 4.1.3 of the 2020 NBCC, 2015 NBCC or 2010 NBCC, CSA A23.3-19 Annex C, CSA A23.3-14 Annex C or CSA A23.3-04 Annex C, as applicable. Condition B applies where supplementary reinforcement in conformance with CSA A23.3-19 D.5.3 (c), CSA A23.3-14 D.5.3(c) or CSA A23.3-04 D.5.4(c), as applicable, is not provided, or where pullout or pryout strength governs. For cases where the presence of supplementary reinforcement can be verified, the strength reduction factors associated with Condition A may be used

4. The Power-Stud+ SD2 is considered a ductile steel element in tension as defined by CSA A23.3-19 D.2, CSA A23.3-14 D.2 or CSA A23.3-04 D.2, as applicable.

5. Tabulated values for steel strength in tension are based on test results per ACI 355.2 and must be used for design in lieu of calculation.

6. For all design cases $\Psi_{e,N} = 1.0$. The appropriate effectiveness factor for cracked concrete (ker) or uncracked concrete (kuner) must be used.

7. For all design cases $\Psi_{c,P}$ =1.0. For the calculation of N_{cpr}, see CSA A23.3 (-19, -14, -04) D.6.3.

8. Pullout strength does not control design of indicated anchors. Do not calculate pullout strength for indicated anchor size and embedment.

9. Anchors are permitted to be used in lightweight concrete in accordance with CSA A23.3 (-19, -14, -04) D.4.

10. Tabulated values for characteristic pullout strength in tension are for seismic applications are based on test results per ACI 355.2 (Section 9.5), as referenced in CSA

11. Actual stiffness of the mean value varies considerably depending on concrete strength, loading and geometry of application.

ECHANICAL

ANCHORS

CODE LISTED

Shear Design Information for Power-Stud+ SD2 in Concrete^{1,2,8}

Shear Design information for Fower-Su	IUT JUZ						Ľ	CC-ES ESR-2502	2 Ants
Desiry Characteristic	Notation	Unite		I	Nominal And	chor Diamet	er (inch)		
Design Characteristic	Notation	Units	3/8	1/	/2	5.	/8	3/	/4
Anchor category	1,2 or 3	-	1		1		1		1
		STEEL STREM	IGTH IN SHEAR ⁴						
Minimum specified yield strength (threads)	fy	N/mm²	530	46	69	46	69	38	36
Minimum specified ultimate strength (threads)	f _{uta}	N/mm²	690	60)7	60)7	55	51
Effective tensile stress area (threads)	Ase, v	mm²	50.0	65	5.7	10	4.9	21	5.8
Steel strength in shear ⁵	Vsa	kN	13.9	21	.4	45	i.2	56.1	
Steel embedment material resistance factor for reinforcement	$\phi_{ m s}$	-				0.85			
Resistance modification factor for steel strength, shear ^{3,4}	R	-				0.75			
	CONCRE	ETE BREAKOU	T STRENGTH IN	SHEAR					
Load-bearing length of anchor		mm	51 51 83 83 103 95 127						
Nominal anchor diameter	Vsa, eq	mm	9.5	12	2.7	15	5.9	19).1
Steel embedment material resistance factor for reinforcement	$\phi_{\scriptscriptstyle \mathrm{S}}$	-				0.65			
Resistance modification factor for shear, concrete failure modes, Condition B^3	R	-				1.0			
	I	PRYOUT STRE	NGTH IN SHEAR®	i .					
Coefficient for pryout strength (1.0 for $h_{ef} < 2.5$ in.)	Kcp	-	1.0	1.0	2.0	2.0	2.0	2.0	2.0
Effective embedment	hef	mm	51	51	83	83	108	95	127
Steel embedment material resistance factor for reinforcement	$\phi_{ m s}$	-				0.65			
Resistance modification factor for pryout strength ³ R - 1.0									
S	TEEL STREN	GTH IN SHEAF	R FOR SEISMIC A	PPLICATION	S				
Steel strength in shear, seismic ⁷	$V_{sa,eq}$	kN	11.0	21	.4	30).1	35	ō.9
Steel embedment material resistance factor for reinforcement	$\phi_{ m s}$	-				0.85			
Resistance modification factor for steel strength, shear, seismic ³	R	-				0.75			

For SI: 1 inch = 25.4 mm, 1 ft-lbf = 1.356 N-m, 1 ksi = 6.895 N/mm^2 , 1 lbf = 0.0044 kN.

