## R Roseburg

## Engineered Wood Products



## Residential Design \& Installation Guide



ROSEBURG FRAMING SYSTEM®
RigidLam ${ }^{\circledR}$ LVL • RigidLam ${ }^{\circledR}$ LVL Studs•RigidLam ${ }^{\circledR}$ LVL Columns
RigidLam ${ }^{\circledR}$ LVL Stair Stringers •RigidRim ${ }^{\circledR}$ Rimboard •RFPI ${ }^{\circledR}$-Joist
USA - ALLOWABLE STRESS DESIGN

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# Conscientious Stewards Of Our Environment. 


#### Abstract

These five words are the foundation for every action Roseburg takes in its interactions with the environment. The phrase means not just taking care of the lands, but making them better for future generations. Harvesting a tree is easy; studying how our harvest activity impacts everything around it and finding ways to improve upon the environment is more difficult.

We have been up to the task. We are not only in the business of producing quality wood products, but also in the business of conserving and enhancing the wonderful natural resources that each of us enjoys. Visit any of our harvest sites, and you'll see these words in action. While using tractors and skidders may often be the easiest and least expensive alternative for removing logs, we look at other, more environmentally-friendly harvesting options such as helicopter logging to protect the soils that grow our trees. Often, you'll find us placing large, woody debris in streams to enhance the fish spawning habitat, or replacing old culverts with larger, better-placed culverts to provide better fish passage. Roseburg was among the first in the industry to set aside some of its own land in order to study and improve upon fish habitat. Several years ago, we began working with Oregon State University and other agencies on a company-owned area near the Hinkle Creek Watershed to gain current research on the effects of logging on fish. We are now lobbying other companies to replicate the study on their own lands.

Finally, it's important to note that we are a highly self-sufficient manufacturer. We now own more than 600,000 acres of timberland, which supply the majority of wood fiber we need to produce our products. The ability to rely on our own forests gives us the flexibility to match our resources to our product mix. We take a great deal of pride in our partnership with the natural world. However, we don't go to all of this effort and expense simply because it makes us feel good; we do it because it's the right thing to do.


- We manage our natural resources in a responsible manner
- Our EWP products enable builders to use timber resources more efficiently
- We offer composite panels and plywood products that have no added urea formaldehyde
- We have biomass cogeneration plants which use wood waste material from our mills to produce clean energy for our plants and nearby communities
- We produce a broad array of products that are SCS and EPP certified
- Our integrated manufacturing facilities dramatically reduce vehicle carbon emissions
- We plant over 5 million tree seedlings annually
- We are progressively involved in stream research and enhancement


## Design Support

The various charts and tables in this literature are based on accepted, typical loading conditions, on center spacing, deflection criteria and/or spans. This printed information allows the end user to identify and install properly sized RFP engineered wood products without the need for specific design or engineering calculations. Design software; however, such as Simpson Strong$\mathrm{Tie}^{\circledR}$ Component Solutions ${ }^{\text {TM }}$, allows the user to input project specific information into the software which may give a less restrictive solution than the generic information in the printed literature. Rest assured that both the literature and the Component Solutions ${ }^{T M}$ software are based on the appropriate design properties listed in the current code reports.

For additional assistance with specific product design questions, product availability, and Roseburg representative locations, please visit our website at www.Roseburg.com, or contact Roseburg Forest Products at 1-800-347-7260, or at the address listed on the back cover.

## Important

All Roseburg Engineered Wood Products are intended and warranted for use in dry-service conditions (i.e. where the average equilibrium moisture content of solid-sawn lumber is less than 16\%).

## Roseburg Engineered Wood Products



## ENGINEERED WOOD PRODUCTS

Roseburg's engineered wood plants are located in Riddle, Oregon and Chester, South Carolina. These state-of-the-art facilities are focused on ensuring the highest quality standards are maintained.

Roseburg's signature trademarks of vertical integration capabilities and cutting-edge manufacturing practices help ensure that quality Engineered Wood Products are produced. Our production capacity, complete product offering, focus on service and product availability, commitment to the EWP business, and acceptability of the product by builders and homeowners all translate into significant advantages for our clients.

## ROSEBURG FRAMING SYSTEM®

The Roseburg Framing System ${ }^{\circledR}$ consists of: RFPI ${ }^{\circledR}$ Joists used in floor and roof construction; RigidLam ${ }^{\circledR}$ LVL which is used for headers, beams, studs, columns, stair stringers, and RigidRim ${ }^{\circledR}$ Rimboard. All of the components are engineered to the industry's highest standards to help contractors build solid, durable, and better performing framing systems compared to ordinary dimension lumber.
As an acting member of APA-The Engineered Wood Association, Roseburg has adopted the Performance Standard for wood I-Joists, the Performance Standard for rimboard and the Performance Standard for laminated veneer lumber (LVL). Adherence to the strict APA quality standards assures Roseburg engineered wood product quality and consistency for the market. All engineered wood products described in this document meet the APA standards.

This guide emphasizes residential applications, including technical information on span ratings, installation details, cantilever designs, architectural specifications and engineering design properties. However, much of the basic information can be used for other construction applications. Review by a design professional is required for applications beyond the scope of this document. The Roseburg Framing System ${ }^{\circledR}$, combined with other wood components produced by Roseburg, offers one of the most complete framing packages available from a single manufacturing supplier today.

## What does roseburg's ewp PROGRAM HAVE TO OFFER?

- Dependable supply of engineered wood
- Experienced sales, technical, engineering and customer service teams
- A commitment to quality and predictable performance
- A complete framing package with RFPI-Joists, RigidLam LVL, and RigidRim Rimboard.


## THE COMPANY

Since 1936, Roseburg has served the industry providing quality products for residential, commercial, industrial applications. Our natural resource base, state-of-the-art manufacturing facilities, talented and experienced associates, and reputation for quality products and service have been keys to our clients' success.
Integrated manufacturing, wide variety of wood products, and over 600,000 acres of forestlands throughout Southern Oregon, North Carolina and Virginia are assets that will support our strategic growth plans well into the 21st Century.

## Software Tools

Roseburg offers a software tool that will aid you in generating accurate, professional layout drawings and member calculations. This software tool includes the Component Solutions ${ }^{\text {Tw }}$ (CS) EWP Studio Software Suite provided by Simpson Strong-Tie ${ }^{\circledR}$.
As a supplier of connectors for engineered wood products, Simpson Strong-Tie has been involved in the structural building industry for decades. This experience has provided invaluable insights into the needs of designers and suppliers, resulting in the latest addition to the Simpson Strong-Tie ${ }^{\circledR}$ software product line for light-frame construction. Choose Simpson Strong-Tie ${ }^{\circledR}$ Component Solutions ${ }^{\text {TM }}$ EWP Studio ${ }^{\text {TM }}$ for your EWP design needs.

## COMPONENT SOLUTIONS ${ }^{T M}$ EWP STUDIOTM

CS EWP Studio is a state-of-the art EWP analysis program. Whether you are looking for a single-member sizing utility or a robust layout and design solution, CS EWP Studio offers a wide range of tools and functions to meet your design, supply and reporting needs.

## DESIGN TOOL

The Design tool is a powerful yet easy-to-use single-member sizing feature that enables you to size Roseburg engineered wood products for almost any structural condition. You provide a description of the spans, supports and loads of a specific sizing problem, and CS EWP Studio will deliver pass/fail information and even present you with a list of multiple product solutions. After selecting a product, you can print out a professional, easy-to-read calc sheet.
The program designs RFPI ${ }^{\circledR}$-Joists at their optimum on-center spacing and RigidLam ${ }^{\circledR}$ LVL beams at their optimum depth. Rectangular or circular holes can be analyzed for RFPI Joists and circular holes can be analyzed for RigidLam ${ }^{\circledR}$ LVL at a given size and location. Cantilever reinforcements can be utilized for RFP1 ${ }^{\circledR}$-Joists used in load-bearing cantilever applications.

RigidLam ${ }^{\circledR}$ LVL columns and studs can be sized using any combination of axial and lateral loading and a variety of default and custom bracing conditions for individual stud and column members.

## PLAN TOOL

The Plan tool is the complete automation system for Roseburg engineered wood products. The Plan tool software is available to qualified users who use the software to promote and support the sale of Roseburg engineered wood products. The Plan tool includes all of the analysis functionality within the Design tool as well as additional features for creating a 3D model, defining floor and roof systems, generating layouts, and reporting. With this effective tool, the designer describes the building geometry and specifies the framing layout while the software does the analysis, including the following:

- Developing loads throughout the structure
- Sizing all framing members for Roseburg engineered wood products
- Specifying hangers
- Generating placement plans
- Generating material cut lists and hanger schedules

Simpson Strong-Tie provides all training and software support necessary to successfully learn and implement these software programs. To obtain the single-member sizing software, "EWP Studio - Design Tool," or the layout and design software, "EWP Studio - Plan Tool", please contact your local Roseburg representative.


## Explanation Of Important EWP Terms

1. Live Load, Dead Load \& Total Load: Most people would feel very uncomfortable in buildings if there were no consideration to deflection or sag even though they were designed to safely support their total design load. That's because all structures (buildings, bridges, floors, etc.) can safely deflect well beyond the limits that make us feel uncomfortable. Limiting deflection is considered a "serviceability" requirement because it is independent of strength. In floor design, limiting sag is also necessary to prevent cracking in the sheet rock (on the bottom of the joists) due to load being applied and removed during the day.
To do this, it is necessary to define that portion of the load that varies and that portion of the load that is always present. By definition, Live Load is people, furniture and pets etc. that can be moved on and off the floor. Dead Load is defined as the weight of the floor system itself or any other load that is permanently attached to the floor. Together, the dead load and the live load make up the total load.
2. $\mathbf{L} / \mathbf{3 6 0}, \mathrm{L} / \mathbf{4 8 0}$ : A method used to limit the maximum allowable deflection (or sag) when designing joists and beams. Specifically, the term L is the span of the joist or beam expressed in inches and the ratio $\mathrm{L} / 360$ would be the maximum building code allowable deflection the joist would be expected to deflect. It does not represent what the actual deflection of the joist is in the field, just the maximum value it would be allowed to deflect under full design load. L/480 is an industry standard ratio for floor systems which exceeds building code requirements.
The "L over" ratio is always associated with either live load or total load. The most common values are:

| Floors: | Live Load - L/480 (or L/360) | Total Load - L/240 |
| :--- | :--- | :--- |
| Roofs: | Live Load - L/240 | Total Load - L/180 |

For example, a typical residential floor (40 psf LL / 10 psf DL) with RFPI-Joists would be designed to an L/480 Live Load limit and an L/240 Total Load limit. For an 18' span, this would be equivalent to:
$\frac{L}{480}=\frac{18^{\prime} \times 12}{480}=\frac{216}{480}=0.45^{\prime \prime}$ Allowable Live Load Deflection And $\frac{L}{240}=\frac{18^{\prime} \times 12}{240}=\frac{216}{240}=0.90^{\prime \prime}$ Allowable Total Load Deflection
The actual Live Load deflection of the floor system would be determined with a surveyor's transit taking readings before and after a true 40 psf load (i.e., cinder blocks) was applied. The deflection reading obtained in the field must be less than (or equal to) the $0.45^{\prime \prime}$. The same applies to the $0.9^{\prime \prime}$ under a true 50 psf load.
3. PSF Load: This is the design load, in pounds per square foot that is "applied" to the entire floor or roof area. By code, most residential floors must be designed to support a live load of 40 psf . The live load for roofs is determined by local code and depends on the amount of annual snow expected for that region where the house is.
The design dead load psf is determined by the weight of each component of the floor or roof. A typical residential floor will have a dead load of 10 psf but depending on the components used, it can be as high as 20-24 psf. Dead load psf is based on standard material weights found in any of the National Model Building Codes. A typical method for calculating dead load is shown below:

## DEAD LOAD CALCULATION FOR TYPICAL RESIDENTIAL FLOOR



## TYPICAL BUILDING MATERIAL WEIGHTS

| Floors |  |
| :---: | :---: |
| Hardwood-1" thick | 4.0 psf |
| Concrete-1" thick |  |
| Regular | 12.0 psf |
| Lightweight | 8.0-12.0 psf |
| Gypcrete-3/4" thick | 6.5 psf |
| Sheet vinyl | 0.5 psf |
| Carpet and pad | 1.0 psf |
| $3 / 4$ " ceramic or quarry tile | 10.0 psf |
| Linoleum or soft tile | 1.5 psf |
| 1/2" mortar bed | 6.0 psf |
| 1 " mortar bed | 12.0 psf |
| Ceilings |  |
| Acoustical fiber tile | 1.0 psf |
| 1/2" gypsum board | 2.2 psf |
| 5/8" gypsum board | 2.8 psf |
| Plaster-1" thick | 8.0 psf |
| Metal suspension system (including tile) | 1.8 psf |


| Insulation - 1" Thick |  |
| :--- | ---: |
| Polystyrene foam \& Styrofoam | 0.2 psf |
| Foamglass | 0.8 psf |
| Rigid fiberglass | 1.5 psf |
| Glass wool | 0.1 psf |
| Rock wool | 0.2 psf |
|  |  |
| Douglas-fir Sheathing | 1.5 psf |
| 1/2" plywood | 1.8 psf |
| 5/8" plywood | 2.3 psf |
| 3/4" plywood | 1.7 psf |
| 1/2" OSB | 2.0 psf |
| 5/8" OSB | 2.5 psf |
| 3/4" OSB | 2.9 psf |
| 7/8" OSB |  |
| Miscellaneous |  |
| Mechanical ducts | $2.0-4.0 \mathrm{psf}$ |
| Stucco - 1" thick | 10.0 psf |


| Roofing Materials |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Asphalt shingles |  | 2.5 psf |  |  |
| Wood shingles |  | 2.0 psf |  |  |
| Clay tile |  | 9.0-14.0 psf |  |  |
| Slate-3/8" thick |  | 15.0 psf |  |  |
| Weights of Douglas-fir Framing - PSF |  |  |  |  |
| Nomina Size | Joist Spacing |  |  |  |
|  | 12" | 16" | 19.2" | 24" |
| 2x4 | 1.4 | 1.1 | 0.9 | 0.7 |
| $2 \times 6$ | 2.2 | 1.7 | 1.4 | 1.1 |
| 2x8 | 2.9 | 2.2 | 1.8 | 1.5 |
| Weights of Sprinkler Lines |  |  |  |  |
| Pipe Size | Schedule 40 |  | Schedule 10 |  |
|  | Dry (plf) | Wet (plf) | Dry (plf) | Wet (plf) |
| 1" | 1.7 | 2.1 | 1.4 | 1.8 |
| 1-1/2" | 2.7 | 3.6 | 2.1 | 3.1 |
| 2 " | 3.7 | 5.2 | 2.7 | 4.2 |

## Floor System Performance

It is always a good idea to consider the performance (i.e., vibration, bounce etc.) of any floor system. Currently, there are no true industry standard guidelines to use for l-joists but there are several practical aids that have shown to be useful. Some are design aids, some are installation aids and some are retrofit aids. They are offered as tools to help you minimize complaints about floor performance but cannot be guaranteed to eliminate all floor performance problems.
Begin by using the concepts of fundamental natural frequency and damping when designing floor systems. The fundamental natural frequency (FNF) is a measure of how the floor vibrates when you walk on it and is measured in cycles per second (called a Hertz or Hz). Damping is a measure of how quickly a floor stops vibrating and is expressed as a percent between 1 and 100 (most residential floors have a range between $5 \%-25 \%$ damping).
Our bodies are extremely sensitive to vibrations below 9 Hz so the ideal floor would have a high FNF with high damping. Most problem floors have a combination of a low FNF (below 9 Hz ) and a low damping (around 5\%). The following list will help you determine the effect of different parameters on floor performance. It is the combination and interaction of these parameters that determines how the floor "feels".