1. The data in this table is intended to be used with the design provisions of CSA A23.3 (-19,-14, -04) Annex D, as applicable; for anchors resisting seismic load combinations the additional requirements CSA A23.3 (-19, -14, -04) D.4.3, as applicable, must apply.

2. Installation must comply with published instructions and details.

3. All values of R for use with the load combinations of Division B, Part 4, Section 4.1.3 of the 2020 NBCC, 2015 NBCC or 2010 NBCC, CSA A23.3-19 Annex C, CSA A23.3-14 Annex C or CSA A23.3-04 Annex C, as applicable. Condition B applies where supplementary reinforcement in conformance with CSA A23.3-19 D.5.3 (c), CSA A23.3-14 D.5.3(c) or CSA A23.3-04 D.5.4(c), as applicable, is not provided, or where pullout or pryout strength governs. For cases where the presence of supplementary reinforcement can be verified, the strength reduction factors associated with Condition A may be used.

4. The Power-Stud+ SD2 is considered a ductile steel element as defined by CSA A23.3-19 D.2, CSA A23.3-14 D.2 or CSA A23.3-04 D.2, as applicable.

5. Tabulated values for steel strength in shear are based on test results per ACI 355.2, Section 9.4 and must be used for design. These tabulated values are lower than calculated results using equation D.31 in CSA A23.3-19 or CSA A23.3-14.

6. Anchors are permitted to be used in lightweight concrete in accordance with CSA A23.3 (-19, -14, -04) D.4.6.

7. Tabulated values for steel strength in shear are for seismic applications and based on test results in accordance with ACI 355.2 (Section 9.6), as referenced in CSA A23.3- Section D.4.3.4.

Performance Wedge Expansion Anchor

SD2

+®QN,

-S-

DEWALT. Anchors & Fasteners

Tension and Shear Design Data for Power-Stud+ SD2 Anchors in the Soffit of Concrete-Filled Steel Deck Assemblies^{1,2,7}



	leainn Characharialiae	Notation	Ilmite			Nominal Anchor Size (inch)					
Ľ	esign characteristics	Notation	Units	0.375	0	.5	0.6	625	0.75		
Anchor Category		1, 2 or 3	-	1		1		1	1		
Minimum Nominal	Embedment Depth	hnom	mm (in.)	60 (2-3/8)	64 (2-1/2)	83 (3-3/4)	98 (3-7/8)	124 (4-7/8)	114 (4-1/2)		
Effective Embedme	ent	hef	mm (in.)	51 (2.00)	51 (2.00)	83 (3.25)	83 (3.25)	108 (4.25)	95 (3.75)		
Minimum Hole Dep	oth	h₀	mm (in.)	67 (2-5/8)	70 (2-3/4)	102 (4)	108 (4-1/4)	133 (5-1/4)	27 (5)		
P	ULLOUT STRENGTH IN TENSION FOR ANCHO	ORS IN SOFFI	r of sand li	GHTWEIGHT AN	ID NORMAL-W	EIGHT CONCRI	ETE OVER STEI	EL DECK'			
According to	Characteristic pullout strength,	N _{p,deck,uncr}	kN	8.3	9.2	17.5	20.8	32.8	21.8		
Detail A	uncracked concrete over steel deck ²		(lbf)	(1,855)	(2,065)	(3,930)	(4,665)	(7,365)	(4,900)		
4-1/2-inch-wide	Characteristic pullout strength,	N _{p,deck,cr}	kN	6.4	6.5	11.6	14.7	23.2	15.4		
deck flute	cracked concrete over steel deck ^{2,3}	(N _{p,deck,eq})	(lbf)	(1,445)	(1,465)	(2,600)	(3,305)	(5,215)	(3,470)		
According to	Characteristic pullout strength,	Np,deck,uncr	kN	9.9	12.4	24.9	19.9	32.3	Not		
Detail B	uncracked concrete over steel deck ²		(lbf)	(2,235)	(2,785)	(5,600)	(4,480)	(7,265)	Applicable		
3-7/8-inch-wide	Characteristic pullout strength,	N _{p,deck,cr}	kN	7.8	8.8	16.4	14.1	22.9	Not		
deck flute	cracked concrete over steel deck ^{2,3}	(N _{p,deck,eq})	(lbf)	(1,745)	(1,975)	(3,695)	(3,175)	(5,145)	Applicable		
According to	Characteristic pullout strength,	Np,deck,uncr	kN	7.1	9.0	Not	Not	Not	Not		
Detail C	uncracked concrete over steel deck ²		(lbf)	(1,600)	(2,025)	Applicable	Applicable	Applicable	Applicable		
1-3/4-inch-wide	Characteristic pullout strength,	N _{p,deck,cr}	kN	5.6	6.4	Not	Not	Not	Not		
deck flute	cracked concrete over steel deck ^{2,3}	(N _{p,deck,eq})	(lbf)	(1,250)	(1,435)	Applicable	Applicable	Applicable	Applicable		
Reduction factor for	or pullout strength ⁶	ϕ	-			0.	65				
	STEEL STRENGTH IN SHEAR FOR ANCHORS	S IN SOFFIT O	F SAND-LIGH	TWEIGHT AND	NORMAL WEI	HT CONCRETE	OVER STEEL				
According to	Steel strength in shear,	V _{sa,deck}	lbf	2,170	3,815	5,040	4,015	6,670	4,325		
Detail A	concrete over steel deck		(kN)	(9.7)	(17.0)	(22.4)	(17.9)	(29.7)	(19.2)		
4-1/2-inch-wide	Steel strength in shear, seismic,	V _{sa,deck,eq}	lbf	1,715	3,815	5,040	2,675	4,445	2,820		
deck flute	concrete over steel deck		(kN)	(7.6)	(17.0)	(22.4)	(11.9)	(19.8)	(12.5)		
According to	Steel strength in shear,	V _{sa,deck}	lbf	3,040	2,675	4,930	Not	Not	Not		
Detail B	concrete over steel deck		(kN)	(13.5)	(11.9)	(21.9)	Applicable	Applicable	Applicable		
3-7/8-inch-wide	Steel strength in shear, seismic,	Vsa,deck,eq	lbf	2,400	2,675	4,930	Not	Not	Not		
deck flute	concrete over steel deck		(kN)	(10.6)	(11.9)	(21.9)	Applicable	Applicable	Applicable		
According to	Steel strength in shear,	Vsa,deck	lbf	2,170	2,880	Not	Not	Not	Not		
Detail C	concrete over steel deck		(kN)	(9.7)	(12.8)	Applicable	Applicable	Applicable	Applicable		
1-3/4-inch-wide deck flute	1-3/4-inch-wide deck flute Steel strength in shear, seismic, concrete over steel deck Vsr			1,715 (7.6)	2,880 (12.8)	Not Applicable	Not Applicable	Not Applicable	Not Applicable		
Reduction factor fo concrete over steel	r steel strength in shear, deck ⁶	φ	-			0.	65				

1. For all design cases $\Psi_{c,P} = 1.0$. For concrete compressive strength greater than 20.68 MPa [3,000 psi], N_{Pn}=(pullout strength value from table) * (specified concrete compressive strength/20.68)ⁿ. For all anchors n=1/2 with exception of the 3/8-inch-diameter anchor size, where n=1/3.

Values for N_{pdekk} are for sand-lightweight concrete (f¹c, min = 3,000 psi) and additional lightweight concrete reduction factors need not be applied. In addition, evaluation for the concrete breakout capacity in accordance with ACI 318-19 17.6.2, ACI 318-14 17.4.2 or ACI 318 D.5.2 and CSA A23.3 (-19, -14, -04) Annex D, as applicable, is not required for anchors installed in the deck soffit (flute).