## DESIGN PARAMETERS

Longer Spans
Higher "L over" deflection limit (L/480 vs. Code Minimum L/360)
Using an absolute upper limit on live load deflection (Usually between 1/3" to 1/2" max)
Using deeper l-joists
Reduced on-center spacing
Adding perpendicular partition walls
Increasing overall weight of floor

Using an absolute upper limit on live load deflection (Usually between 1/3" to 1/2" max)
Using deeper I-joists
Reduced on-center spacing

Increasing overall weight of floor

INSTALLATION PARAMETERS

| Unlevel bearings (walls, beams \& hangers) |
| :--- |
| Direct applied sheet-rock ceiling |
| Thicker sub-floor |
| Screw \& Glued sub-floor |
| T\&G sub-floor |

## RETROFIT PARAMETERS

| I-joist mid span blocking (one row) |
| :--- |
| $2 \times 4$ flat on I-joist bottom (perpendicular) |
| $2 \times 4$ strong back on I-joist bottom ( $2 \times 4$ nailed to side of flat perpendicular) |

## EFFECT ON FNF EFFECT ON DAMPING

significantly lowers significantly increases significantly increases increases increases little or no effect significantly lowers
little or no effect little or no effect little or no effect little or no effect little or no effect significantly increases significantly increases

| significantly lowers | significantly lowers |
| :--- | :--- |
| significantly increases | significantly increases |
| increases | increases |
| increases | increases |
| increases | increases |
|  |  |
| little or no effect | increases |
| little or no effect <br> increases | increases |
| significantly increases |  |

## 2x10 \& 2x12 Floor Joist Comparison

## RFPIO-JOIST SUBSTITUTION GUIDE FOR SOLID-SAWN LUMBER ${ }^{(1)}$

| 2x10 No. 2 Solid-Sawn Live Load Deflection $=$ L/360 |  | Simple Span |  |  | Multiple Span |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 9-1/2" RFPI Joist Live Load Deflection = L/480 |  |  | 9-1/2" RFPI Joist Live Load Deflection = L/480 |  |  |
| Species | Maximum Simple Span @16" o.c. ${ }^{(2)}$ | 16" o.c. | 19.2" o.c. | 24" o.c. | 16" o.c. | 19.2" o.c. | 24" o.c. |
| Spruce-Pine-Fir | 15'-5" | RFPI 20 | RFPI 40S/400 | RFPI 70 | RFPI 20 | RFPI 20 | RFPI 400/40 |
| Hem-Fir | 15'-2" | RFPI 20 | RFPI 40S/400 | RFPI 60S/70 | RFPI 20 | RFPI 20 | RFPI 400/40 |
| Douglas-fir-Larch | 15'-7" | RFPI 20 | RFPI 40/60S | RFPI 70 | RFPI 20 | RFPI 20 | RFPI 400/40 |
| Southern Pine | 14'-0" | RFPI 20 | RFPI 20 | RFPI 40S/400 | RFPI 20 | RFPI 20 | RFPI 20 |
|  |  |  |  |  |  |  |  |
| 2x12 No. 2 Solid-Sawn Live Load Deflection = L/360 |  | Simple Span |  |  | Multiple Span |  |  |
|  |  | 11-7/8" RFPI Joist Live Load Deflection = L/480 |  |  | 11-7/8" RFPI Joist Live Load Deflection = L/480 |  |  |
| Species | Maximum Simple Span @16" o.c. ${ }^{(2)}$ | 16" o.c. | 19.2" o.c. | 24" o.c. | 16" o.c. | 19.2" o.c. | 24" o.c. |
| Spruce-Pine-Fir | 17'-10" | RFPI 20 | RFPI 40S/400 | RFPI 60S/70 | RFPI 20 | RFPI 40S | RFPI 40/60S |
| Hem-Fir | 17'-7" | RFPI 20 | RFPI 20 | RFPI 40/60S | RFPI 20 | RFPI 40S | RFPI 400 |
| Douglas-fir-Larch | 18'-1" | RFPI 20 | RFPI 40S/400 | RFPI 60S/70 | RFPI 20 | RFPI 40S | RFPI 40/60S |
| Southern Pine | 16'-6" | RFPI 20 | RFPI 20 | RFPI 20 | RFPI 20 | RFPI 20 | RFPI 40S |

[^0]
## Safety \& Construction Precautions

WARNING: l-joists and LVL beams are not stable until completely installed, and will not carry any load until fully braced and sheathed

## AVOID ACCIDENTS BY FOLLOWING THESE IMPORTANT GUIDELINES:

1. Brace and nail each l-joist as it is installed, using hangers, blocking panels, rimboard, and/or cross-bridging at joist ends.
2. When the building is completed, the floor sheathing will provide lateral support for the top flanges of the I-joists. Until this sheathing is applied, temporary bracing, often called struts, or temporary sheathing must be applied to prevent I-joist rollover or buckling.
3. Temporary bracing or struts must be $1 \times 4$ inch minimum, at least eight feet long, spaced no more than eight feet on center, and must be secured with a minimum of two 8d nails fastened to the top surface of each l-joist. Nail bracing to a lateral restraint at the end of each bay. Lap ends of adjoining bracing over at least two l-joists.
Or, sheathing (temporary or permanent) can be nailed to the top flange of the first four feet of 1 -joists at the end of the bay.
4. For cantilevered I-joists, brace top and bottom flanges, and brace ends with closure panels, rimboard, or cross-bridging.
5. Install and nail permanent sheathing to each l-joist before placing loads on the floor system. Then, stack building materials over beams or walls only. See APA Technical Note number J735C "Temporary Construction Loads Over I-Joist Roofs and Floors" for additional information regarding proper stacking of building materials.

## 6. NEVER INSTALL A DAMAGED I-JOIST OR LVL MEMBER.

Improper storage or installation, failure to follow applicable building codes, failure to follow span ratings for RFPI ${ }^{\circledR}$ Joists or RigidLam ${ }^{\circledR}$ LVL, failure to properly use allowable hole sizes and locations, or failure to use web stiffeners when required can result in serious accidents. Follow these installation guidelines carefully.
These are general recommendations and in some cases additional precautions may be required.

## Storage \& Handling Guidelines

- Do not drop I-joists or LVL off the delivery truck. Best practice is use of a forklift or boom.
- Store bundles upright on a smooth, level, well-drained supportive surface.
- Do not store I-joists or LVL in direct contact with the ground. Bundles should be a minimum of 6 " off the ground and supported every 10' or less.
- Always stack and handle I-joists in their upright position only.
- Place $2 x$ or LVL spacers (at a maximum of 10 ' apart) between bundles stored on top of one another. Spacers above should be lined up with spacers below.
- Bundles should remain wrapped, strapped, and protected from the weather until time of installation.
- Do not lift l-joist bundles by top flange.
- Avoid excessive bowing or twisting of I-joists or LVL during all phases of handling and installation (i.e. measuring, sawing or placement). Never load I-joists in the flat-wise orientation.
- Take care to avoid forklift damage. Reduce forklift speed to avoid "bouncing" the load.
- When handling l-joists with a crane ("picking"), take a few simple precautions to prevent damage to the l-joists and injury to your work crew:
- Pick I-joists in the bundles as shipped by the supplier.
- Orient the bundles so that the webs of the I-joists are vertical.
- Pick the bundles at the 5 th points, using a spreader bar if necessary.
- Do not stack LVL bundles on top of I-Joist bundles.
- All field repairs must be approved by a Design Professional.
- NEVER USE A DAMAGED I-JOIST OR LVL.


Do not allow workers to walk on I-joists or LVL beams until they are fully installed and braced, or serious injuries can result.


Never stack building materials over unsheathed I-joists. Stack only over braced beams or walls.


## Engineered to Make the Job Easier

RFPIs are the ideal choice for designers and builders who want to provide their customers with high-quality floor systems. They provide consistent performance for the most demanding residential applications.

## SIMPLE TO INSTALL

I-joists save builders time, and money. I-joists are typically precut and shipped to the jobsite ready to install. This minimizes jobsite cutting and material waste. 1 -joists can be cut and fastened with traditional framing tools and fasteners - no special tools are required. Since I-joists can typically be used at greater joist spacings than lumber, fewer pieces must be cut and handled on the jobsite, making l-joist installation less costly and less wasteful for the builder.

## DESIGN FLEXIBILITY

The availability of long lengths allows multiple span installations thus speeding construction by eliminating the need to lap joists over bearing walls or support beams. This also means fewer pieces to handle. The availability of long lengths and relatively deep joists also gives designers the freedom to create more open spaces and reduces the need for supporting walls, columns, or beams.

## LIGHTWEIGHT

Because I-joists typically weigh less than half of comparable conventional framing lumber, they can be installed quickly and efficiently.

## DIMENSIONALLY STABLE

I-joists will not warp, twist, or shrink, and are more uniform in their dimensions than sawn lumber joists. The floor vibration criteria combined with their straightness and uniformity provides a stiffer, more uniform floor with fewer squeaks, and higher customer satisfaction.

## WEB HOLES

The OSB webs in Roseburg's l-joists permit holes to be easily cut on the jobsite to permit the passage of electrical wiring, plumbing and ductwork. This cannot always be accomplished with sawn lumber joists where the mechanical systems must be passed under the joist system. Roseburg also provides knockout holes along the length of the joists to facilitate the installation of electrical wiring or light plumbing lines. These knockouts can easily be removed with a hammer as needed.

## APA OUALITY ASSURED

The APA trademark ensures superior l-joist quality and consistent performance. All products are subject to the proven quality assurance program of APA.

## RESOURCE FRIENDLY

Wood I-joists use up to $50 \%$ less wood fiber in their production than conventional lumber joists, allowing more efficient use of our natural resources.

## Installation Notes

1. Engineered lumber must not remain in direct contact with concrete or masonry construction and must be used in dry use conditions only.
2. Except for cutting to length, top and bottom flanges of RFPI-Joists shall not be cut, drilled or notched.
3. Concentrated loads greater than those that can normally be expected in residential construction should only be applied to the top surface of the top flange. Normal concentrated loads include track lighting fixtures, audio equipment and security cameras. Never suspend unusual or heavy loads from the l-joist's bottom flange. Whenever possible, suspend all concentrated loads from the top of the I-joist. Or, attach the load to blocking that has been securely fastened to the l-joist web.
4. Any fastening, resistance to uplift or application not specifically detailed is subject to local approval.
5. l-joist end bearing length must be at least 1-3/4". Intermediate bearings of multiple span joists must be at least 3-1/2".
6. RFPI-Joists must be restrained against rotation at the ends of joists by use of rimboard, rim joists, blocking panels, or cross-bracing. To laterally support cantilevered joists, blocking panels must also be installed over supports nearest the cantilever.
7. Additionally, rimboard, rim joists, blocking panels, or squash blocks must be provided under all exterior walls and interior load bearing
walls to transfer loads from above to the supports below.
8. Plywood or OSB subfloor fastened to the top flange of an RFPI-Joist is adequate to provide lateral support.
9. Install I-joists so that top and bottom flanges are straight and remain within $1 / 2$ inch of true alignment.
10. Roseburg does not require mid-span blocking or bridging in RFPI floor applications.
11. RFPI-Joists are produced without camber so either flange can be the top or bottom flange; however, orienting the floor I-joists so the prescored knockouts are on the bottom may ease installation of electrical wiring or residential sprinkler systems.
12. See table below for recommended sheathing attachment with nails. If sheathing is to be attached with screws, the screw size should be equal to or only slightly larger than the recommended nail size. Space the screws the same as the required nail spacing. The unthreaded shank of the screw should extend beyond the thickness of the panel to assure that the panel is pulled securely against the l-joist flange. Use screws intended for structural assembly of wood structures. It is recommended to use screws from a manufacturer that can provide an ICC-ES Report (or similar) with approved application specifications and design values. Drywall screws can be brittle and should not be used.

## RECOMMENDED NAIL SIZE \& SPACING ${ }^{(8)}$

|  |  | Flange Face Nailing (in) ${ }^{(b)(c)}$ |  | Flange Edge Nailing (in) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Flange Material | Fastener Diameter ${ }^{(d)(e)}$ | End Distance | Nail Spacing | End Distance | Nailed to one flange edge | Nailed to both flange edges ${ }^{(f)}$ |
| LVL Flange | dia. $\leq 0.128^{\prime \prime}$ (8d box or sinker, 10d box or sinker, 12d box) | 3 | 2 | 3 | 3 | 6 |
| I-Joist | $0.128^{\prime \prime} \leq$ dia. $\leq 0.148$ " ( 8 d com, 10d com, 12d sinker or com, 16d box or sinker) | 3 | 3 | 3 | $3^{(9)}$ | $6^{(9)}$ |
| Solid Sawn | dia. $\leq 0.128$ " (8d box or sinker, 10d box or sinker, 12d box) | 2 | 2 | 2 | 2 | 4 |
| I-Joist | $0.128^{\prime \prime} \leq$ dia. $\leq 0.148$ " ( 8 d com, 10 d com, 12d sinker or com, 16d box or sinker) | 2 | 3 | 2 | 3 | 6 |

## Nailing Notes:

a. Nail spacings shown are guidelines for RFP| ${ }^{\circledR}$-Joists used in conventional framing applications. For cases where horizontal diaphragm load capacity is required, refer to Table 4 of APA Product Report ${ }^{\circledR}$ PR-L259 for allowable diaphragm loads and the applicable RFPI-Joist series, panel grade and thickness, and nail size and spacing.
b. For conventional framing, attach sheathing to RFPI-Joists in accordance with applicable building code or approved building plan. However, do not use nails larger or spaced closer than shown in the table above.
c. If more than one row of nails is required, rows must be offset by at least $1 / 2^{n}$ and staggered.
d. 14 gauge staples may be substituted for $8 \mathrm{~d}\left(2-1 / 2^{\prime \prime}\right)$ nails if staples penetrate the joist at least 1 ".
e. $10 \mathrm{~d}\left(3^{\prime \prime}\right)$ box nails may be substituted for $8 \mathrm{~d}\left(2-1 / 2^{\prime \prime}\right)$ common nails.
f. Nails on opposing flange edges must be offset one-half the minimum spacing
g. Maximum of 0.131 " diameter ( 8 d common).

## RFPI ${ }^{\circledR}$-Joist Design Properties

LVL FLANGE I-JOIST DIMENSIONS

| RFPI ${ }^{\circ} 20$ |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| $1-3 / 4^{\prime \prime}$ wide $\times 1-3 / 8^{\prime \prime}$ LVL Flange 3/8" OSB Web | $2-1 / 16^{\prime \prime}$ wide $\times 1-3 / 8^{\prime \prime}$ LVL Flange 3/8" OSB Web | $\begin{gathered} 2-5 / 16^{\prime \prime} \text { wide } \times 1-3 / 8^{\prime \prime} \text { LVL Flange } \\ 3 / 8^{\prime \prime} \text { OSB Web } \end{gathered}$ | 2-5/16" wide $\times 1-1 / 2^{\prime \prime}$ LVL Flange 3/8" OSB Web | $\begin{gathered} 3-1 / 2^{\prime \prime} \text { wide } \times 1-1 / 2^{\prime \prime} \text { LVL Flange } \\ 7 / 16^{\prime \prime} \text { OSB Web } \end{gathered}$ |

## SOLID SAWN FLANGE I-JOIST DIMENSIONS



DESIGN PROPERTIES FOR RFPI@_JOISTS(1)

| Joist Depth | Joist Series | APA Designation | $\mathrm{El}^{(2)} \times 10^{6} \mathrm{lb}-\mathrm{in}^{2}$ | $\mathrm{M}^{(3)} \mathrm{lb}-\mathrm{ft}$ | $\mathrm{V}^{(4)} \mathrm{l}$ bs | VLC ${ }^{(5)} \mathrm{lbs} / \mathrm{ft}$ | $\mathrm{K}^{(6)} \times 10^{6} \mathrm{lb}$ | Weight plf |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 9-1/2" | RFPI $20{ }^{(7)}$ | 9-1/2" PRI 20 | 165 | 2,820 | 1,220 | 2,000 | 4.94 | 2.06 |
|  | RFPI 40S ${ }^{(7)}$ | 9-1/2" PRI 40 | 193 | 2,735 | 1,120 | 2,000 | 4.94 | 2.56 |
|  | RFPI 400 | Not Applicable | 193 | 3,345 | 1,220 | 2,000 | 4.94 | 2.29 |
|  | RFPI 40 ${ }^{(7)}$ | 9-1/2" PRI 40 | 215 | 3,760 | 1,330 | 2,000 | 4.94 | 2.56 |
|  | RFPI 60S ${ }^{(7)}$ | 9-1/2" PRI 60 | 231 | 3,780 | 1,120 | 2,000 | 4.94 | 2.56 |
|  | RFPI 70 | Not Applicable | 266 | 5,130 | 1,330 | 2,000 | 4.94 | 2.57 |
|  | RFPI 90 | Not Applicable | 398 | 7,830 | 1,890 | 2,000 | 4.94 | 3.70 |
| 11-7/8" | RFPI $20{ }^{(7)}$ | 11-7/8" PRI 20 | 283 | 3,640 | 1,420 | 2,000 | 6.18 | 2.37 |
|  | RFPI 40S ${ }^{(7)}$ | 11-7/8" PRI 40 | 330 | 3,545 | 1,420 | 2,000 | 6.18 | 2.83 |
|  | RFPI 400 | Not Applicable | 330 | 4,315 | 1,480 | 2,000 | 6.18 | 2.60 |
|  | RFPI 40 ${ }^{(7)}$ | 11-7/8" PRI 40 | 366 | 4,855 | 1,550 | 2,000 | 6.18 | 2.81 |
|  | RFPI 60S ${ }^{(7)}$ | 11-7/8" PRI 60 | 396 | 4,900 | 1,420 | 2,000 | 6.18 | 2.83 |
|  | RFPI 70 ${ }^{(7)}$ | 11-7/8" PRI 70 | 455 | 6,645 | 1,550 | 2,000 | 6.18 | 2.95 |
|  | RFPI 80S ${ }^{(7)}$ | 11-7/8" PRI 80 | 547 | 6,970 | 1,590 | 2,000 | 6.18 | 3.79 |
|  | RFPI 90 ${ }^{(7)}$ | 11-7/8" PRI 90 | 676 | 10,145 | 2,050 | 2,000 | 6.18 | 4.17 |
| 14" | RFPI 20 | Not Applicable | 420 | 4,330 | 1,610 | 2,000 | 7.28 | 2.60 |
|  | RFPI 40S ${ }^{(7)}$ | 14" PRI 40 | 482 | 4,270 | 1,710 | 2,000 | 7.28 | 3.07 |
|  | RFPI 400 | Not Applicable | 486 | 5,140 | 1,710 | 2,000 | 7.28 | 2.98 |
|  | RFPI 40 ${ }^{(7)}$ | 14 " PRI 40 | 540 | 5,785 | 1,770 | 2,000 | 7.28 | 3.13 |
|  | RFPI 60S ${ }^{(7)}$ | 14" PRI 60 | 584 | 5,895 | 1,710 | 2,000 | 7.28 | 3.07 |
|  | RFPI 70 ${ }^{(7)}$ | 14" PRI 70 | 672 | 7,925 | 1,770 | 2,000 | 7.28 | 3.21 |
|  | RFPI 80S ${ }^{(7)}$ | 14 " PRI 80 | 802 | 8,390 | 1,835 | 2,000 | 7.28 | 4.03 |
|  | RFPI 90 ${ }^{(7)}$ | 14" PRI 90 | 992 | 12,100 | 2,195 | 2,000 | 7.28 | 4.51 |
| $16 "$ | RFPI 40S ${ }^{(7)}$ | 16" PRI 40 | 657 | 4,950 | 1,970 | 2,000 | 8.32 | 3.31 |
|  | RFPI 400 | Not Applicable | 665 | 5,880 | 1,970 | 2,000 | 8.32 | 3.19 |
|  | RFPI 40 ${ }^{(7)}$ | 16 " PRI 40 | 737 | 6,615 | 1,970 | 2,000 | 8.32 | 3.34 |
|  | RFPI 60S ${ }^{(7)}$ | 16 " PRI 60 | 799 | 6,835 | 1,970 | 2,000 | 8.32 | 3.31 |
|  | RFPI 70 ${ }^{(7)}$ | 16 " PRI 70 | 918 | 9,080 | 1,970 | 2,000 | 8.32 | 3.48 |
|  | RFPI 80S ${ }^{(7)}$ | 16 " PRI 80 | 1,092 | 9,730 | 2,070 | 2,000 | 8.32 | 4.26 |
|  | RFPI 90 ${ }^{(7)}$ | 16 " PRI 90 | 1,350 | 13,865 | 2,330 | 2,000 | 8.32 | 4.80 |

1. The tabulated values are design values for $100 \%$ duration of load. All values except for El and K are permitted to be adjusted for other load durations as permitted by code, with the further exception that VLC shall not be increased for shorter durations of load. Design values listed are applicable for Allowable Stress Design (ASD).
2. Bending stiffness (EI) of the I-joist.
3. Moment capacity (M) of a single I-joist. Moment capacity of the I-Joist shall not be increased by any repetitive member use factor.
4. Shear capacity $(\mathrm{V})$ with a minimum bearing length of 4 inches.
5. Vertical Load Capacity when continuously supported.
6. Coefficient of shear deflection (K), used to calculate deflections for I-joist applications. Equations 1 and 2 below are provided for uniform load and center point load conditions for simple spans.

$$
\text { Uniform Load: } \quad \text { Center-Point Load: }
$$

$[1] \delta=\frac{5 \omega \ell^{4}}{384 \mathrm{EI}}+\frac{\omega \ell^{2}}{\mathrm{~K}}$
[2] $\delta=\frac{\mathrm{P} \ell^{3}}{48 \mathrm{EI}}+\frac{2 \mathrm{P} \ell}{\mathrm{K}}$
where:
$\delta=$ calculated deflection (in) $\quad \omega=$ uniform load (lb/in)
$\mathrm{El}=$ bending stiffness of the I -joist $\left(\mathrm{lb}-\mathrm{in}^{2}\right) \quad \ell=$ design span (in)
$\mathrm{K}=$ coefficient of shear deflection (lb) $\quad \mathrm{P}=$ concentrated load (lb)
7. Design properties meet or exceed the requirements of the PRI-400 Performance Standard for APA I-Joists for the corresponding I-joist series and depth.