3. Values for $N_{\text{p,deck,cr}}$ are applicable for seismic loading.

4. Shear loads for anchors installed through steel deck into concrete may be applied in any direction.

5. Values for V_{sa,deck} and V_{sa,deck} are for sand-lightweight concrete (I⁺c, min = 20.68 MPa [3,000 psi]) and additional lightweight concrete reduction factors need not be applied. In addition, evaluation for the concrete breakout capacity in accordance with ACI 318-19 17.7.2, ACI 318-14 17.5.2 or ACI 318 D.6.2 and CSA A23.3 (-19, -14, -04) Annex D, as applicable and the pryout capacity in accordance with ACI 318-14 17.5.3 or ACI 318-11 D.6.3 and CSA A23.3 (-19, -14, -04) Annex D, as applicable, is not required for anchors installed in the deck soffit (flute).

6. All values of φ were determined from the load combinations of IBC Section 1605.2, ACI 318 (-19 or -14) Section 5.3 or ACI 318-11 Section 9.2 and CSA A23.3 (-19, -14, -04) Annex D, as applicable. If the load combinations of ACI 318-11 Appendix C are used, then the appropriate value of φ must be determined in accordance with ACI 318-11 D.4.4.

7. Anchors shall have an axial spacing along the flute soffit equal to the greater of $3h_{eff}$ or 1.5 times the flute width.

Tension and Shear Design Strengths for Power-Stud+ SD2 in Cracked Concrete^{1,2,3,4,5,6}

					Minim	um Concrete C	ompressive St	rength			
Nominal Anchor	Nominal Embed.	f'c = 17	7.2 Mpa	f'c = 20).7 Mpa	f'c = 27	.6 Mpa	f'c = 41	.4 Mpa	f'c = 55	5.2 Mpa
Diameter (in.)	hnom (mm.)	Nr Tension (kN)	Vr Shear (kN)								
3/8	60	6.2	6.9	6.6	7.5	7.3	8.7	8.4	8.9	9.2	8.9
1/0	64	6.9	6.9	7.5	7.5	8.7	8.7	10.7	10.7	12.3	12.3
1/2	95	12.7	13.6	13.9	13.6	16.0	13.6	19.7	13.6	22.7	13.6
E /0	98	14.3	17.4	15.7	19.1	18.1	22.0	22.1	27.0	25.6	28.8
0/C	124	21.2	28.8	23.2	28.8	26.8	28.8	32.9	28.8	37.9	28.8
2/4	114	17.5	31.3	19.2	34.3	22.1	35.8	27.1	35.8	31.3	35.8
3/4	143	22.8	35.8	25.0	35.8	28.9	35.8	35.4	35.8	40.8	35.8

🔲 - Anchor Pullout/Pryout Strength Controls 🔲 - Concrete Breakout Strength Controls 📕 - Steel Strength Controls

Tension and Shear Design Strengths for Power-Stud+ SD2 in Uncracked Concrete^{1,2,3,4,5,6}

					Minim	um Concrete C	ompressive St	rength			
Nominal Anchor	Nominal Embed.	f'c = 17	7.2 Mpa	f'c = 20).7 Mpa	f'c = 27	.6 Mpa	f'c = 41	.4 Mpa	f'c = 55	i.2 Mpa
Diameter (in.)	hnom (mm.)	Nr Tension (kN)	Vr Shear (kN)								
3/8	60	8.0	8.9	8.8	8.9	10.1	8.9	12.4	8.9	14.3	8.9
1/0	64	9.8	9.8	10.8	10.8	12.4	12.4	15.2	13.6	17.6	13.6
1/2	95	19.1	13.6	20.9	13.6	24.2	13.6	29.6	13.6	31.6	13.6
E /0	86	20.4	24.4	22.4	26.7	25.8	28.8	31.6	28.8	36.5	28.8
5/8	117	30.3	28.8	33.2	28.8	38.3	28.8	39.6	28.8	39.6	28.8
2/4	102	25.0	35.8	27.4	35.8	31.6	35.8	38.7	35.8	44.7	35.8
3/4	143	38.6	35.8	42.3	35.8	48.9	35.8	59.9	35.8	64.2	35.8

🔲 - Anchor Pullout/Pryout Strength Controls 🔲 - Concrete Breakout Strength Controls 📕 - Steel Strength Controls

1- Tabular values are provided for illustration and are applicable for single anchors installed in normal-weight concrete with minimum slab thickness, $h_a = h_{min}$, and with the following conditions:

- c_{a1} is greater than or equal to the critical edge distance, c_{ac} (table values based on $c_{a1} = c_{ac}$).

- C_{a2} is greater than or equal to 1.5 times C_{a1} .

2- Calculations were performed following methodology in CSA A23.3 (-19,-14,-04), Annex D. The load level corresponding to the failure mode listed (e.g. For tension: steel, concrete breakout and pullout; For shear: steel, concrete breakout and pryout). Furthermore, the capacities for concrete breakout strength in tension and pryout in shear are calculated using the effective embedment values, hef, for the selected anchors as noted in the design information tables. Please also reference the installation specifications for more information.

3- Material Resistance factors (ø) are in accorance with CSA A23.3 (-19,-14,-04) Annex D, Sections 8.4.2 and 8.4.3. Mondification Factors (R) are in accordance with CSA A23.3 (-19,-14,-04) Annex D, Section D.5.3 Condition B; it is assumed that supplementary reinforcement not present. Material resistance factors for steel strength are taken as 0.85 for tension and shear; Modification factors are taken as 0.80 for Tension and 0.75 for Shear; values correspond to ductile steel elements.

4- Tabular values are permitted for short-term static loads only, seismic loading is not considered with these tables.

5- For designs that include combined tension and shear, the interaction of tension and shear loads must be calculated in accordance with CSA A23.3 (-19,-14,-04) Annex D, Section D.8.

6- Interpolation is not permitted to be used with the tabular values. For intermediate base material compressive strengths, please see CSA A23.3 Annex D and information contained in this product supplement. For other design conditions including seismic considerations please see CSA A23.3 Annex D.

PERFORMANCE DATA (ASD)

Converted Allowable Loads for Power-Stud+ SD2 in Cracked Concrete^{1,2}

					Minim	um Concrete C	compressive St	trength			
Nominal	Nominal Embed	f'c = 1	7.2 Mpa	f'c = 2	D.7 Mpa	f'c = 2	7.6 Mpa	f'c = 4	1.4 Mpa	f'c = 5	5.2 Mpa
Size (in.)	hnom (mm)	Tallowable,ASD Tension (kN)	Vallowable,ASD Shear (kN)								
3/8	60	4.5	4.9	4.7	5.4	5.2	6.2	6.0	6.3	6.6	6.3
1/0	64	4.9	4.9	5.4	5.4	6.2	6.2	7.6	7.6	8.8	8.8
1/2	95	9.1	9.7	9.9	9.7	11.5	9.7	14.1	9.7	16.2	9.7
E /0	98	10.2	12.4	11.2	13.6	12.9	15.7	15.8	19.3	18.3	20.6
5/6	124	15.1	20.6	16.6	20.6	19.2	20.6	23.5	20.6	27.1	20.6
2/4	114	12.5	22.4	13.7	24.5	15.8	25.5	19.4	25.5	22.4	25.5
3/4	143	16.3	25.5	17.9	25.5	20.6	25.5	25.3	25.5	29.2	25.5

1. Allowable load values are calculated using a conversion factor, α , from the Factored Resistances and conditions shown on the previous page.

2. Tabulated allowable load values assume 50% dead load and 50% live load, with controlling load combination 1.2D + 1.6L. Calculated weighted average for the conversion factor α : 1.2(0.5) + 1.6(0.5) = 1.4.