## RFP| ${ }^{\circledR}$-Joist Allowable Reaction Information

General Note: Determine the allowable reaction capacity from Table 1 and Table 2 and use the lesser of the two values (refer to the notes for each table).
TABLE 1: RFPI@-JOIST REACTION CAPACITIES WITH OR WITHOUT WEB STIFFENERS (W.S.) ${ }^{(1)}$

| Joist Depth | Joist Series | End Reaction (lbs) |  |  |  | Intermediate Reaction (lbs) |  |  |  | Web Stiffener Nails ${ }^{(2)}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 1-3/4" Bearing |  | 4" Bearing |  | 3-1/2" Bearing |  | 5-1/4" Bearing |  |  |
|  |  | No W.S. | With W.S. | No W.S. | With W.S. | No W.S. | With W.S. | No W.S. | With W.S. |  |
| 9-1/2" | RFPI-20 | 910 | 1,150 | 1,220 | 1,220 | 1,775 | 1,875 | 2,000 | 2,300 | 4-8d |
|  | RFPI-40S | 1,080 | 1,120 | 1,120 | 1,120 | 2,160 | 2,240 | 2,240 | 2,240 | 4-8d |
|  | RFPI-400 | 1,025 | 1,220 | 1,220 | 1,220 | 2,150 | 2,250 | 2,300 | 2,440 | 4-8d |
|  | RFPI-40 | 1,080 | 1,220 | 1,330 | 1,330 | 2,250 | 2,500 | 2,550 | 2,650 | 4-8d |
|  | RFPI-60S | 1,080 | 1,120 | 1,120 | 1,120 | 2,160 | 2,240 | 2,240 | 2,240 | 4-8d |
|  | RFPI-70 | 1,120 | 1,330 | 1,330 | 1,330 | 2,335 | 2,500 | 2,550 | 2,650 | 4-8d |
|  | RFPI-90 | 1,330 | 1,585 | 1,700 | 1,890 | 3,020 | 3,445 | 3,445 | 3,475 | 4-8d |
| 11-7/8" | RFPI-20 | 950 | 1,225 | 1,420 | 1,420 | 1,935 | 2,035 | 2,135 | 2,435 | 4-8d |
|  | RFPI-40S | 1,200 | 1,340 | 1,420 | 1,420 | 2,500 | 2,625 | 2,660 | 2,840 | 4-8d |
|  | RFPI-400 | 1,050 | 1,265 | 1,480 | 1,480 | 2,250 | 2,350 | 2,350 | 2,650 | 4-8d |
|  | RFPI-40 | 1,200 | 1,400 | 1,550 | 1,550 | 2,500 | 2,625 | 2,660 | 2,870 | 4-8d |
|  | RFPI-60S | 1,200 | 1,340 | 1,420 | 1,420 | 2,500 | 2,625 | 2,660 | 2,840 | 4-8d |
|  | RFPI-70 | 1,200 | 1,470 | 1,550 | 1,550 | 2,500 | 2,625 | 2,660 | 2,870 | 4-8d |
|  | RFPI-80S | 1,280 | 1,590 | 1,550 | 1,590 | 2,810 | 3,180 | 3,100 | 3,180 | 4-10d |
|  | RFPI-90 | 1,400 | 1,745 | 1,885 | 2,050 | 3,355 | 3,475 | 3,475 | 3,675 | 4-10d |
| 14" | RFPI-20 | 950 | 1,290 | 1,550 | 1,610 | 1,935 | 2,035 | 2,135 | 2,435 | 4-8d |
|  | RFPI-40S | 1,200 | 1,530 | 1,550 | 1,710 | 2,500 | 2,740 | 2,755 | 3,050 | 4-8d |
|  | RFPI-400 | 1,050 | 1,305 | 1,550 | 1,710 | 2,250 | 2,350 | 2,350 | 2,650 | 4-8d |
|  | RFPI-40 | 1,200 | 1,560 | 1,550 | 1,770 | 2,500 | 2,740 | 2,755 | 3,065 | 4-8d |
|  | RFPI-60S | 1,200 | 1,530 | 1,550 | 1,710 | 2,500 | 2,740 | 2,755 | 3,050 | 4-8d |
|  | RFPI-70 | 1,200 | 1,590 | 1,550 | 1,770 | 2,500 | 2,740 | 2,755 | 3,065 | 4-8d |
|  | RFPI-80S | 1,280 | 1,750 | 1,550 | 1,835 | 3,020 | 3,360 | 3,210 | 3,600 | 4-10d |
|  | RFPI-90 | 1,400 | 1,885 | 1,885 | 2,195 | 3,355 | 3,500 | 3,500 | 3,850 | 4-10d |
| 16" | RFPI-40S | 1,200 | 1,710 | 1,550 | 1,970 | 2,500 | 2,850 | 2,850 | 3,250 | 4-8d |
|  | RFPI-400 | 1,050 | 1,340 | 1,550 | 1,970 | 2,250 | 2,350 | 2,350 | 2,650 | 4-8d |
|  | RFPI-40 | 1,200 | 1,710 | 1,550 | 1,970 | 2,500 | 2,850 | 2,850 | 3,250 | 4-8d |
|  | RFPI-60S | 1,200 | 1,710 | 1,550 | 1,970 | 2,500 | 2,850 | 2,850 | 3,250 | 4-8d |
|  | RFPI-70 | 1,200 | 1,710 | 1,550 | 1,970 | 2,500 | 2,850 | 2,850 | 3,250 | 4-8d |
|  | RFPI-80S | 1,280 | 1,900 | 1,550 | 2,070 | 3,020 | 3,525 | 3,310 | 4,000 | 4-10d |
|  | RFPI-90 | 1,400 | 2,025 | 1,885 | 2,330 | 3,355 | 3,525 | 3,525 | 4,025 | 4-10d |

Notes:

1. The tabulated design values in Table 1 are for $100 \%$ duration of load. Interpolation between tabulated values is permitted. All values in Table 1 shall be permitted to be adjusted for other load durations.
2. Number of nails required for web stiffeners. Refer to page 24 for web stiffener and nail installation requirements.

## TABLE 2: RFP(@-JOIST REACTION CAPACITIES BASED ON FLANGE ALLOWABLE COMPRESSION PERP.-TO-GRAIN ${ }^{(11) 2]}$

| Depth | Joist Series | End Reaction (lbs) |  |  |  | Intermediate Reaction (Ibs) |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 1-3/4" Bearing |  | 4" Bearing |  | 3-1/2" Bearing |  | 5-1/4" Bearing |  |
|  |  | No W.S. | With W.S. | No W.S. | With W.S. | No W.S. | With W.S. | No W.S. | With W.S. |
|  | RFPI-20 | 1,835 |  | 4,205 |  | 4,070 |  | 5,910 |  |
|  | RFPI-40S | 1,760 |  | 4,020 |  | 3,895 |  | 5,655 |  |
|  | RFPI-400 | 2,195 |  | 5,015 |  | 4,860 |  | 7,055 |  |
| All Depths in | RFPI-40 | 2,475 |  | 5,665 |  | 5,490 |  | 7,970 |  |
| each series | RFPI-60S | 2,175 |  | 4,970 |  | 4,815 |  | 6,990 |  |
|  | RFPI-70 | 2,475 |  | 5,665 |  | 5,490 |  | 7,970 |  |
|  | RFPI-80S | 3,090 |  | 7,070 |  | 6,850 |  | 9,940 |  |
|  | RFPI-90 | 3,830 |  | 8,755 |  | 8,480 |  | 12,310 |  |

## Notes:

1. Maximum allowable reaction capacity based on flange Fc perp. Interpolation between tabulated values in Table 2 is permitted.
2. The values in Table 2 are for $100 \%$ duration of load and shall not be increased for shorter durations of load.

## Allowable Floor Clear Spans For RFP| ${ }^{\circledR}$-Joists

## 40 PSF LIVE LOAD AND 10 PSF DEAD LOAD

L/480 Live Load Deflection

| Joist <br> Depth | Joist Series | 40/10 Simple Span |  |  |  | 40/10 Multiple Span |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 12" o.c. | 16" o.c. | 19.2" o.c. | 24" o.c. | 12" o.c. | 16" o.c. | 19.2" o.c. | 24" o.c. |
| 9-1/2" | RFPI 20 | 17' - 2" | 15' - 9" | 14' - 10" | 13'-10" | 18'-9" | 17' - 1" | 16'-2" | 14' - 0 " |
|  | RFPI 40S | 18' - 0" | 16'-5" | 15' - 6" | 14' - 6" | 19'-7" | 17'-11" | 16' - 4' | 14' - 7" |
|  | RFPI 400 | 18' - 0" | $16^{\prime}-5^{\prime \prime}$ | 15' - 6" | 14' - 6" | 19'-7" | 17'-10" | 16'-10" | 15'-9" |
|  | RFPI 40 | 18'-7" | 16'-11" | 16' - 0" | 14'-11" | 20'-2" | 18'-5" | 17'-5' | 16'-2" |
|  | RFPI 60S | 18'-11" | 17'-4" | 16' - 4" | 15' - ${ }^{\prime \prime}$ | 20' - 8" | 18'-10" | 17'-9" | 16' - 6" |
|  | RFPI 70 | 19'-9" | 18' - 0" | 17' - 0" | 15'-10" | 21'-6" | 19'-7" | 18'-5" | 17'-2" |
|  | RFPI 90 | 22' - 3" | 20' - ${ }^{\prime \prime}$ | 19' - 0" | 17' - 9" | 24' - 2" | 22'-0" | 20' - 8' | 19'-3" |
| 11-7/8" | RFPI 20 | 20' - 6" | 18'-9" | 17' - 9" | 16' - 6" | 22'-4" | 20' - ${ }^{\prime \prime}$ | 18'-10" | 15' - ${ }^{\prime \prime}$ |
|  | RFPI 40S | 21' - 5" | 19'-7" | 18' - 6" | 16' - 8" | 23' - 5" | 20' - ${ }^{\prime \prime}$ | 18'-7" | 16' - 7" |
|  | RFPI 400 | 21'-5" | 19'-7" | 18' - 6" | $17^{\prime}-3^{\prime \prime}$ | 23'-4" | 21'-4" | 20'-1" | 17' - 9" |
|  | RFPI 40 | 22' - 1" | 20'-2" | 19' - 0" | 17' - 9" | 24'-1" | 22' - 0" | 20'-8" | 19'-3" |
|  | RFPI 60S | 22'-7" | 20'-8" | 19' - 6" | 18'-2" | 24'-8" | 22' - 6" | 21'-2" | 19'-7" |
|  | RFPI 70 | 23' - 7" | 21'-6" | 20' - 3" | 18'-10" | 25'-8" | 23' - ${ }^{\prime \prime}$ | 22'-0" | 19'-9" |
|  | RFPI 80S | 24'-11" | 22'-8" | 21'-4" | 19'-11" | 27'-1" | 24' - 8" | 23'-3" | 21'-7" |
|  | RFPI 90 | 26' - 6" | 24'-1" | 22' - 8" | 21'-1" | 28'-10' | 26' - 3" | 24'-8" | 22'-11" |
| 14" | RFPI 20 | 23'-4" | 21'-4" | 20'-2" | 18' - 6" | 25'-5" | 22'-7" | 19'-2" | 15'-3" |
|  | RFPI 40S | 24' - 4" | 22'-3" | 20' - 6" | 18'-4" | 25'-11" | 22'-5" | 20'-5" | 18' - 3" |
|  | RFPI 400 | 24'-4" | 22'-3' | 21'-0" | 19'-7" | 26'-7" | 24' - 3" | 22'-3" | 17' - 9" |
|  | RFPI 40 | 25'-2" | 22'-11" | 21'-8" | 20'-2" | 27'-5' | 25' - 0" | 23'-7" | 19'-9" |
|  | RFPI 60S | 25' - 9' | 23'-6" | 22'-2" | 20' - 8" | 28'-0" | 25'-7" | 24'-1" | 19'-9" |
|  | RFPI 70 | 26'-10" | 24' - 5' | 23' - ${ }^{\prime \prime}$ | 21'-5" | 29'-3" | 26'-7" | 24'-9" | 19'-9" |
|  | RFPI 80S | 28' - ${ }^{\prime \prime}$ | 25'-9" | 24' - 3" | 22'-7" | 30'-9" | 28'-0" | 26'-4" | 23'-11" |
|  | RFPI 90 | 30'-1" | 27'-5" | 25'-9" | 23'-11" | 32'-10' | 29'-10" | 28'-1" | 26' - 0" |
| 16" | RFPI 40S | 26'-11' | 24' - 3' | 22'-1" | 19'-9" | 27'-11" | 24' - 2" | 22'-0" | 19' - 8" |
|  | RFPI 400 | 27' - 0" | 24' - 8" | 23'-4" | 20'-10" | 29'-6" | 26'-4" | 22'-3" | 17' - 9" |
|  | RFPI 40 | 27'-10" | 25'-5" | 24' - 0" | 22'-4" | 30'-4" | 27' - 8' | 24'-9" | 19' - 9" |
|  | RFPI 60S | 28'-6" | 26'-0" | 24'-7" | 22'-11" | 31'-1" | 28'-4" | 24'-9" | 19'-9" |
|  | RFPI 70 | 29'-9" | 27'-1" | 25' - 6" | 23' - 9" | 32'-5" | 29' - 6" | 24'-9" | 19'-9" |
|  | RFPI 80S | 31'-4" | 28'-6" | 26'-10" | 25' - 0" | 34' - ${ }^{\prime \prime}$ | 31'-1" | 29'-3" | 23'-11" |
|  | RFPI 90 | 33'-4" | 30'-4" | 28'-7" | 26' - 7" | 36'-5' | 33'-1" | 31'-1" | 26' - 7" |

40 PSF LIVE LOAD AND 20 PSF DEAD LOAD

## L/480 Live Load Deflection

| Joist <br> Depth | Joist Series | 40/20 Simple Span |  |  |  | 40/20 Multiple Span |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 12" o.c. | 16" o.c. | 19.2" o.c. | 24" o.c. | 12" o.c. | 16" o.c. | 19.2" o.c. | 24" o.c. |
| 9-1/2" | RFPI 20 | 17' - 2" | 15' - 9" | 14' - 10" | 13' - 7" | 18' - 9" | 16' - 7" | 14' - 7" | 11'-7" |
|  | RFPI 40S | 18' - 0" | 16' - 5" | 14'-11" | 13' - 4" | 18'-11" | 16' - 4' | 14'-11" | 13' - 3" |
|  | RFPI 400 | 18' - 0" | 16' - 5" | 15' - 6" | 14' - 6" | 19'-7" | 17'-10" | 16' - 6" | 14' - 1" |
|  | RFPI 40 | 18' - 7" | 16'-11" | 16' - 0" | 14'-11" | 20'-2" | 18' - 5' | 17' - 5' | 14' - 9" |
|  | RFPI 60S | 18'-11" | 17' - 4" | 16' - 4" | 15'-3" | 20' - 8' | 18'-10" | 17'-6" | 14' - 2" |
|  | RFPI 70 | 19'-9" | 18' - 0" | 17' - 0" | 15'-10" | 21'-6" | 19'-7" | 18'-5" | 15' - 4" |
|  | RFPI 90 | 22' - 3" | 20' - 3" | 19' - 0" | 17'-9" | 24' - 2" | 22' - 0" | 20'-8" | 19'-3" |
| 11-7/8" | RFPI 20 | 20' - 6" | 18' - 9" | 17' - 3" | 15' - 5" | 21'-10" | 18' - 10" | 15'-11" | 12' - 8" |
|  | RFPI 40S | 21'-5" | 18' - 8" | 17' - 1" | 15' - 3" | 21' - 6" | 18'-7" | 17' - 0" | 15' - 2" |
|  | RFPI 400 | 21'-5" | 19'-7" | 18' - 6" | 16'-10" | 23'-4" | 20' - 7" | 18'-6" | 14' - 9" |
|  | RFPI 40 | 22' - 1" | 20'-2" | 19' - 0" | 17' - 9" | 24' - 1" | 21'-10" | 19'-11" | 16' - 5" |
|  | RFPI 605 | 22' - 7" | 20' - 8" | 19' - 6" | 17'-11" | 24' - 8" | 21'-11" | 20'-0" | $16^{\prime}$ - ${ }^{\prime \prime}$ |
|  | RFPI 70 | 23'-7" | 21'-6" | 20' - 3" | 18'-10" | 25' - 8' | 23' -5" | 20'-7" | $16^{\prime}$ - 5" |
|  | RFPI 80S | 24'-11" | 22' - 8" | 21'-4" | 19'-11" | 27'-1" | 24' - 8" | 23'-2" | 18' - 6" |
|  | RFPI 90 | 26' - 6" | 24'-1" | 22' - 8" | 21'-1" | 28'-10" | 26'-3" | 24'-8" | 22'-2" |
| 14" | RFPI 20 | 23' - 4' | 20' - 8' | 18' - 10" | 15' - 8' | 23' - 10" | 19'-2" | 15'-11" | $12^{\prime}$ - "' $^{\prime \prime}$ |
|  | RFPI 40S | 23'-9" | 20' - 6" | 18' - 9" | 16' - 9" | 23' - 8" | 20' - 5" | 18' - 8" | 16' - 5" |
|  | RFPI 400 | 24' - 4" | 22'-3" | 20' - 7" | 17' - 4" | 26' - 0" | 22'-3" | 18'-6" | 14' - 9" |
|  | RFPI 40 | 25'-2" | 22'-11" | 21'-8" | 19' - 6" | 27' - 5' | 23'-10" | 20' - 7" | 16' - 5" |
|  | RFPI 60S | 25' - 9" | 23' - 6" | 22' - 0" | 19'-8" | 27'-10" | 24' - 1" | 20'-7" | $16^{\prime}$ - 5" |
|  | RFPI 70 | 26'-10" | 24' - 5" | 23' - 0" | 19'-10" | 29' - 3' | 24' - 9" | 20' - 7" | 16' - 5" |
|  | RFPI 80S | 28' - 3' | 25'-9" | 24' - 3' | 21'-2" | 30'-9" | 28' - 0" | 24'-11" | 19'-11" |
|  | RFPI 90 | 30' - 1' | 27' - 5" | 25'-9" | 23'-2" | 32'-10" | 29'-10" | 27'-9" | 22'-2" |
| 16" | RFPI 40S | 25' - 7" | 22'-1" | 20' - 2" | 18' - ${ }^{\prime \prime}$ | 25' - 6" | 22' - 0" | 20' - $1^{\prime \prime}$ | $16^{\prime}-5^{\prime \prime}$ |
|  | RFPI 400 | 27' - 0" | 24' - 1" | 21'-9" | 17'-4" | 27'-9" | 22' - 3" | 18'-6" | 14' - 9" |
|  | RFPI 40 | 27'-10" | 25' - 5" | 23' - 4" | 19'-10" | 29' - 6" | 24' - 9" | 20'-7" | 16' - 5" |
|  | RFPI 60S | 28' - 6" | 26' - 0" | 23'-9" | 19'-10" | 30' - $0^{\prime \prime}$ | 24'-9" | 20' - 7" | 16' - 5" |
|  | RFPI 70 | 29'-9" | 27' - 1" | 24' - 10" | 19'-10" | 32' - 5" | 24'-9" | 20' - 7" | 16' - 5" |
|  | RFPI 80S | 31'-4" | 28' - 6" | 26' - 6" | 21'-2" | 34'-2" | 30' - 0" | 24'-11" | 19'-11" |
|  | RFPI 90 | 33'-4' | 30'-4" | 28'-7" | 23'-2' | $36^{\prime}-5^{\prime \prime}$ | $33^{\prime}-1^{\prime \prime}$ | 27' - 9" | 22'-2" |