	Nominal				Minim	um Concrete C	ompressive St	rength			
Nominal Anchor	Nominal Embed.	f'c = 17	7.2 Mpa	f'c = 20).7 Mpa	f'c = 27	7.6 Mpa	f'c = 41	I.4 Mpa	f'c = 55	i.2 Mpa
Size (in.)	h (mm)	Tallowable,ASD Tension (kN)	Vallowable,ASD Shear (kN)								
3/8	60	5.7	6.3	6.3	6.3	7.2	6.3	8.9	6.3	10.2	6.3
1/0	64	7.0	7.0	7.7	7.7	8.9	8.9	10.9	9.7	12.6	9.7
1/2	95	13.7	9.7	15.0	9.7	17.3	9.7	21.2	9.7	22.6	9.7
E /0	98	14.6	17.4	16.0	19.1	18.4	20.6	22.6	20.6	26.1	20.6
5/6	124	21.6	20.6	23.7	20.6	27.4	20.6	28.3	20.6	28.3	20.6
2/4	114	17.9	25.5	19.6	25.5	22.6	25.5	27.7	25.5	31.9	25.5
3/4	143	27.9	25.5	30.2	25.5	34.9	25.5	42.8	25.5	45.9	25.5

Converted Allowable Loads for Power-Stud+ SD2 in Uncracked Concrete^{1,2}

1. Allowable load values are calculated using a conversion factor, α , from the Factored Resistances and conditions shown on the previous page.

2. Tabulated allowable load values assume 50% dead load and 50% live load, with controlling load combination 1.2D + 1.6L. Calculated weighted average for the conversion factor α : 1.2(0.5) + 1.6(0.5) = 1.4.

Ultimate and Allowable Load Capacities for Power-Stud+ SD2 in Grouted Filled Concrete Masonry^{1,23}



Neminal	Installation	Minimum		Minim	um Masonry C f'm = 1,500 p			
Anchor Size in.	Torque Tinst ftIb. (N-m)	Embedment Depth (mm)	Installation Location ³	Ulimate Load Tension Ibs. (kN)	Allowable Load Tension Ibs. (kN)	Ulimate Load Shear Ibs. (kN)	Allowable Load Shear Ibs. (KN)	Minimum Fini H
3/8	20 (27)	2-1/2 (51)	Wall Face or End Min. 2-1/2" Edge and End Distances	1,670 (7.4)	335 (1.5)	2,075 (9.2)	415 (1.8)	Grout Filled CRAU (199) Mortar Jaint
1/2	40 (54)	2-1/2 (51)	Wall Face or End Min. 3" Edge and End Distances	2,295 (10.2)	460 (2.0)	1,310 (5.8)	260 (1.2)	Wall Face Permissible Anchor Locations (Un-hatched Area)
		3-3/4 (95)	Top of Wall Min. 1-3/4" Edge and 4" End Distances	3,320 (14.8)	665 (3.0)	1,140 (5.1)	230 (1.0)	

1. Tabulated load values are for anchors installed in minimum 6-inch wide, minimum Grade N, Type II, lightweight, medium-weight or normal-weight concrete masonry units conforming to ASTM C90. Mortar must be Type N, S or M. Masonry compressive strength must be at the specified minimum at the time of installation.

2. Allowable load capacities listed are calculated using and applied safety factor of 5.0. Consideration of safety factors of 10 or higher may be necessary depending upon the application such as life safety.

3. Anchor installations into grouted masonry walls are limited to one per masonry cell. The tabulated values are for anchors installed at a minimum of 16 anchor diameters on center for 100 percent capacity.

ORDERING INFORMATION

Power-Stud+ SD2 (Carbon Steel Body with Stainless Steel Expansion Clip)

	Approx. Thread Length	Pack Qty.	Carton Qty.	Suggested ANSI Carbide Drill Bit Cat. No.								
Anchor Size				Full Head SDS-Plus	SDS-Plus	SDS-Max	Hollow Bit SDS-Plus	Hollow Bit SDS-Max				
3/8" x 3"	1-3/4"	50	300	DW5527	DW5427	-	-	-				
3/8" x 3-1/2"	2-1/4"	50	300	DW5527	DW5427	-	-	-				
3/8" x 3-3/4"	2-1/2"	50	300	DW5527	DW5427	-	-	-				
3/8" x 5"	3-3/4"	50	300	DW55300	DW5429	-	-	-				
1/2" x 3-3/4"	2-1/8"	50	200	DW5537	DW5437	DW5803	DWA54012	-				
1/2" x 4-1/2"	2-7/8"	50	200	DW5539	DW5438	DW5803	DWA54012	-				
1/2" x 5-1/2"	3-7/8"	50	150	DW5539	DW5438	DW5803	DWA54012	-				
1/2" x 7"	5-3/8"	25	100	DW5539	DW5438	DW5803	DWA54012	-				
1/2" x 8-1/2"	6-7/8"	25	100	DW5539	DW5439	DW5804	DWA54012	-				
5/8" x 4-3/4"	2-7/8"	25	100	-	DW5446	DW5806	DWA54058	DWA58058				
5/8" x 5"	3-1/8"	25	50	-	DW5446	DW5806	DWA54058	DWA58001				
5/8" x 6"	4-1/8"	25	75	-	DW5446	DW5806	DWA54058	DWA58001				
5/8" x 7"	5-1/8"	25	75	-	DW5447	DW5806	DWA54058	DWA58001				
5/8" x 8-1/2"	6-5/8"	25	75	-	DW5447	DW5809	DWA54058	DWA58001				
3/4" x 5-1/2"	3-1/4"	20	60	-	DW5453	DW5810	DWA54074	DWA58034				
3/4" x 6-1/4"	4"	20	60	-	DW5455	DW5810	DWA54074	DWA58034				
3/4" x 7"	4-3/4"	20	60	-	DW5455	DW5810	DWA54074	DWA58034				
3/4" x 8-1/2"	6-1/4"	10	40	-	DW5455	DW5812	DWA54074	DWA58034				
	Anchor Size 3/8" x 3" 3/8" x 3-3/4" 3/8" x 3-3/4" 3/8" x 5" 1/2" x 3-3/4" 1/2" x 4-1/2" 1/2" x 5-1/2" 1/2" x 7" 1/2" x 8-1/2" 5/8" x 4-3/4" 5/8" x 5" 5/8" x 6" 5/8" x 7" 5/8" x 8-1/2" 3/4" x 5-1/2" 3/4" x 7" 3/4" x 8-1/2"	Anchor Size Approx. Thread Length 3/8" x 3" 1-3/4" 3/8" x 3-1/2" 2-1/4" 3/8" x 3-3/4" 2-1/2" 3/8" x 5" 3-3/4" 1/2" x 3-3/4" 2-1/2" 3/8" x 5" 3-3/4" 1/2" x 3-3/4" 2-1/8" 1/2" x 5-1/2" 3-7/8" 1/2" x 5-1/2" 3-7/8" 1/2" x 7" 5-3/8" 1/2" x 8-1/2" 6-7/8" 5/8" x 4-3/4" 2-7/8" 5/8" x 5" 3-1/8" 5/8" x 6" 4-1/8" 5/8" x 7" 5-1/8" 5/8" x 8-1/2" 6-5/8" 3/4" x 5-1/2" 3-1/4" 3/4" x 6-1/4" 4" 3/4" x 7" 4-3/4" 3/4" x 8-1/2" 6-1/4"	Anchor Size Approx. Thread Length Pack Qty. 3/8" x 3" 1-3/4" 50 3/8" x 3-1/2" 2-1/4" 50 3/8" x 3-3/4" 2-1/2" 50 3/8" x 3-3/4" 2-1/2" 50 3/8" x 3-3/4" 2-1/2" 50 3/8" x 5" 3-3/4" 50 1/2" x 3-3/4" 2-1/8" 50 1/2" x 3-3/4" 2-1/8" 50 1/2" x 5-1/2" 3-7/8" 50 1/2" x 8-1/2" 6-7/8" 25 5/8" x 4-3/4" 2-7/8" 25 5/8" x 4-3/4" 2-7/8" 25 5/8" x 5" 3-1/8" 25 5/8" x 6" 4-1/8" 25 5/8" x 8-1/2" 6-5/8" 25 3/4" x 5-1/2" 3-1/4" 20 3/4" x 6-1/4" 4" 20 3/4" x 7" 4-3/4" <t< td=""><td>Anchor Size Approx. Thread Length Pack Qty. Carton Qty. 