Notes:

- Clear span is the clear distance between the face of supports.
- Spans are based on uniform loads as shown above. Use appropriate software (e.g. Simpson Strong-Tie ${ }^{\circledR}$ Component Solutions ${ }^{\text {™ }}$ ) or engineering analysis for other loading.
- Web stiffeners are not required for spans shown but may be required for hangers.
- Maximum deflection is limited to L/480 for live load and L/240 for total load.
- A minimum of $1-3 / 4$ " is required for end bearing, $3-1 / 2$ " for intermediate bearing.
- Multiple Span lengths shown require adequate bottom flange lateral bracing.
- Spans are based on composite action with glued-nailed sheathing meeting the APA requirements shown in the adjacent table.
- Adhesives shall meet APA Specification AFG-01 or ASTM D3498.
- Spans shall be reduced by 12 inches when floor sheathing is nailed only.

|  | Min Thickness | Span Rating | Floor Joist Spacing |
| :---: | :---: | :---: | :---: |
| Rated Sheathing Rated Sheathing | $\begin{aligned} & 19 / 32^{\prime \prime} \\ & 23 / 32^{\prime \prime} \end{aligned}$ | $\begin{aligned} & (40 / 20) \\ & (48 / 24) \end{aligned}$ | $\begin{gathered} 19.2^{\prime \prime} \text { or less } \\ 24 \text { or less } \end{gathered}$ |
| Rated Sturd-I Floor Rated Sturd-I Floor | $\begin{aligned} & 19 / 32 " \\ & 23 / 32^{\prime \prime} \end{aligned}$ | $\begin{aligned} & 20 " \text { o.c. } \\ & 24 \text { " о.c. } \end{aligned}$ | $19.2^{\prime \prime}$ or less <br> 24 " or less |

## Web Hole Specifications

One of the benefits of using RFPI-Joists in residential floor and roof construction is that holes may be cut in the joist webs to accommodate electrical wiring, plumbing lines and other mechanical systems, therefore minimizing the depth of the floor system.

## RULES FOR CUTTING HOLES IN RFPI-JOISTS

1. See chart on page 13 for allowable hole sizes and locations. The distance between the inside edge of the nearest support and the centerline of any hole shall not be less than that shown in the chart on page 13.
2. Except for cutting to length, NEVER cut, drill or notch I-joist flanges.
3. Whenever possible center holes vertically in the middle of the web. However, holes may be located vertically anywhere in the web provided a minimum of $1 / 8^{\prime \prime}$ of web remains between the edge of the hole and the flanges.
4. The maximum size hole that can be cut into an I-joist web shall equal the clear distance between the flanges of the $I$-joist minus $1 / 4^{\prime \prime}$. A minimum of $1 / 8^{\prime \prime}$ should always be maintained between the top or bottom of the hole and the adjacent I-joist flange.
5. The sides of square holes or longest side of rectangular holes should not exceed three fourths of the diameter of the maximum round hole permitted at that location. DO NOT overcut the sides of square or rectangular holes.
6. Where more than one hole is necessary, the distance between adjacent hole edges must be a minimum of twice the diameter of the largest round hole or twice the size of the largest square hole (or twice the length of the longest side of the longest rectangular hole) and each hole must be sized and located in compliance with the requirements of the chart on page 13.
7. Knockouts are pre-scored holes for the contractor's convenience to install electrical or small plumbing lines. They are $1-1 / 2^{\prime \prime}$ in diameter, and are spaced approximately 16 " on center along the length of the I-joist. Where possible, it is preferable to use knockouts instead of field cutting holes. For floor applications, positioning the I -joists so the knockouts are all on the bottom of the joist may ease the installation of electrical wiring or residential sprinkler systems. DO NOT hammer holes in web, except at knock outs.
8. A knockout is not considered a hole and may be utilized anywhere it occurs. It can be ignored for purposes of calculating minimum distances between holes.
9. $1-1 / 2$ " holes shall be permitted anywhere in a cantilevered section of an RFPI-Joist. Holes of greater size may be permitted subject to verification.
10. A $1-1 / 2$ " hole can be placed anywhere in the web provided that it meets the requirements of rule 6 on this page. Holes are not permitted to be cut within the area 6 " off face of support and 6 " vertically off bottom flange. Knockouts can be removed if they fall in this area.
11. A group of round holes at approximately the same location shall be permitted if they meet the requirements for a single round hole circumscribed around them (see diagram on page 13).
12. All holes shall be cut in a workman-like manner in accordance with the restrictions listed herein.


Never drill, cut or notch the flange, or over-cut the web. Holes in webs should be cut with a sharp saw. For rectangular holes, avoid over-cutting the corners, as this can cause unnecessary stress concentrations. Slightly rounding the corners is recommended. Start the rectangular hole by drilling a 1 "-diameter hole in each of the four corners and then make the cuts between the holes to minimize damage to the l-joist.

## Holes For RFPI ${ }^{\circledR}$-Joists Used In Residential Floor/ Roof Applications



RFPI-JOIST TYPICAL HOLES - See "HOW TO USE HOLE CHART" below and "Rules for Cutting Holes in RFPI Joists" on page 12

Knockouts: See notes 7 and 8 on page 12. DO NOT hammer holes in web, except at knockouts.

## HOLE CHART - MINIMUM DISTANCE FROM FACE OF NEAREST JOIST SUPPORT TO CENTER OF HOLE (11)[2]

| Joist <br> Depth | Joist <br> Series | SAF(3) | Round Hole Diameter (in) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | 2 | 3 | 4 | 5 | 6 | 6-1/4 | 7 | 8 | 8-5/8 | 9 | 10 | 10-3/4 | 11 | 12 | 12-3/4 |
|  |  |  | Minimum Distance from Inside Face of Nearest Support to Center of Hole (ft-in) ${ }^{(1)(2)}$ |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 9-1/2" | RFPI 20 | 11.58 | 0'-7" | 0'-8" | 2'-0" | 3'-6" | 5'-4" | 5'-9" |  |  |  |  |  |  |  |  |  |
|  | RFPI 40S | 13.25 | 1'-2" | 2'-2" | 3'-3" | 4'-4" | 5'-9" | 6'-3" |  |  |  |  |  |  |  |  |  |
|  | RFPI 400 | 14.08 | $1{ }^{\prime}-0 \mid$ | 2'-1" | 3'-3" | 4'-9" | 6'-4" | 6'-9" |  |  |  |  |  |  |  |  |  |
|  | RFPI 40 | 14.75 | 0'-8" | 1'-11" | 3'-2' | 4'-9" | 6'-6" | 6'-11" |  |  |  |  |  |  |  |  |  |
|  | RFPI 60S | 14.17 | 2'-0" | 3'-3" | 4'-8" | 6'-1" | 7'-7" | 8'-0" |  |  |  |  |  |  |  |  |  |
|  | RFPI 70 | 15.33 | $1^{\prime \prime}$ '1" | 2'-3" | 3'-10" | 5'-6" | 7'-3" | 7'-8" |  |  |  |  |  |  |  |  |  |
|  | RFPI 90 | 17.75 | 3'-7" | 4'-11" | 6'-3" | 7'-8" | 9'-2" | 9'-6" |  |  |  |  |  |  |  |  |  |
| 11-7/8" | RFPI 20 | 12.67 | 0'-7" | 0'-8' | 0'-8" | 1'-9" | 3'-4" | 3'-9" | 5'-0" | 6'-10" | 8'-0" |  |  |  |  |  |  |
|  | RFPI 40S | 15.17 | 0'-7" | 0'-10" | 1'-10" | 2'-11" | 4'-0" | 4'-4" | 5'-2" | 6'-8" | 7'-11" |  |  |  |  |  |  |
|  | RFPI 400 | 14.75 | 0'-7" | 0'-8" | 1'-7" | 2'-11" | 4'-4" | 4'-8" | 5'-10" | 7'-8" | 8'-10" |  |  |  |  |  |  |
|  | RFPI 40 | 16.42 | 0'-7" | $0^{\prime}-10^{\prime \prime}$ | 2'-0" | 3'-5" | 4'-11" | 5'-3" | 6'-5" | 8'-2" | 9'-6" |  |  |  |  |  |  |
|  | RFPI 60S | 16.42 | 0'-8" | 1'-10" | 3'-2" | 4'-5" | 5'-10" | 6'-2" | 7'-4" | 8'-11" | 10'-1" |  |  |  |  |  |  |
|  | RFPI 70 | 16.42 | 0'-7" | 1'-0" | 2'-5" | 3'-10" | 5'-6" | 6'-0" | 7'-4" | 9'-4" | 10'-8" |  |  |  |  |  |  |
|  | RFPI 80S | 18.50 | 0'-11' | 2'-4" | 3'-10" | 5'-4" | 6'-11' | 7'-4" | 8'-7" | 10'-4" | 11'-6" |  |  |  |  |  |  |
|  | RFPI 90 | 21.08 | 0'-7" | 1'-4" | 2'-9" | 4'-4" | 5'-11' | 6'-4" | 7'-7" | 9'-5" | 10'-10' |  |  |  |  |  |  |
| 14" | RFPI 20 | 12.67 | 0'-7" | 0'-8" | 0'-8" | 0'-9" | 0'-9" | 1'-1" | 2'-3" | 4'-2" | 5'-4" | 6'-1' | 8'-2" | 9'-11" |  |  |  |
|  | RFPI 40S | 16.42 | 0'-7" | 0'-8" | 0'-8" | 1'-4" | 2'-5" | 2'-8" | 3'-6" | 4'-7" | 5'-5" | 6'-0" | 7'-7" | 9'-4" |  |  |  |
|  | RFPI 400 | 14.75 | 0'-7" | 0'-8" | 0'-8" | 0'-9" | 1'-11" | 2'-4" | $3^{\prime}-7{ }^{\prime \prime}$ | 5'-3" | 6'-4" | 7'-0" | 9'-0" | 10'-10' |  |  |  |
|  | RFPI 40 | 16.42 | 0'-7" | 0'-8" | 0'-8" | 1'-3" | 2'-7" | 2'-11" | 4'-2" | 5'-11" | 7'-0" | 7'-9" | 9'-8" | 11'-7" |  |  |  |
|  | RFPI 60S | 16.42 | 0'-7" | 0'-8" | 0'-8" | 1'-8" | 3'-2" | 3'-6" | 4'-9" | $6^{\prime}-6{ }^{\prime \prime}$ | 7'-8" | 8'-4" | 10'-4" | 12'-2" |  |  |  |
|  | RFPI 70 | 16.42 | 0'-7" | 0'-8" | 0'-8" | 1'-6" | 3'-1" | 3'-6" | 4'-10" | 6'-7" | 7'-9" | 8'-6" | 10'-11" | 12'-11" |  |  |  |
|  | RFPI 80S | 19.92 | 0'-7" | 0'-9" | 2'-2" | 3'-7" | 5'-1" | 5'-5" | 6'-7" | 8'-5" | 9'-7" | 10'-4" | 12'-5" | $14^{\prime}-0{ }^{\prime \prime}$ |  |  |  |
|  | RFPI 90 | 22.17 | 0'-7" | 0'-8' | 1'-3" | 2'-11" | 4'-7" | 5'-1" | 6'-5" | 8'-3" | 9'-5" | 10'-2" | 12'-3" | 14'-0" |  |  |  |
| 16" | RFPI 40S | 16.42 | 0'-7" | 0'-8" | 0'-8" | 0'-9" | 0'-9" | 0'-10" | 1'-5" | 2'-9" | 3'-7" | 4'-1" | 5'-6" | 6'-7" | 7'-0" | 8'-9" | 10'-9" |
|  | RFPI 400 | 14.75 | 0'-7" | 0'-8" | 0'-8" | 0'-9" | 0'-9" | 0'-10" | 0'-10" | $1^{\prime}-11^{\prime \prime}$ | $3{ }^{\prime}-1{ }^{\prime \prime}$ | 3'-10" | 5'-11" | 7'-6" | 8'-0" | 10'-4" | 12'-3" |
|  | RFPI 40 | 16.42 | 0'-7" | 0'-8' | 0'-8" | 0'-9" | 0'-9" | 0'-10" | $1^{\prime}-10^{\prime \prime}$ | 3'-6" | $4^{\prime}-6{ }^{\prime \prime}$ | 5'-2" | 6'-11" | 8'-5" | $9^{\prime}-0{ }^{\prime \prime}$ | 11'-5" | 13'-4" |
|  | RFPI 60S | 16.42 | 0'-7" | 0'-8" | 0'-8" | 0'-9" | 0'-9" | 0'-10" | 1'-10" | 3'-6" | 4'-6" | 5'-2" | 7'-3" | 8'-11" | 9'-6" | 11'-10" | 13'-9" |
|  | RFPI 70 | 16.42 | 0'-7" | 0'-8" | 0'-8" | 0'-9' | 0'-9" | 0'-10" | 2'-1" | 4'-2" | 5'-6" | 6'-4" | 8'-7" | 10'-5" | 11'-0" | 13'-6" | 15'-6" |
|  | RFPI 80 S | 19.92 | 0'-7" | 0'-8" | 0'-8" | 1'-2" | 2'-10" | 3'-3" | 4'-6" | 6'-3" | 7'-5" | 8'-1" | 9'-11" | 11'-5" | 11'-11" | 14'-3" | 16'-5" |
|  | RFPI 90 | 22.17 | 0'-7" | 0'-8' | 0'-8" | 0'-10" | 2'-9" | 3'-2' | 4'-7" | $6^{\prime}-7{ }^{\prime \prime}$ | 7'-10" | 8'-7" | 10'-8" | 12'-4" | 12'-11" | 15'-2" | 17'-1' |

## How to Use Hole Chart

1. Read across the top of Hole Chart to the desired hole size
2. Follow this column down to the row that represents the l-joist depth and designation. This number indicates the minimum distance from the face of the nearest support to the centerline of the hole.

Example: Need a 4-1/2-inch hole in an 11-7/8" RFPI ${ }^{\circledR}-400$ joist: From Hole Chart,
For a 4 -inch round hole, the minimum distance is $1^{\prime}-7$ ".
For a 5 -inch round hole, the minimum distance is $2^{\prime}-11^{\prime \prime}$
Therefore the minimum distance for the $4-1 / 2$-in round hole is $2^{\prime}-3$ " (halfway between $1^{\prime}-7^{\prime \prime}$ and $2^{\prime}-11^{\prime \prime}$ ).

## Notes:

1. Distances in this hole chart are based on uniformly loaded I-joists and allowable I-joist reactions without web stiffeners on minimum required bearing lengths. This chart conservatively accounts for the worst case created by the allowable simple or multiple floor spans shown elsewhere in this guide at on-center spacings of $12^{\prime \prime}, 16^{\prime \prime}, 19.2^{\prime \prime}$ and $24^{\prime \prime}$ with floor loads of 40 psf live load +10 psf dead load or 40 psf live load +20 psf dead load. Holes in conditions that fall outside of the hole chart parameters (including the use of web stiffeners, longer bearing lengths or other loading conditions) may still be acceptable. The most accurate method of determining the acceptability of a given hole is the use of appropriate software (e.g. Simpson Strong-Tie ${ }^{\circledR}$ Component Solutions ${ }^{\text {™ }}$ ) or engineering analysis for the actual condition.
2. Hole location distance is measured from inside face of nearest support to center of hole.
3. $\mathrm{SAF}=$ Span Adjustment Factor for optional hole calculation, used as defined on this page.