3/8" x 3" 1-3/4" 50 300 3/8" x 3-1/2" 2-1/4" 50 300 3/8" x 3-3/4" 2-1/2" 50 300 3/8" x 5" 3-3/4" 50 200 1/2" x 3-3/4" 2-1/8" 50 200 1/2" x 5-1/2" 3-7/8" 50 150 1/2" x 5-1/2" 3-7/8" 50 100 5/8" x 5" 3-1/8" 25 100 5/8" x 4-3/4" 2-7/8" 25 100 5/8" x 4-3/4" 2-7/8" 25 100 5/8" x 4-3/4" 2-7/8" 25 50 5/8" x 6" 4-1/8" 25 75 5/8" x 8-1/2" 6-5/8" 25<</td><td>Anchor Size Approx. Thread Length Pack Qty. Carton Qty. Full Head SDS-Plus 3/8" x 3" 1-3/4" 50 300 DW5527 3/8" x 3-1/2" 2-1/4" 50 300 DW5527 3/8" x 3-1/2" 2-1/4" 50 300 DW5527 3/8" x 3-3/4" 2-1/2" 50 300 DW5527 3/8" x 3-3/4" 2-1/2" 50 300 DW5527 3/8" x 3-3/4" 2-1/2" 50 300 DW5527 3/8" x 5" 3-3/4" 50 200 DW55300 1/2" x 4-1/2" 2-7/8" 50 200 DW5539 1/2" x 5-1/2" 3-7/8" 50 150 DW5539 1/2" x 7" 5-3/8" 25 100 DW5539 1/2" x 8-1/2" 6-7/8" 25 100 - 5/8" x 4-3/4" 2-7/8" 25 50 - 5/8" x 4-3/4" 2-7/8" 25 50 - 5/8" x 4-3/4" 2-7/8" 25</td><td>$\begin{array}{c c c c c c c c c c c c c c c c c c c$</td><td>Anchor Size Approx. Thread Length Pack Qty. Carton Qty. 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No. 3/8" x 3" 1 -3/4" 50 300 DW5527 DW5427 - - 3/8" x 3" 1 -3/4" 50 300 DW5527 DW5427 - - 3/8" x 3-1/2" 2-1/4" 50 300 DW5527 DW5427 - - 3/8" x 3-3/4" 2-1/2" 50 300 DW5527 DW5427 - - 3/8" x 3" 3-3/4" 50 300 DW5537 DW5427 - - 3/8" x 5" 3-3/4" 50 200 DW5539 DW5438 DW5803 DWA54012 1/2" x 4-1/2" 2-7/8" 50 100 DW5539 DW5438 DW5803 DWA54012 1/2" x 5-1/2" 3-7/8" 50 100 DW5539 DW5438 DW5803 DWA54012 1/2" x 7" 5-3/8" 25 100 - DW5438 DW5803 DWA54012</td></t<>	Anchor Size Approx. Thread Length Pack Qty. Carton Qty. 3/8" x 3" 1-3/4" 50 300 3/8" x 3-1/2" 2-1/4" 50 300 3/8" x 3-3/4" 2-1/2" 50 300 3/8" x 5" 3-3/4" 50 200 1/2" x 3-3/4" 2-1/8" 50 200 1/2" x 5-1/2" 3-7/8" 50 150 1/2" x 5-1/2" 3-7/8" 50 100 5/8" x 5" 3-1/8" 25 100 5/8" x 4-3/4" 2-7/8" 25 100 5/8" x 4-3/4" 2-7/8" 25 100 5/8" x 4-3/4" 2-7/8" 25 50 5/8" x 6" 4-1/8" 25 75 5/8" x 8-1/2" 6-5/8" 25<	Anchor Size Approx. Thread Length Pack Qty. Carton Qty. 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The published size includes the diameter and the overall length of the anchor.

Allow for fixture thickness (as applicable) plus one anchor diameter for the nut and washer thickness when selecting a length.

All anchors are packaged with nuts and washers.

Hollow drill bits must be used with a dust extraction vacuum (e.g. Cat. No. DW012).



POWER-STUD®+ SD2 High Performance Wedge Expansion Anchor

CHANICAL ANCHORS