## Optional Hole Calculation

The Hole Chart is based on the I-joists being used at their maximum span. If the I-joists are placed at less than their full allowable span the minimum distance from the centerline of the hole to the face of the nearest joist support (D) as given above may be reduced as follows:

$$
D_{\text {reduced }}=\frac{L_{\text {actual }}}{S A F} \times D
$$

Where:
$D_{\text {reduced }}=$ Minimum distance from the inside face of the nearest joist support to center of hole, reduced for less-than-maximum span applications (ft).
$L_{\text {actual }}=$ The actual measured span distance between the inside faces of supports (ft) (for multi-span joist, use the longest span for $L_{\text {actual }}$ )
SAF = Span Adjustment Factor given in chart
D $\quad=$ The minimum distance from the inside face of the nearest joist support to center of hole from Hole Chart above.

## Rectangular Duct Chases

A duct chase is a large rectangular hole that is often required within the web of an I-joist to provide passage for ventilation ducts. While rectangular holes can be cut in the webs of I-joists using the Rules For Cutting Holes in RFPI ${ }^{\circledR}$-Joists discussed on page 12, the size of rectangular holes generated by this method is often insufficient for this use. The charts below have been generated specifically for duct chase applications.

## SIMPLE SPAN-MINIMUM DISTANCE FROM FACE OF NEAREST JOIST SUPPORT TO CENTER OF DUCT CHASE (11)(2)(3)

| Joist Depth | Joist Series | Minimum Distance from Inside Face of Nearest Support to Center of Duct Chase (ft-in) |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Duct Chase Length (in) |  |  |  |  |  |  |  |  |
|  |  | 8 | 10 | 12 | 14 | 16 | 18 | 20 | 22 | 24 |
| 9-1/2" | RFPI-20 | 6'-3" | 6'-7" | 6'-11" | 7'-3" | 7'-8" | 8'-1" | 8'-6" |  |  |
|  | RFPI-40S | 4'-11" | 5'-4" | 5'-9" | 6'-3" | 6'-8" | 7'-2" | 7'-7" | 8'-1" | 8'-8" |
|  | RFPI-400 | 6'-3" | 6'-7" | 6'-11" | 7'-4" | 7'-9" | 8'-3" | 8'-10" |  |  |
|  | RFPI-40 | 5'-9" | 6'-1" | $6^{\prime}-6{ }^{\prime \prime}$ | 6'-10" | 7'-2" | 7'-6" | 7'-11" | 8'-5" | 9'-0" |
|  | RFPI-60S | $6^{\prime}-0{ }^{\prime \prime}$ | 6'-4" | 6'-8" | 7'-0" | 7'-4" | 7'-9" | 8'-2" | 8'-8" | 9'-3" |
|  | RFPI-70 | 6'-4" | 6'-8" | 7'-0" | 7'-4" | 7'-9" | 8'-2" | 8'-7" | $9{ }^{\prime \prime}$-1" | 9'-9" |
|  | RFPI-90 | $6^{\prime}-7{ }^{\prime \prime}$ | 6'-11" | 7'-4" | 7'-8" | $8^{\prime}-0{ }^{\prime \prime}$ | 8'-4" | 8'-9" | $9^{\prime}-2{ }^{\prime \prime}$ | $9^{\prime \prime} 8^{\prime \prime}$ |
| 11-7/8" | RFPI-20 | 8'-0" | 8'-4" | 8'-9" | 9'-2" | $9{ }^{\prime}-8$ " | 10'-1" |  |  |  |
|  | RFPI-40S | 6'-3" | 6'-9" | 7'-3" | 7'-9" | 8'-4" | 8'-11" | 9'-6" | 10'-2" |  |
|  | RFPI-400 | 7'-11" | 8'-4" | 8'-9" | 9'-2" | 9'-9" | 10'-4" |  |  |  |
|  | RFPI-40 | 7'-6" | 7'-10" | 8'-2" | 8'-7" | 8'-11" | $9^{\prime}-5 "$ | 9'-11" | 10'-7" |  |
|  | RFPI-60S | 7'-7" | 8'-0" | 8'-5" | 8'-10" | 9'-3" | 9'-9" | 10'-3" | 10'-10" |  |
|  | RFPI-70 | 8'-2" | 8'-6" | 8'-11" | 9'-4" | 9'-9" | $10^{\prime}-3 "$ | 10'-10" | 11'-6" |  |
|  | RFPI-80S | 7'-11" | 8'-3" | 8'-7" | 9'-0" | 9'-4" | 9'-8" | 10'-2" | 10'-8" | 11'-3" |
|  | RFPI-90 | 8'-7" | 9'-0" | 9'-4" | 9'-8" | 10'-1" | 10'-6" | 11'-0" | 11'-7" | 12'-2" |
| 14" | RFPI-20 | 9'-6" | 9'-11" | 10'-5" | 10'-11" | 11'-4" |  |  |  |  |
|  | RFPI-40S | 7'-6" | 8'-0" | 8'-7" | 9'-2" | 9'-9" | 10'-4" | 10'-11" | 11'-7" |  |
|  | RFPI-400 | 9'-5" | 9'-11" | 10'-4" | 10'-11" | 11'-6" | 12'-1" |  |  |  |
|  | RFPI-40 | 8'-11" | 9'-4" | 9'-9" | 10'-2" | 10'-8" | 11'-2"' | 11'-10" | 12'-5" |  |
|  | RFPI-60S | 9'-2" | 9'-7" | 10'-0" | 10'-6" | 11'-0" | 11'-7" | 12'-2" | 12'-10" |  |
|  | RFPI-70 | 9'-9" | 10'-2" | 10'-7" | 11'-1" | 11'-7" | 12'-3" | 12'-10" |  |  |
|  | RFPI-80S | $9^{\prime}-4{ }^{\prime \prime}$ | 9'-9" | 10'-2" | 10'-7" | 11'-1" | 11'-6" | 12'-0" | 12'-7" | 13'-3" |
|  | RFPI-90 | 10'-3" | 10'-8" | 11'-1" | 11'-7" | 12'-1" | 12'-7" | $13^{\prime}-1{ }^{\prime \prime}$ | $13^{\prime}-9{ }^{\prime \prime}$ | 14'-5" |
| 16" | RFPI-40S | 8'-8" | 9'-3' | 9'-10" | 10'-5" | 11'-0" | 11'-8" | 12'-5" | 13'-3" |  |
|  | RFPI-400 | 10'-10" | 11'-4" | 12'-0" | 12'-7" | 13'-2" |  |  |  |  |
|  | RFPI-40 | 10'-3" | 10'-9" | 11'-2" | 11'-8" | 12'-3" | 12'-10" | 13'-6" |  |  |
|  | RFPI-60S | 10'-7" | 11'-1" | 11'-7" | 12'-0" | 12'-8" | 13'-3" | 13'-11" |  |  |
|  | RFPI-70 | 11'-3" | 11'-9" | 12'-3" | 12'-9" | 13'-5" | 14'-0" | 14'-8" |  |  |
|  | RFPI-80S | 10'-9" | 11'-3" | 11'-9" | 12'-3" | 12'-9" | 13'-3" | 13'-10" | 14'-6" | 15'-2" |
|  | RFPI-90 | 12'-0" | 12'-5" | 12'-10" | 13'-4" | 13'-10" | $14^{\prime}-5 "$ | 15'-1" | 15'-9" | 16'-5" |

MULTIPLE SPAN-MINIMUM DISTANCE FROM FACE OF NEAREST JOIST SUPPORT TO CENTER OF DUCT CHASE (11(2)(3)

| Joist Depth | Joist Series | Minimum Distance from Inside Face of Nearest Support to Center of Duct Chase (ft-in) |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Duct Chase Length (in) |  |  |  |  |  |  |  |  |
|  |  | 8 | 10 | 12 | 14 | 16 | 18 | 20 | 22 | 24 |
| 9-1/2" | RFPI-20 <br> RFPI-40S <br> RFPI-400 <br> RFPI-40 <br> RFPI-60S <br> RFPI-70 <br> RFPI-90 | $\begin{gathered} 9^{\prime}-5 " \\ 7^{\prime \prime}-5 " \\ 9^{\prime}-4 " \\ 8^{\prime \prime}-10^{\prime \prime} \\ 9^{\prime}-0^{\prime \prime} \\ 9^{\prime}-7{ }^{\prime \prime \prime}-3^{\prime \prime} \end{gathered}$ | $\begin{aligned} & 7 '-11 " \\ & 9 '-10 " \\ & 9^{\prime}-3 " \\ & 9^{\prime}-5 " \\ & 10 '-5 " \\ & 10 '-8 " \end{aligned}$ | $\begin{gathered} 8^{\prime}-6 " \\ 9^{\prime}-8 " \\ 9^{\prime}-11^{\prime \prime \prime} \\ 10^{\prime}-6^{\prime \prime} \\ 11^{\prime}-\mathbf{c}^{\prime \prime} \end{gathered}$ | $\begin{gathered} 9^{\prime}-1 " \\ 10^{\prime \prime}-1 " \\ 11^{\prime \prime}-5 " \end{gathered}$ | $\begin{gathered} 9^{\prime}-7 " \\ \\ 11^{\prime}-11^{\prime \prime} \end{gathered}$ |  |  |  |  |
| 11-7/8" | RFPI-20 <br> RFPI-40S <br> RFPI-400 <br> RFPI-40 <br> RFPI-60S <br> RFPI-70 <br> RFPI-80S <br> RFPI-90 | $\begin{gathered} 9^{\prime}-4 " \\ 11^{\prime \prime}-3^{\prime \prime} \\ 11^{\prime}-5^{\prime \prime} \\ 12^{\prime}-5^{\prime \prime} \\ 12^{\prime}-0^{\prime \prime} \\ \hline 1 \text { " } \end{gathered}$ | $\begin{aligned} & 10^{\prime}-0 " \\ & 11^{\prime}-8 " \\ & 12^{\prime}-1 " \\ & 12 '-6 " \\ & 13^{\prime}-8 " \end{aligned}$ | $\begin{aligned} & 10^{\prime}-8 " \\ & \\ & 12^{\prime}-11^{\prime \prime} \\ & 14^{\prime \prime}-2^{\prime \prime} \end{aligned}$ | $\begin{aligned} & 11^{\prime}-5 " \\ & 13^{\prime}-5 " \end{aligned}$ |  |  |  |  |  |
| 14" | RFPI-20 <br> RFPI-40S <br> RFPI-400 <br> RFPI-40 <br> RFPI-60S <br> RFPI-70 <br> RFPI-80S <br> RFPI-90 | $\begin{gathered} 11^{\prime}-2 " \\ 13^{\prime \prime}-6^{\prime \prime} \\ 13^{\prime}-10^{\prime \prime} \\ 14^{\prime}-6^{\prime \prime} \\ 15^{\prime}-10^{\prime \prime} \end{gathered}$ | $\begin{gathered} 11^{\prime}-11^{\prime \prime} \\ \\ \\ 15^{\prime}-00^{\prime \prime} \\ 16^{\prime}-4 \prime \prime \end{gathered}$ | 12'-8" |  |  |  |  |  |  |
| 16" | RFPI-40S <br> RFPI-400 <br> RFPI-40 <br> RFPI-60S <br> RFPI-70 <br> RFPI-80S <br> RFPI-90 | $\begin{aligned} & 12^{\prime}-9 " \\ & 15^{\prime}-1 " \\ & 15^{\prime}-6^{\prime \prime} \\ & 16^{\prime}-9 " \end{aligned}$ | 13'-5" |  |  |  |  |  |  |  |

Chart Notes:

1. Top chart is applicable to uniformly loaded Simple Span conditions only. Bottom chart is applicable to uniformly loaded Multiple Span conditions only.
2. Duct chase location distance is measured from inside face of nearest support to center of duct chase.
3. Distances in these duct charts are based on uniformly loaded $I$-joists and allowable $I$-joist reactions without web stiffeners on minimum required bearing lengths. These charts conservatively account for the worst case created by the allowable Simple Spans (top chart) or Multiple Spans (bottom chart) shown elsewhere in this guide at on-center spacings of $12^{\prime \prime}, 16^{\prime \prime}, 19.2^{\prime \prime}$ and $24^{\prime \prime}$ with floor loads of 40 psf live load +10 psf dead load or 40 psf live load +20 psf dead load. Ducts in conditions that fall outside of the duct chart parameters (including the use of web stiffeners, longer bearing lengths or other loading conditions) may still be acceptable. The most accurate method of determining the acceptability of a given duct is the use of appropriate software (e.g. Simpson Strong-Tie ${ }^{\circledR}$ Component Solutions ${ }^{\text {TM }}$ ) or engineering analysis for the

## Rules for cutting duct chases in RFPI-Joists:

a. The maximum length of duct chase shall be as shown in the charts above.
b. Except for cutting to length, I-joist top and bottom flanges must NEVER be cut, notched or otherwise modified.
c. The maximum depth of the duct chases shall equal the clear distance between the flanges of the $I$-joist minus $1 / 4^{\prime \prime}$. A minimum of $1 / 8$ " should always be maintained between the top or the bottom of the chase and the adjacent I-joist flange.
d. When a duct chase is being placed within the web of an I-joist in conjunction with additional holes, the edge of the holes shall not be placed any closer to the edge of the duct than two times the length of the duct. All holes must be sized in accordance with the chart on page 13.
e. A knockout is not considered a hole and may be utilized wherever it occurs and may be ignored for purposes of calculating minimum distances between holes and duct chases.
f. All duct chases shall be cut in a workman-like manner in accordance with the restrictions listed above.

## Load Development

## LOAD DEVELOPMENT FOR RFPI-JOISTS WITH UNIFORM LOAD

## STEP ONE: CALCULATE THE TRIBUTARY WIDTH

Tributary Width (or Trib width) = Half of the distance to the next supporting member on both sides of the joist. It represents the width of the floor the joist is responsible to support.
Trib Width $=($ O.C. $\div 2)+($ O.C. $\div 2)=$ O.C. (expressed in units of feet) In the diagram below, if the O.C. spacing equals 16 ", the Trib Width = $16^{\prime \prime} / 12=1.33^{\prime}$

## STEP TWO: CALCULATE THE PLF ON THE JOIST

Pounds per Lineal Foot (or "PLF") $=($ PSF Load $) \times($ Trib Width $)$. This is the loading that the joist "feels" being applied along the top flange.
PLF $_{\text {Live Load }}=(40$ PSF $) \times\left(1.33^{\prime}\right)=53$ PLF Live Load
PLF $_{\text {Total Load }}=(50$ PSF $) \times\left(1.33^{\prime}\right)=67$ PLF Total Load
(Use these numbers to size the RFPI-Joist from the PLF table on page 16)


## Design Load

for floor given in pounds per square foot (PSF) for entire floor. Typical Residential Value is:
40 PSF Live Load
10 PSF Dead Load

## Pounds per Lineal Foot (PLF) <br> Trib Area of I-joist <br> Design Load - Pounds per Square Foot (PSF)

## LOAD DEVELOPMENT FOR RFPI-JOISTS WITH LOAD BEARING WALL

## STEP ONE

Calculate the portion of the wall load carried by each joist. This is also determined by the joist O.C. spacing and is given by:
$P_{\text {Live Load }}=(P L F)_{\text {Wall Live Load }} \times(O . C$.
$P_{\text {Total Load }}=(P L F)_{\text {Wall Total Load }} \times($ O.C. $)$
Where: O.C. = Joist on-center spacing (feet)
PLF = Wall loading (pounds per lineal foot) $\mathrm{P}=$ Concentrated load supported by joist (pounds)
As far as each joist is concerned, it feels the wall as a concentrated load (units of pounds). The greater the on-center spacing, the greater the portion of wall it must support.

## STEP TWO

Calculate the equivalent uniform PLF load due to this concentrated wall load. This equivalent PLF load will allow you to safely size the joist using the PLF table on page 16 no matter where the wall is located over the joists. It is given by:

$$
P L F_{E Q ~ L i v e ~ L o a d ~}=2 \mathrm{P}_{\text {Live Load }} \div \mathrm{L} \quad P L F_{E Q \text { Total Load }}=2 \mathrm{P}_{\text {Total Load }} \div \mathrm{L}
$$

For example, assume the wall was applying a 400 PLF total load on the joists. If the joists are spaced at 16 " O.C. and span 20 ft , then:
PLF Wall Load Portion of PLF wall load carried by joist
$\square$ Trib
Trib Area of I-joist

$$
\begin{aligned}
& P_{\text {Live Load }}=\frac{4}{5}(400 \text { PLF }) \times\left(1.33^{\prime}\right)=426 \mathrm{lbs} \\
& \mathrm{P}_{\text {Total Load }}=(400 \text { PLF }) \times\left(1.33^{\prime}\right)=532 \mathrm{lbs} \\
& \mathrm{PLF}_{\text {EQ Live Load }}=\frac{2 \times 426 \mathrm{lbs}}{20^{\prime}}=43 \mathrm{PLF} \\
& \mathrm{PLF}_{\text {EQ Total Load }}=\frac{2 \times 532 \mathrm{lbs}}{20^{\prime}}=54 \text { PLF } \\
& \text { (Assuming a } 40 / 10 \text { loading from above })
\end{aligned}
$$



These PLF loads are in addition to the original PSF Design Loads and must be added before using the table. Using the example from above, these joists should be sized to carry:
Live Load PLF: 53 PLF + 43 PLF = 96 PLF Live Load Total Load PLF: 67 PLF + 54 PLF $=121$ PLF Total Load
If a joist could not be sized, redo this with a smaller oncenter spacing or use Simpson Strong-Tie ${ }^{\circledR}$ Component Solutions ${ }^{\text {TM }}$ to size the joist more accurately.

## PSF TO PLF CONVERSION - LOAD IN POUNDS PER LINEAL FOOT (PLF)

O.C. Spacing

| (in) | (ft) | $\mathbf{2 0}$ | $\mathbf{2 5}$ | $\mathbf{3 0}$ |
| :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1 2}$ | 1.00 | 20 | 25 | 30 |
| $\mathbf{1 6}$ | 1.33 | 27 | 33 | 40 |
| $\mathbf{1 9 . 2}$ | 1.60 | 32 | 40 | 48 |
| $\mathbf{2 4}$ | 2.00 | 40 | 50 | 60 |

O.C. spacing [ft] $\times$ load [PSF] = load [PLF]. See load development above.

## Allowable Floor Uniform Load For RFP ${ }^{\circledR}$-Joists (plf)

| Joist Clear Span (ft) | RFPI 20 |  |  |  |  |  | RFPI 40S |  |  |  |  |  |  |  | RFPI 400 |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 9-1/2" |  | 11-7/8" |  | 14" |  | 9-1/2" |  | 11-7/8" |  | 14" |  | 16" |  | 9-1/2" |  | 11-7/8" |  | 14" |  | 16" |  |
|  | Live | Total | Live | Total | Live | Total | Live | Total | Live | Total | Live | Total | Live | Total | Live | Total | Live | Total | Live | Total | Live | Total |
| 6 | - | 226 | - | 247 | - | 246 | - | 275 | - | 319 | - | 319 | - | 318 | - | 274 | - | 287 | - | 287 | - | 286 |
| 7 | - | 195 | - | 212 | - | 212 | - | 237 | - | 274 | - | 274 | - | 274 | - | 236 | - | 247 | - | 247 | - | 246 |
| 8 | - | 171 | - | 186 | - | 186 | - | 208 | - | 241 | - | 240 | - | 240 | - | 207 | - | 216 | - | 216 | - | 216 |
| 9 | - | 152 | - | 166 | - | 165 | - | 185 | - | 214 | - | 214 | - | 214 | - | 184 | - | 193 | - | 192 | - | 192 |
| 10 | - | 137 | - | 149 | - | 149 | - | 167 | - | 193 | - | 193 | - | 192 | - | 166 | - | 174 | - | 173 | - | 173 |
| 11 | 116 | 125 | - | 136 | - | 135 | 133 | 151 | - | 175 | - | 175 | - | 175 | 133 | 151 | - | 158 | - | 158 | - | 157 |
| 12 | 91 | 114 | - | 124 | - | 124 | 105 | 139 | - | 161 | - | 161 | - | 160 | 105 | 138 | - | 145 | - | 145 | - | 144 |
| 13 | 73 | 105 | - | 115 | - | 115 | 84 | 123 | 139 | 148 | - | 148 | - | 148 | 84 | 128 | - | 134 | - | 133 | - | 133 |
| 14 | 59 | 98 | 99 | 107 | - | 106 | 69 | 106 | 113 | 137 | - | 138 | - | 137 | 69 | 119 | 113 | 124 | - | 124 | - | 124 |
| 15 | 49 | 91 | 82 | 99 | - | 99 | 57 | 92 | 94 | 120 | - | 128 | - | 128 | 57 | 111 | 94 | 116 | - | 115 | - | 115 |
| 16 | 41 | 80 | 68 | 93 | - | 93 | 47 | 81 | 79 | 105 | 112 | 120 | - | 120 | 47 | 92 | 79 | 108 | - | 108 | - | 108 |
| 17 | 34 | 67 | 58 | 88 | 84 | 87 | 40 | 71 | 66 | 93 | 95 | 112 | - | 113 | 40 | 77 | 66 | 102 | 96 | 102 | - | 102 |
| 18 | 29 | 56 | 49 | 83 | 71 | 82 | 34 | 63 | 56 | 83 | 81 | 100 | - | 106 | 34 | 65 | 56 | 96 | 82 | 96 | - | 96 |
| 19 | - | - | 42 | 77 | 61 | 78 | 29 | 55 | 48 | 74 | 70 | 89 | 93 | 101 | 29 | 56 | 48 | 91 | 70 | 91 | - | 91 |
| 20 | - | - | 36 | 69 | 53 | 74 | - | - | 42 | 67 | 60 | 80 | 81 | 94 | - | - | 42 | 81 | 61 | 86 | 82 | 86 |
| 21 | - | - | 32 | 61 | 46 | 70 | - | - | 36 | 60 | 52 | 73 | 71 | 85 | - | - | 36 | 70 | 53 | 82 | 71 | 82 |
| 22 | - | - | 28 | 53 | 40 | 67 | - | - | 32 | 55 | 46 | 66 | 62 | 77 | - | - | 32 | 61 | 46 | 78 | 63 | 78 |
| 23 | - | - | - | - | 36 | 62 | - | - | 28 | 50 | 41 | 60 | 55 | 70 | - | - | 28 | 54 | 41 | 73 | 55 | 75 |
| 24 | - | - | - | - | 31 | 57 | - | - | - | - | 36 | 55 | 48 | 64 | - | - | - | - | 36 | 67 | 49 | 71 |
| 25 | - | - | - | - | 28 | 52 | - | - | - | - | 32 | 51 | 43 | 59 | - | - | - | - | 32 | 62 | 44 | 68 |



| 6 | - | 287 | - | 319 | - | 319 | - | 318 | - | 275 | - | 319 | - | 319 | - | 318 |
| :---: | ---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 7 | - | 247 | - | 274 | - | 274 | - | 274 | - | 237 | - | 274 | - | 274 | - | 274 |
| 8 | - | 217 | - | 241 | - | 240 | - | 240 | - | 208 | - | 241 | - | 240 | - | 240 |
| 9 | - | 193 | - | 214 | - | 214 | - | 214 | - | 185 | - | 214 | - | 214 | - | 214 |
| 10 | - | 174 | - | 193 | - | 193 | - | 193 | - | 167 | - | 193 | - | 193 | - | 192 |
| 11 | 145 | 158 | - | 176 | - | 175 | - | 175 | - | 151 | - | 175 | - | 175 | - | 175 |
| 12 | 115 | 145 | - | 161 | - | 161 | - | 161 | 123 | 139 | - | 161 | - | 161 | - | 160 |
| 13 | 93 | 134 | - | 149 | - | 148 | - | 148 | 99 | 128 | - | 148 | - | 148 | - | 148 |
| 14 | 76 | 124 | 124 | 138 | - | 138 | - | 138 | 81 | 119 | 133 | 138 | - | 138 | - | 137 |
| 15 | 62 | 116 | 103 | 129 | - | 128 | - | 128 | 67 | 111 | 110 | 129 | - | 128 | - | 128 |
| 16 | 52 | 102 | 86 | 121 | - | 120 | - | 120 | 56 | 104 | 92 | 120 | - | 120 | - | 120 |
| 17 | 44 | 85 | 73 | 113 | 105 | 113 | - | 113 | 47 | 91 | 78 | 113 | 112 | 113 | - | 113 |
| 18 | 37 | 72 | 62 | 107 | 90 | 107 | - | 107 | 40 | 77 | 67 | 107 | 96 | 107 | - | 106 |
| 19 | 32 | 62 | 53 | 101 | 77 | 101 | - | 101 | 34 | 66 | 57 | 101 | 83 | 101 | - | 101 |
| 20 | 28 | 53 | 46 | 90 | 67 | 96 | 90 | 96 | 30 | 57 | 50 | 93 | 72 | 96 | - | 96 |
| 21 | - | - | 40 | 78 | 58 | 91 | 78 | 91 | - | - | 43 | 84 | 63 | 91 | 84 | 91 |
| 22 | - | - | 35 | 68 | 51 | 87 | 69 | 87 | - | - | 38 | 73 | 55 | 87 | 74 | 87 |
| 23 | - | - | 31 | 59 | 45 | 83 | 61 | 83 | - | - | 33 | 64 | 48 | 83 | 65 | 83 |
| 24 | - | - | 27 | 52 | 40 | 76 | 54 | 79 | - | - | 30 | 56 | 43 | 77 | 58 | 79 |
| 25 | - | - | - | - | 36 | 68 | 48 | 76 | - | - | 26 | 50 | 38 | 71 | 52 | 76 |
| 26 | - | - | - | - | 32 | 61 | 43 | 73 | - | - | - | - | 34 | 65 | 46 | 73 |
| 27 | - | - | - | - | 28 | 54 | 38 | 68 | - | - | - | - | 31 | 58 | 41 | 70 |
| 28 | - | - | - | - | - | - | 35 | 63 | - | - | - | - | 28 | 52 | 37 | 65 |
| 29 | - | - | - | - | - | - | 31 | 59 | - | - | - | - | - | - | 34 | 61 |
| 30 | - | - | - | - | - | - | 28 | 54 | - | - | - | - | - | - | 31 | 57 |

To Use PLF Table:

1. Select the span required.
2. Compare the design total load (plf) to the Total column and compare the design live load (plf) to the Live column
3. Select a product that meets or exceeds both the design total and live loads. When no value is shown in the Live column, Total load will govern.

## General Notes:

1. Table values apply to uniformly loaded simple or multiple span joists.
2. Clear span is the clear distance between the face of supports.
3. Live load column is based on an $L / 480$ deflection limit.
4. An L/480 live load deflection limit is recommended (see Floor System Performance on page 6). For L/360 (minimum stiffness allowed by code), multiply the L/480 value by 1.33 .
5. Total load column is based on an L/240 deflection limit
6. Verify that the deflection criteria conform to local building code requirements.
7. Total load is based on $100 \%$ duration of load.
8. Minimum end bearing length is $1-3 / 4$ ". Minimum intermediate bearing length is $3-1 / 2$ ".
9. Web stiffeners are not required for loads shown
10. This table does not account for added stiffness from glued or nailed sheathing.
11. Use appropriate software (e.g. Simpson Strong-Tie ${ }^{\circledR}$ Component Solutions ${ }^{\text {™ }}$ ) or engineering analysis to analyze multiple span joists if the length of any span is less than half the length of an adjacent span.
12. Use appropriate software or engineering analysis to analyze conditions outside of the scope of this table such as cantilevers and concentrated loads.
13. Provide lateral support at bearing points and continuous lateral support along the compression flange of each joist.
14. For double joists, double the table values and connect the joists per the detail on page 22.
15. For proper installation procedures refer to the appropriate sections in this publication

| Joist Clear Span (ft) | RFPI 70 |  |  |  |  |  |  |  | RFPI 80S |  |  |  |  |  | RFPI 90 |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 9-1/2" |  | 11-7/8" |  | 14" |  | 16" |  | 11-7/8" |  | 14" |  | 16" |  | 9-1/2" |  | 11-7/8" |  | 14" |  | 16" |  |
|  | Live | Total | Live | Total | Live | Total | Live | Total | Live | Total | Live | Total | Live | Total | Live | Total | Live | Total | Live | Total | Live | Total |
| 6 | - | 298 | - | 319 | - | 318 | - | 318 | - | 358 | - | 384 | - | 384 | - | 385 | - | 428 | - | 427 | - | 427 |
| 7 | - | 256 | - | 274 | - | 274 | - | 274 | - | 308 | - | 331 | - | 330 | - | 331 | - | 368 | - | 368 | - | 367 |
| 8 | - | 225 | - | 240 | - | 240 | - | 240 | - | 270 | - | 290 | - | 290 | - | 290 | - | 323 | - | 322 | - | 322 |
| 9 | - | 200 | - | 214 | - | 214 | - | 214 | - | 240 | - | 258 | - | 258 | - | 258 | - | 287 | - | 287 | - | 287 |
| 10 | - | 180 | - | 193 | - | 193 | - | 192 | - | 216 | - | 232 | - | 232 | - | 233 | - | 259 | - | 258 | - | 258 |
| 11 | - | 164 | - | 175 | - | 175 | - | 175 | - | 197 | - | 211 | - | 211 | - | 212 | - | 235 | - | 235 | - | 235 |
| 12 | 138 | 150 | - | 161 | - | 161 | - | 160 | - | 180 | - | 194 | - | 193 | 191 | 194 | - | 216 | - | 215 | - | 215 |
| 13 | 112 | 139 | - | 148 | - | 148 | - | 148 | - | 166 | - | 179 | - | 179 | 156 | 179 | - | 199 | - | 199 | - | 199 |
| 14 | 91 | 129 | - | 138 | - | 138 | - | 137 | - | 154 | - | 166 | - | 166 | 129 | 166 | - | 185 | - | 185 | - | 184 |
| 15 | 76 | 120 | 124 | 129 | - | 128 | - | 128 | - | 144 | - | 155 | - | 154 | 107 | 155 | 172 | 173 | - | 172 | - | 172 |
| 16 | 63 | 113 | 104 | 120 | - | 120 | - | 120 | 122 | 135 | - | 145 | - | 145 | 90 | 145 | 146 | 162 | - | 161 | - | 161 |
| 17 | 53 | 104 | 88 | 113 | - | 113 | - | 113 | 104 | 127 | - | 136 | - | 136 | 77 | 137 | 124 | 152 | - | 152 | - | 151 |
| 18 | 45 | 88 | 76 | 107 | - | 107 | - | 106 | 89 | 120 | 127 | 129 | - | 128 | 65 | 127 | 107 | 143 | - | 143 | - | 143 |
| 19 | 39 | 76 | 65 | 101 | 94 | 101 | - | 101 | 77 | 113 | 109 | 122 | - | 121 | 56 | 109 | 92 | 136 | 131 | 135 | - | 135 |
| 20 | 34 | 65 | 56 | 96 | 81 | 96 | - | 96 | 67 | 107 | 95 | 115 | - | 115 | 49 | 94 | 80 | 129 | 114 | 129 | - | 128 |
| 21 | 29 | 56 | 49 | 91 | 71 | 91 | - | 91 | 58 | 102 | 83 | 110 | - | 110 | 43 | 82 | 70 | 123 | 100 | 122 | - | 122 |
| 22 | - | - | 43 | 83 | 62 | 87 | 84 | 87 | 51 | 97 | 73 | 105 | 98 | 104 | 37 | 71 | 62 | 117 | 88 | 117 | - | 116 |
| 23 | - | - | 38 | 73 | 55 | 83 | 74 | 83 | 45 | 86 | 65 | 100 | 87 | 100 | 33 | 62 | 55 | 105 | 78 | 111 | 104 | 111 |
| 24 | - | - | 34 | 64 | 49 | 79 | 66 | 79 | 40 | 76 | 58 | 96 | 77 | 95 | 29 | 55 | 49 | 93 | 70 | 107 | 93 | 106 |
| 25 | - | - | 30 | 57 | 44 | 76 | 59 | 76 | 36 | 67 | 51 | 92 | 69 | 92 | - | - | 43 | 83 | 62 | 102 | 83 | 102 |
| 26 | - | - | 27 | 51 | 39 | 73 | 53 | 73 | 32 | 60 | 46 | 88 | 62 | 88 | - | - | 39 | 74 | 56 | 98 | 74 | 98 |
| 27 | - | - | - | - | 35 | 67 | 47 | 70 | 29 | 53 | 41 | 78 | 55 | 85 | - | - | 35 | 66 | 50 | 94 | 67 | 94 |
| 28 | - | - | - | - | 32 | 60 | 43 | 68 | - | - | 37 | 70 | 50 | 81 | - | - | 31 | 59 | 45 | 86 | 61 | 91 |
| 29 | - | - | - | - | 29 | 54 | 39 | 65 | - | - | 34 | 63 | 45 | 78 | - | - | 28 | 53 | 41 | 78 | 55 | 87 |
| 30 | - | - | - | - | - | - | 35 | 63 | - | - | 31 | 57 | 41 | 76 | - | - | - | - | 37 | 70 | 50 | 84 |

## Floor Framing \& Construction Details

Some framing elements such as blocking panels have been omitted for clarity.


## tYPICAL RFPI-JOIST FLOOR FRAMING AND CONSTRUCTION DETAILS

All nails shown in the details below are assumed to be common nails unless otherwise noted. 10 d box nails may be substituted for 8 d common nails shown in details. If nails must be installed into the sides of LVL flanges, see table on page 8 for "Recommended Nail Size and Spacing". Individual components not shown to scale for clarity.
(1a) blocking panels


RFP ${ }^{\circledR}$ blocking panel vertical load transfer $=2,000$ plf maximum -or- RigidRim Rimboard (see Page 35 for design properties)

Attach I-joist to top plate per 1b

Attach blocking panel (or Rimboard) to top plate with 8d nails at 6" o.c. (when used for lateral shear transfer, nail to bearing plate with same nailing as required for decking)

## Floor Framing \& Construction Details

Some framing elements such as blocking panels have been omitted for clarity.
(1b) RIGIDRIM ${ }^{\circledR}$ RIMBOARD


RigidRim ${ }^{*}$ Rimboard
(see page 35 for design properties)
One 8d nail at top and bottom flange

Attach RigidRim ${ }^{\circ}$ Rimboard to top plate using 8 d box toenails at 6 " o.c.

One 8 d nail each side of the RFPI-Joist at bearing. To avoid splitting flange, install nails a minimum of 1-1/2" from end of I-joist. Nails may be driven at an angle to avoid splitting of bearing plate.


1c) RFPI® RIM JOIST


RFPI ${ }^{\circ}$ Rim Joist vertical load transfer $=2,000$ plf maximum

Attach rim joist to floor joist with one nail at top and bottom. Nail must provide one inch minimum penetration into floor joist. For rim joist with flanges 2" and wider toenails may be used.

Minimum 1-3/4" bearing required ( $2 \times 6$ bearing plate required for rim joists with flange widths greater than 1-3/4")

## BLOCKING PANELS AND RIM JOIST

Blocking panels and rim joist prevent floor joists from overturning and help transfer loads through the floor system into the structure below. Due to differences in depth and possible shrinkage, common framing lumber set on edge is unacceptable as blocking. I-joist blocking panels must be cut to the proper length to fit between the l-joists, and their depth must match the depth of the l-joists.

Blocking panels may be used:

1. To stabilize l-joists laterally at supports, as shown in Figures 1 a and 1 g . Lateral support is required during installation and is necessary to obtain design carrying capacity.
2. To transmit vertical loads up to 2,000 plf per blocking panel in accordance with Figures 1a, 1c, 1f, and 1 g .
3. For closures such as that shown in Figures 1a and 1e.
4. To transmit lateral forces to shear walls. Shear transfer nailing into the flanges must be specified by the building designer.
5. To provide lateral stability to walls.
6. Do not connect deck ledgers to web of blocking panels or rim joist.

1d SQUASH BLOCK DETAIL

(1e) BEARING BLOCK DETAIL


Use single I-joist for loads up to 2,000 plf, double l-joists for loads up to 4,000

(1i) HANGER TO LVL BEAM DETAIL


Top- or face-mounted hanger installed per hanger manufacturer's recommendations
Note: Unless hanger sides laterally support the top flange, web stiffeners shall be used. (See Figure B on page 24)

## See pages

 41 and 42 for details on attaching multiple ply LVL beams.
## 1 g rfPI BLOCKING PANELS AT INTERIOR SUPPORT



RFPI ${ }^{\circledR}$ blocking panel vertical load transfer $=2,000$ plf maximum -orRigidRim Rimboard (see Page 35 for design properties)
(1k) HANGER TO $2 X$ PLATE DETAIL


Top-mounted hanger installed per hanger manufacturer's recommendations

Note: Unless hanger sides laterally support the top flange, web stiffeners shall be used. (See Figure B on page 24)

## Multiple I-joist

header with full depth filler block shown. RigidLam ${ }^{\circledR}$ LVL headers may also be used. Verify double I-joist capacity to support concentrated loads.

## (1p) HEADER DETAIL



## BACKER BLOCK DEPTH

| Joist Depth | 9-1/2" | 11-7/8" | 14" | 16" |
| :---: | :---: | :---: | :---: | :---: |
| Top Mount Hangers - Min Backer Block Depth | 5-1/2" | 5-1/2" | 7-1/4" | 7-1/4" |
| Face Mount Hangers <br> - Req'd Backer Block Depth | 6-1/4" | 8-5/8" | 10-3/4" | 12-3/4" |



Note: Blocking required at bearing for lateral support, not shown for clarity.

## backer block and header detail

Backer block required for face-mount hangers (both sides of I-joist) \& when top mount hanger load exceeds 250 lbs
See tables for backer block thickness \& depth.
Install backer block tight to the top flange.
Attach backer block to web with 16-10d common nails, clinched. See table for maximum capacity for this detail.

Backer block must be wide enough to permit required nailing without splitting (min width of 12 " recommended)

## GENERAL NOTES:

For hanger capacity see hanger manufacturer recommendations.
Verify I-joist capacity to support concentrated load from "header joist" in addition to all other loads.

If a double l-joist is required to support "header joist" load, refer to page 22 for double l-joist connection guidelines.
Before installing a backer block to a double I-joist, drive four additional 10d nails from both sides of double l-joist through the webs and filler block at backer block location. Clinch nails.

| -Joist <br> Flange <br> Width | Backer block <br> Material <br> Thickness <br> Required(a)(b) | Max. load capacity <br> using $16-10 \mathrm{l}$ |
| :---: | :---: | :---: |
| com. nails |  |  |

(a) Minimum grade for backer material shall be Utility grade SPF or better for solid sawn lumber and Rated Sheathing grade for wood structural panels.
(b) Glue 2-ply backer blocks together with construction grade adhesive (ASTM D-3498)

## Cantilever Details

Please refer to note 6 on page 8 .

RFPI®-JOIST INTERIOR CANTILEVER DETAIL


## Lumber cantilever detail for exterior and interior balconies

Full depth backer block with 1/8" gap between block and top flange of I-joist. See Detail 1p. Nail with two rows of 10d nails @ 6" o.c. and clinch.
$2 \times 8$ min $^{(1)}$ Nail to backer block and joist with two rows of 10d nails @ 6" o.c. and clinch. (Cantilever nails may be used to attach backer block if length

(1) See APA Technical Topic TT-125 for additional information regarding required size, grade and design considerations for lumber cantilevers.
cantilever detail for vertical building offset - [refer to table on page 23 for recommended reinforcement)

METHOD 1
Sheathing Reinforcement One Side


| APA RATED SHEATHING |  |
| :---: | :---: |
| Strength Axis |  |
|  | $4^{\prime}$ |

## Cantilever Details

## CANTILEVER DETAIL FOR VERTICAL BUILDING OFFSET

ALTERNATIVE METHOD 2
Double RFPI ${ }^{\circ}$-Joist


Attach RFP ${ }^{\circledR}$-Joist blocking panel or RigidRim ${ }^{\circledR}$ Rimboard blocking to top plate per Detail 1a

Block I-joists together with filler blocks for the full length of the reinforcement, sized and attached in accordance with Figure A below. For I-joist flange widths greater than three inches place an additional row of 10 d nails along the centerline of the reinforcing panel from each side. Clinch when possible.

Filler block does not function as a web stiffener. If web stiffeners are required it is recommended to install continuous filler block and install web stiffener below filler block prior to attaching I-joist reinforcement. Leave a $1 / 8$ "- $1 / 4$ " gap between top of filler block and bottom of top I-joist flange. Web stiffeners must be tight between top of bottom flange and bottom of filler block.
Figure A
Double RFPI ${ }^{\circ}$-Joist Construction
 filler block

## Notes:

1. Double I-joists may be required to frame openings, support concentrated loads, support partitions parallel to floor joists, or support any other loads which would exceed the capacity of a single I-joist. Install double I-joists when noted in the building drawings.
2. Filler blocks do not function as web stiffeners. Install web stiffeners as required.
3. Support back of I-joist web during nailing to prevent damage to web/flange connection.
4. Leave a $1 / 8^{\prime \prime}-1 / 4^{\prime \prime}$ gap between top of filler block and bottom of top I-joist flange.
5. For side-loaded conditions or cantilever reinforcement, filler block is required between joists for full length of double member.
6. Nail joists together with two rows of 10 d nails at 6 inches o.c. (staggered) on each side of the double I-joist. Total of eight nails per foot required.
7. Filler block thickness may be achieved by using multiple layers of structural wood panels.
8. The maximum load that may be applied to one side of the double joist using this detail is $620 \mathrm{lbs} / \mathrm{ft}$


## TABLE A: FILLER BLOCK

REQUIREMENTS FOR DOUBLE RFPI@-JOIST CONSTRUCTION
$\square$

## RFPI®-Joist Cantilever Reinforcement



See table below for RFPI-Joist reinforcement requirements at cantilever.


For hip roofs with the hip trusses running parallel to the cantilevered floor joists, the I-joist reinforcement requirements for a span of 26 ' may be used.

RFPI@-JOIST CANTILEVER REINFORCEMENT METHODS ALLOWED

| Joist Depth (in) | Roof Truss Span (ft) | ROOF LOADINGS |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | $\text { TL = } 35 \text { psf }$ <br> LL not to exceed 20 psf |  |  |  | $\mathrm{TL}=45 \mathrm{psf}$ <br> LL not to exceed 30 psf |  |  |  | $\text { TL = } 55 \text { psf }$ <br> LL not to exceed 40 psf |  |  |  |
|  |  | Joist Spacing (in) |  |  |  | Joist Spacing (in) |  |  |  | Joist Spacing (in) |  |  |  |
|  |  | 12 | 16 | 19.2 | 24 | 12 | 16 | 19.2 | 24 | 12 | 16 | 19.2 | 24 |
| 9-1/2" | 26 | N | N | N | 1 | N | N | 1 | 2 | N | 1 | 2 | X |
|  | 28 | N | N | N | 1 | N | N | 1 | 2 | N | 1 | 2 | X |
|  | 30 | N | N | N | 1 | N | N | 1 | 2 | N | 1 | 2 | x |
|  | 32 | N | N | 1 | 2 | N | 1 | 1 | X | N | 1 | 2 | X |
|  | 34 | N | N | 1 | 2 | N | 1 | 2 | X | N | 2 | X | X |
|  | 36 | N | N | 1 | 2 | N | 1 | 2 | X | N | 2 | X | X |
| 11-7/8" | 26 | N | N | N | 1 | N | N | 1 | 1 | N | N | 1 | 1 |
|  | 28 | N | N | N | 1 | N | N | 1 | 1 | N | 1 | 1 | 2 |
|  | 30 | N | N | N | 1 | N | N | 1 | 1 | N | 1 | 1 | 2 |
|  | 32 | N | N | N | 1 | N | N | 1 | 1 | N | 1 | 1 | 2 |
|  | 34 | N | N | 1 | 1 | N | N | 1 | 2 | N | 1 | 1 | 2 |
|  | 36 | N | N | 1 | 1 | N | 1 | 1 | 2 | N | 1 | 1 | 2 |
|  | 38 | N | N | 1 | 1 | N | 1 | 1 | 2 | N | 1 | 2 | X |
| 14" | 26 | N | N | N | 1 | N | N | 1 |  | N | N | 1 | 2 |
|  | 28 | N | N | N | 1 | N | N | 1 | 1 | N | 1 | 1 | 2 |
|  | 30 | N | N | N | 1 | N | N | 1 | 1 | N | 1 | 1 | 2 |
|  | 32 | N | N | 1 | 1 | N | N | 1 | 2 | N | 1 | 1 | 2 |
|  | 34 | N | N | 1 | 1 | N | 1 | 1 | 2 | N | 1 | 1 | 2 |
|  | 36 | N | N | 1 | 1 | N | 1 | 1 | 2 | N | 1 | 1 | 2 |
|  | 38 | N | N | 1 | 1 | N | 1 | 1 | 2 | N | 1 | 2 | 2 |
|  | 40 | N | N | 1 | 1 | N | 1 | 1 | 2 | N | 1 | 2 | X |
| 16" | 26 | N | N | N | 1 | N | N | N | 1 | N | N | 1 | 1 |
|  | 28 | N | N | N | 1 | N | N | N | 1 | N | N | 1 | 1 |
|  | 30 | N | N | N | 1 | N | N | N | 1 | N | N | 1 | 2 |
|  | 32 | N | N | N | 1 | N | N | 1 | 1 | N | N | 1 | 2 |
|  | 34 | N | N | N | 1 | N | N | 1 | 1 | N | 1 | 1 | 2 |
|  | 36 | N | N | N | 1 | N | N | 1 | 1 | N | 1 | 1 | 2 |
|  | 38 | N | N | N | 1 | N | N | 1 | 2 | N | 1 | 1 | 2 |
|  | 40 | N | N | N | 1 | N | N | 1 | 2 | N | 1 | 1 | 2 |
|  | 42 | N | N | 1 | 1 | N | 1 | 1 | 2 | N | 1 | 2 | 2 |

Cantilever Reinforcement Legend:
$\mathrm{N}=$ No reinforcement required.
$1=$ RFPI"-Joists reinforced with 23/32" Wood Structural panel or RigidRim ${ }^{*}$ Rimboard on one side only (see Method 1 on Page 21).
$2=$ RFPl"-Joists reinforced with 23/32" Wood Structural panel or RigidRim ${ }^{*}$ Rimboard on both sides or double I-joist (see Method 2 on Page 21 or alternate Method 2 on Page 22),
$\mathrm{X}=$ Try a deeper joist or closer spacing.

Note: For more information see pages 21 \& 22

## Notes:

- Maximum load shall be: Total roof load as shown in table (includes 15 psf roof dead load), 50 psf floor total load, and 80 plf wall load. Wall load is based on $3^{\prime}-0$ " maximum width window or door opening. For larger openings, or multiple $3^{\prime}-0$ " width openings spaced less than $6^{\prime}-0$ " o.c., additional joists beneath the opening's trimmer/jack studs may be required.
- Table applies to joists 12 " to 24 " o.c. Use 12 " o.c. requirements for o.c. spacings less than 12 ".
- For a given I-joist depth, table conservatively accounts for multiple I-joist series.
- For conditions other than those shown or to analyze a specific I-joist series, software with the appropriate design properties, such as Simpson Strong-Tie ${ }^{\circledR}$ Component Solutions ${ }^{\text {TM }}$ software, can be used to analyze specific applications and loading.


## Web Stiffener Requirements

A web stiffener is a block of plywood, OSB, or even a $2 \times 4$ that is added to stiffen the l-joist's web, increase the bearing surface between the web and the flange, and provide additional support for a hanger or other connector. Web stiffeners are common with certain types of joist hanger installations, particularly in roof systems. They are typically placed at the end of the l-joist, between the flanges and against both sides of the web. When used at end bearings, web stiffeners should be installed tight against the bottom flange of the l-joist, but with a minimum $1 / 8$ " gap between the top of the stiffener and the bottom of the top flange. Web stiffeners must be made of utility grade SPF (south) or better for lumber and/or sheathing grade or better for wood structural panels.
When designed in accordance with the load/span conditions set forth in the tables in this guide, RFPI-Joists do not require web stiffeners, with the following exceptions:

- When sides of the hangers do not laterally brace the top flange of each I-joist.
- Birds mouth cuts for roof joists.
- When I-joists are designed to support concentrated loads greater than 1,500 lbs applied to the l-joist's top flange between supports. In these applications only, the gap between the web stiffener and the flange shall be at the bottom flange (see Figure B below).

Web stiffeners may be cut in the field as required for the application.
The use of web stiffeners or bearing lengths that are longer than the minimum required may result in allowable spans that are longer than those shown in this guide. The most accurate method of determining if a joist is adequate and if web stiffeners are required is to use appropriate software (e.g. Simpson Strong-Tie ${ }^{\circledR}$ Component Solutions ${ }^{\text {TM }}$ ) or engineering analysis for the actual conditions.

FIGURE B

## rFPI-JoIST WEB STIFFENER REOUIREMENTS



Flange width greater than 1-3/4"


CONCENTRATED LOAD END BEARING
(Load Stiffener)

(Bearing Stiffener)


TABLE B: WEB STIFFENER SIZE REQUIRED

| RFPI ${ }^{\circledR}$-Joist Flange Width | Web Stiffener Size Each Side of Web |
| :---: | :---: |
| 1-3/4" | 19/32" $\times 2-5 / 16$ " minimum width |
| 2-1/16" | $3 / 4$ " $\times 2-5 / 16$ " minimum width |
| 2-5/16" | $7 / 8^{\prime \prime} \times 2-5 / 16$ " minimum width |
| 2-1/2" | $1 " \times 2-5 / 16$ " minimum width |
| 3-1/2" | $1-1 / 2$ " $\times 2-5 / 16$ " minimum width |

## Fire \& Sound Rated Floor Assemblies

## FIRE-RATED I-JOIST FLOOR-CEILING ASSEMBLIES

Wood I-joists have been used successfully in fire-rated floor assemblies for many years. Several I-joist fire-rated assemblies (1-hour and 2-hour) have been published that are applicable to I-joists that meet or exceed the required specifications provided in the fire-rated assembly description. These "generic" assemblies can be found in the American Wood Council (AWC) publication entitled "Design for Code Acceptance 3" (DCA 3). Most of these DCA 3 assemblies have been adopted by the International Building Code (IBC) and can be found in Table 721.1(3) of the 2012, 2015, 2018, and 2021 IBC. Additional fire-rated systems and associated information can be found in the APA ICC-ES code report ESR-1405 and various other APA publications. The Roseburg ICC-ES I-joist code report, ESR-1251, and APA Product Report, PR-S259, list the various IBC and APA fire-rated floor-ceiling assemblies for which RFPI-Joists have specific code approval. The website addresses for these organizations are as shown below.



The fire-rated assembly shown at left is one of the more common assemblies shown in DCA 3 (WIJ-1.6) and published in the 2012, 2015, 2018, and 2021 IBC (Item 27-1.1) and can be used with any of the RFPI ${ }^{\circledR}$ Joist series and depths,

1. Tests have shown that substitution of OSB or composite APA Rated Sturd-I-Floor for plywood panels in fire-rated single-layer assemblies will not jeopardize fire-resistance ratings. Substitution is based on equivalent panel thickness. OSB panels are listed as alternates to plywood for finish flooring in accordance with product evaluation reports for APA PRI trademarked I-joists.
2. For improved acoustical performance, gypsum wallboard is fastened to resilient metal furring channels in some assemblies.
3. Construction adhesive must conform to APA Specification AFG-01, or ASTM D3498.

| $\begin{gathered} \text { APA } \\ \text { PR-S259 } \end{gathered}$ | American Wood Council DCA3 | $\begin{gathered} 2012,2015,2018,2021 \\ \text { IBC Table 721.1(3) } \end{gathered}$ | APA "Fire Rated Systems" W305 | APA ICC-ES <br> Report ESR-1405 | Duration | RFP-Joist series that meet the assembly requirements |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| RFP 1.1 | WIJ-1.1 | Item 24-1.1 | Fig. 4.3A | - | 1 hr . | RFPI 80S, 90 and 900 |
| RFP 1.2 | WIJ-1.2 | Item 25-1.1 | Fig. 4.3B | - | 1 hr . | RFPI 90 and 900 |
| RFP 1.3 | WIJ-1.3 | Item 23-1.1 | Fig. 4.3C | - | 1 hr . | All RFPI series |
| RFP 1.4 | WIJ-1.4 | - | Fig. 4.3D | - | 1 hr . | RFPI 40S, 60S, 70, 80S, 90, 700 and 900 |
| RFP 1.5 | WIJ-1.5 | - | Fig. 4.3E | - | 1 hr . | RFPI 40S, 60S, 70, 80S, 90, 700 and 900 |
| RFP 1.6 | WIJ-1.6 | Item 27-1.1 | Fig. 4.3F | - | 1 hr . | All RFPI series |
| RFP 1.7 | WIJ-1.7 | Item 30-1.1 | - | - | 1 hr . | RFPI 40S, 60S, 70, 80S, 90, 700 and 900 |
| RFP 1.7a | - | - | - | - | 1 hr . | All RFPI series |
| RFP 1.8 | - | Item 26-1.1 | - | - | 1 hr . | RFPI 40S, 60S, 70, 80S, 90, 700 and 900 |
| RFP 1.9 | - | Item 21-1.1 | - | Assembly 2 | 1 hr . | All RFPI series |
| RFP 1.10 | - | - | - | Assembly 4 | 1 hr . | RFPI 40S, 60S, 80S, 90 and 900 |
| - | - | - | - | Assembly 1 | 1 hr .. | RFPI 40S, 60S, 80S, 90 and 900 |
| - | - | - | - | Assembly 3 | 1 hr . | All RFPI series |
| RFP 2.1 | WIJ-2.1 | Item 28-1.1 | Fig. 5 | - | 2 hr . | RFPI 40S, 60S, $70,80 \mathrm{~S}, 90,700$ and 900 |

## SOUND-RATED FLOOR ASSEMBLY WITH APA PERFORMANCE RATED I-JOISTS

The sound-rated assembly shown at right is one of several assemblies that can be used with I-Joists. For additional STC and IIC sound rating systems for Roseburg RFPI-Joists, refer to APA Product Report PR-S259. Additional general information regarding STC and IIC sound ratings can be found in APA Form No.W460 (http://apawood.org/publications).


EXAMPLE: Sound Ratings for Floors Using APA Performance Rated RFP@-Joist

| Test Sponsor and Number ${ }^{1}$ | Floor | Deck | Gypsum Wallboard Ceiling | Insulation | STC <br> Rating | IIC Rating | $\begin{aligned} & \text { Weight } \\ & \text { (lbs/sq. ft) } \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| G\&H USDA 11 ST | Vinyl Tile | $1-1 / 2$ " of $100-\mathrm{pcf}$ cellular concrete over 3/4" APA Rated Sheathing subfloor on I-joist at 24" o.c. | 5/8" screwed to resilient metal channels | 3" glass fiber | 58 | 50 | 21.0 |
| G\&H USDA 11x ST | Carpet \& Pad |  |  |  | 58 | 77 |  |
|  | None |  |  | None | 57 | None | 20.7 |

1. USDA Forest Service Wood Construction Research (Seattle, WA); acoustical tests by Geiger \& Hamme, Inc. (Ann Arbor, MI)

## SPRINKLER ATTACHMENT

See APA-The Engineered Wood Association publication J745 "Sprinkler Pipe Installation for APA Performance Rated I-Joists" and supplement for sprinkler attachment guidelines.

## Bonus Room Floor Joist Selection Guide

|  |  | RFPI SERIES |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 12" o.c. | 16" o.c. | 19.2" o.c. | 24" o.c. |
| $20^{\prime}$ | $4^{\prime}$ to $6{ }^{\prime}$ | 14 " $20 \cdot 11-7 / 8{ }^{\text {" }} 40$ | $16 " 40 S \cdot 14{ }^{\prime \prime} 400 \cdot 11-7 / 8{ }^{\prime \prime} 80 S$ | 16" $400 \cdot 14$ " $40 \cdot 11-7 / 8{ }^{\prime \prime} 90$ | $16^{\prime \prime} 70^{\text {a }}$ • $14{ }^{\text {" }} 80 \mathrm{~S}^{\text {a }}$ |
| $22^{\prime}$ | $4^{\prime}$ to $6^{\prime}$ | 14" 40S • 11-7/8" 80 S | $16 " 400 \cdot 14$ " $70 \cdot 11-7 / 8{ }^{\prime \prime} 90$ | $16^{\prime \prime} 40^{\text {a }}$ • 14" $80 \mathrm{~S}^{\text {a }}$ | $14^{\prime \prime} 90^{\text {a }}$ |
| $24^{\prime}$ | $4^{\prime}$ to $7^{\prime}$ | 16 " $40 S \cdot 14$ " $70 \cdot 11-7 / 8{ }^{\prime \prime} 90$ | $16 \mathrm{Cl} 605 \cdot 14 \mathrm{C} 90$ | $16^{\prime \prime} 80{ }^{\text {a }}$ | $16^{\prime \prime} 90 \mathrm{ab}$ |

a) Install Concentrated Load Stiffeners to Floor I-joists below Knee Walls (see page 24)
b) Install web stiffeners to each end of $I$-Joist.


## Design Parameters:

1. Roof live load of 30 psf at $115 \%$ (snow load).
2. Roof dead load of 12 psf (asphalt shingles).
3. Roof rafter slope between $8 / 12$ and $12 / 12$.
4. Knee wall weight of 40 plf dead load.
5. Attic storage load of 20 psf live load (outside the knee walls).
6. Floor live load of 40 psf (within the knee walls).
7. Attic and floor dead load of 10 psf .
8. Straight gable roof framing. No hip framing is permitted.
9. Maximum floor deflection is limited to L/480 live load and L/240 total load.
10. Spans are based on composite action with glued-nailed sheathing.
11. For all other conditions, call your local representative.
12. Consult local building code for other bonus room framing and/or loading requirements or restrictions.

## Roof Framing \& Construction Details



## TYPICAL RFPI®-JOIST ROOF FRAMING AND CONSTRUCTION DETAILS

All nails shown in the details below are assumed to be common nails unless otherwise noted. 10d box nails may be substituted for 8d common shown in details. If nails must be installed into the sides of LVL flanges, see table on page 8 for "Recommended Nail Size and Spacing". Individual components not shown to scale for clarity.

## (2a)



Uplift connections may be required.
(2b) UPPER END, BEARING ON WALL
RFPI ${ }^{\circledR}$-Joist blocking panel, $x$-bracing, 23/32" APA Rated Sheathing $48 / 24$, or proper depth of rimboard as continuous closure. (Validate use of $x$-bracing with local building code.)


## Attach blocking panel (or

Rimboard) to top plate with 8d nails at 6" o.c. (when used for lateral shear transfer, nail to bearing plate with same nailing as required for decking)

## (2c) rfpi@-Jolsts above ridge support beam



Uplift connections may be required.
(2d) BIRDSMOUTH CUT - LOW END OF RFPI®-JOIST ONLY


Birdsmouth cut RFPI*-Joist to provide full bearing for bottom flange. Cut must not overhang inside face of plate.

Uplift connections may be required.


BIRDSMOUTH CUT, NO OVERHANG LOW END OF RFPIO-JOIST ONLY


2f) BIRDSMOUTH CUT-LOW END OF RFPI®-JOIST ONLY


Uplift connections may be required.


2g) ROOF OPENINGS, FACE MOUNTED HANGERS


## 2h BEVELED CUT BEARING STIFFENER

Bevel cut web stiffener to match roof slope


Birdsmouth permitted on low end of RFPl ${ }^{\text {- }}$-Joist only

Uplift connections may be required.

## (2m) OVERHANG PARALLEL TO RFPI•-JOIST

 joist spacing, double joist may be

$2 \times 4$ outrigger notched around top flange of RFPI ${ }^{\circ}$-Joist.
(2k) OPTIONAL OVERHANG EXTENSIONS


## Uplift connections may be required.

## Slope Length Conversion Chart



## ALONG-THE-SLOPE SPANS \& CUTTING LENGTHS FOR SLOPED ROOFS

| Slope | Slope <br> Factor | Joist Depth (in) |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 9-1/2" | 11-7/8" | 14" | 16" |
|  |  | Depth Correction (ft) |  |  |  |
| 1 in 12 | 1.00 | 0.07 | 0.08 | 0.10 | 0.11 |
| 2 in 12 | 1.01 | 0.13 | 0.16 | 0.19 | 0.22 |
| 2.5 in 12 | 1.02 | 0.16 | 0.21 | 0.24 | 0.28 |
| 3 in 12 | 1.03 | 0.20 | 0.25 | 0.29 | 0.33 |
| 3.5 in 12 | 1.04 | 0.23 | 0.29 | 0.34 | 0.39 |
| 4 in 12 | 1.05 | 0.26 | 0.33 | 0.39 | 0.44 |
| 4.5 in 12 | 1.07 | 0.30 | 0.37 | 0.44 | 0.50 |
| 5 in 12 | 1.08 | 0.33 | 0.41 | 0.49 | 0.56 |
| 6 in 12 | 1.12 | 0.40 | 0.49 | 0.58 | 0.67 |
| 7 in 12 | 1.16 | 0.46 | 0.58 | 0.68 | 0.78 |
| 8 in 12 | 1.20 | 0.53 | 0.66 | 0.78 | 0.89 |
| 9 in 12 | 1.25 | 0.59 | 0.74 | 0.88 | 1.00 |
| 10 in 12 | 1.30 | 0.66 | 0.82 | 0.97 | 1.11 |
| 11 in 12 | 1.36 | 0.73 | 0.91 | 1.07 | 1.22 |
| 12 in 12 | 1.41 | 0.79 | 0.99 | 1.17 | 1.33 |

Blocking between outriggers.
Attach blocking to top plate with nail size and spacing used for roof sheathing edge nailing. Uplift connections may be required.

# Allowable Roof Uniform Load For RFPI®－Joists（plf） 

|  | RFPI 20 |  |  |  |  |  |  |  |  | RFPI 40S |  |  |  |  |  |  |  |  |  |  |  | RFPI 400 |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 9－1／2＂ |  |  | 11－7／8＂ |  |  | 14＂ |  |  | 9－1／2＂ |  |  | 11－7／8＂ |  |  | 14＂ |  |  | 16＂ |  |  | 9－1／2＂ |  |  | 11－7／8＂ |  |  | 14＂ |  |  | 16＂ |  |  |
|  | Live | Total |  | Live | Total |  | Live | Total |  | Live | Total |  | Live | Total |  | Live | Total |  | Live | Total |  | Live | Total |  | Live | Total |  | Live | Total |  | Live | Total |  |
|  | $\stackrel{\text { O}}{\mathrm{j}}$ | $\begin{aligned} & \stackrel{\circ}{7} \\ & \stackrel{7}{7} \end{aligned}$ | $\stackrel{\stackrel{\circ}{\mathrm{N}}}{ }$ | $\stackrel{\text { O}}{\mathrm{j}}$ | $\begin{aligned} & \stackrel{\circ}{7} \\ & \stackrel{7}{7} \end{aligned}$ | $\stackrel{\text { ®욱 }}{\substack{2}}$ | $\stackrel{\text { O}}{\mathrm{j}}$ | $\begin{aligned} & \stackrel{\circ}{\circ} \\ & \stackrel{7}{7} \end{aligned}$ | 苓 | $\stackrel{\text { O}}{\mathrm{j}}$ | $\stackrel{\circ}{7}$ | 윽 | $\stackrel{\text { O}}{\mathrm{j}}$ | $\begin{aligned} & \stackrel{\circ}{7} \\ & \stackrel{7}{7} \end{aligned}$ | $\begin{aligned} & \stackrel{\circ}{\circ} \\ & \stackrel{\sim}{7} \end{aligned}$ | $\stackrel{\text { O}}{\mathrm{j}}$ | $\stackrel{\circ}{\circ}$ | సे స్ | $\stackrel{\circ}{\mathrm{j}}$ | $\begin{aligned} & \stackrel{\circ}{\circ} \\ & \stackrel{7}{7} \end{aligned}$ | $\begin{aligned} & \stackrel{\circ}{\circ} \\ & \end{aligned}$ | $\stackrel{\text { O}}{\mathrm{j}}$ | $\begin{aligned} & \stackrel{\circ}{\circ} \\ & \stackrel{\rightharpoonup}{7} \end{aligned}$ | 육 | $\stackrel{\text { O}}{\mathrm{j}}$ | $\stackrel{\stackrel{\circ}{7}}{7}$ | $\stackrel{\stackrel{\circ}{\circ}}{7}$ | $\stackrel{\text { O}}{\mathrm{j}}$ | $\stackrel{\circ}{\circ}$ | ̈ㅡㄱ | $\stackrel{\text { Ṅ́ }}{ }$ | $\begin{aligned} & \stackrel{\circ}{7} \\ & \stackrel{7}{7} \end{aligned}$ | 윽 |
| 8 | － | 197 | 214 | － | 214 | 233 | － | 214 | 233 | － | 239 | 260 | － | 277 | 301 | － | 277 | 301 | － | 277 | 301 | － | 238 | 259 | － | 249 | 271 | － | 249 | 271 | － | 249 | 271 |
| 9 | － | 175 | 191 | － | 191 | 208 |  | 191 | 207 | － | 213 | 232 | － | 247 | 268 | － | 246 | 268 | － | 246 | 268 | － | 212 | 231 | － | 222 | 241 |  | 222 | 241 | － | 222 | 241 |
| 10 | － | 158 | 172 | － | 172 | 187 | － | 172 | 187 | － | 192 | 209 | － | 222 | 242 | － | 222 | 242 | － | 222 | 241 | － | 191 | 208 | － | 200 | 218 | － | 200 | 217 | － | 200 | 217 |
| 11 |  | 144 | 156 | － | 156 | 170 | － | 156 | 170 | － | 175 | 190 | － | 202 | 220 | － | 202 | 220 | － | 202 | 220 | － | 174 | 189 | － | 182 | 198 | － | 182 | 198 | － | 182 | 198 |
| 12 | － | 132 | 143 | － | 143 | 156 | － | 143 | 156 | － | 160 | 174 | － | 185 | 202 | － | 185 | 202 | － | 185 | 201 | － | 160 | 174 | － | 167 | 182 | － | 167 | 181 | － | 166 | 181 |
| 13 | － | 122 | 132 | － | 132 | 144 |  | 132 | 144 | － | 141 | 154 | － | 171 | 186 | － | 171 | 186 | － | 171 | 186 | － | 147 | 160 | － | 154 | 168 | － | 154 | 167 | － | 154 | 167 |
| 14 | － | 113 | 123 | － | 123 | 134 |  | 123 | 134 | － | 122 | 133 | － | 158 | 173 | － | 159 | 173 | － | 158 | 173 | － | 137 | 149 | － | 143 | 156 | － | 143 | 155 | － | 143 | 155 |
| 15 | 98 | 105 | 115 |  | 115 | 125 |  | 114 | 125 | － | 106 | 116 | － | 138 | 150 | － | 148 | 161 | － | 148 | 161 | 113 | 128 | 139 | － | 133 | 145 | － | 133 | 145 | － | 133 | 145 |
| 16 | 82 | 97 | 105 | － | 107 | 117 |  | 107 | 117 | － | 93 | 101 | － | 121 | 132 | － | 139 | 151 | － | 139 | 151 | 94 | 115 | 124 | － | 125 | 136 | － | 125 | 136 | － | 125 | 136 |
| 17 | 69 | 86 | 89 | － | 101 | 110 | － | 101 | 110 | 79 | 82 | 90 | － | 107 | 117 | － | 129 | 141 | － | 130 | 142 | 79 | 102 | 104 | － | 118 | 128 | － | 117 | 128 | － | 117 | 128 |
| 18 | 58 | 76 | 76 | － | 95 | 104 | － | 95 | 104 | 68 | 73 | 80 | － | 95 | 104 | － | 115 | 126 | － | 123 | 134 | 68 | 88 | 88 | － | 111 | 121 | － | 111 | 121 | － | 111 | 120 |
| 19 | 50 | 64 | 64 | 84 | 88 | 96 | － | 90 | 98 | 58 | 66 | 71 | － | 85 | 93 | － | 103 | 113 | － | 116 | 127 | 58 | 75 | 75 | 97 | 105 | 114 | － | 105 | 114 | － | 105 | 114 |
| 20 | 43 | 55 | 55 | 73 | 80 | 87 | － | 86 | 93 | 50 | 59 | 64 | － | 77 | 84 | － | 93 | 101 | － | 108 | 118 | 50 | 64 | 64 | 84 | 95 | 103 | － | 100 | 108 | － | 99 | 108 |
| 21 | － | － | － | 63 | 72 | 79 | － | 81 | 89 | 43 | 53 | 55 | － | 70 | 76 | － | 84 | 92 | － | 98 | 107 | 43 | 55 | 55 | 73 | 86 | 93 | － | 95 | 103 | － | 95 | 103 |
| 22 | － | － | － | 55 | 66 | 71 | － | 78 | 85 | － | － | － | － | 63 | 69 | － | 77 | 83 | － | 89 | 97 | － | － | － | 64 | 78 | 82 | － | 90 | 98 | － | 90 | 98 |
| 23 | － | － | － | 49 | 60 | 62 | 71 | 71 | 78 | － | － | － | 56 | 58 | 63 | － | 70 | 76 | － | 81 | 89 | － | － | － | 56 | 71 | 72 | 82 | 85 | 93 | － | 86 | 94 |
| 24 | － | － | － | 43 | 55 | 55 | 63 | 65 | 71 | － | － | － | 50 | 53 | 58 | － | 64 | 70 | － | 74 | 81 | － | － | － | 50 | 64 | 64 | 72 | 78 | 85 | － | 82 | 90 |
| 25 | － | － | － | － | － | － | 56 | 60 | 66 | － | － | － | － | － | － | － | 59 | 64 | － | 68 | 75 | － | － | － | 44 | 56 | 56 | 64 | 72 | 78 | － | 79 | 86 |
| 26 | － | － | － | － | － | － | 50 | 55 | 60 | － | － | － | － | － | － | － | 54 | 59 | － | 63 | 69 | － | － | － | 39 | 50 | 50 | 57 | 66 | 72 | － | 76 | 83 |
| 27 | － | － | － | － | － | － | 45 | 51 | 56 | － | － | － | － | － | － | － | 50 | 55 | － | 58 | 64 | － | － | － | － | － | － | 52 | 61 | 66 | 70 | 70 | 76 |
| 28 | － | － | － | － | － | － | － | － | － | － | － | － | － | － | － | － | － | － | － | 54 | 59 | － | － | － | － | － | － | 46 | 57 | 59 | 63 | 65 | 71 |
| 29 | － | － | － | － | － | － | － | － | － | － | － | － | － | － | － | － | － | － | － | 50 | 55 | － | － | － | － | － | － | 42 | 53 | 53 | 57 | 60 | 66 |


|  | RFPI 40 |  |  |  |  |  |  |  |  |  |  |  | RFPI 60S |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 9－1／2＂ |  |  | 11－7／8＂ |  |  | 14＂ |  |  | 16＂ |  |  | 9－1／2＂ |  |  | 11－7／8＂ |  |  | 14＂ |  |  | 16＂ |  |  |
|  | Live | Total |  | Live | Total |  | Live | Total |  | Live | Total |  | Live | Total |  | Live | Total |  | Live | Total |  | Live | Total |  |
|  | $\stackrel{\text { O}}{\mathrm{j}}$ | $\stackrel{\circ}{\stackrel{\circ}{7}}$ | 육 | $\stackrel{\text { O}}{\mathrm{j}}$ | $\stackrel{\stackrel{\circ}{7}}{7}$ | $\stackrel{\stackrel{\circ}{7}}{\substack{7}}$ | $\stackrel{\text { O}}{\text { ㅇ }}$ | $\begin{aligned} & \stackrel{\circ}{7} \\ & \underset{7}{7} \end{aligned}$ | $\stackrel{\stackrel{\circ}{\circ}}{\stackrel{\circ}{7}}$ | $\stackrel{\text { ㅇ́̇ }}{\substack{2}}$ | $\stackrel{\circ}{\stackrel{\circ}{7}}$ | 윽 | $\stackrel{\text { O}}{\mathrm{j}}$ | $\stackrel{\stackrel{\circ}{7}}{7}$ | $\stackrel{\stackrel{\circ}{\mathrm{N}}}{\substack{2}}$ | $\stackrel{\text { ণ}}{\mathrm{j}}$ | $\stackrel{\circ}{\circ}$ | ®욱 | $\stackrel{\text { O}}{\mathrm{j}}$ | $\stackrel{\circ}{\circ}$ | 苓 | $\stackrel{\text { O}}{\mathrm{j}}$ | $\stackrel{\circ}{\circ}$ | $\stackrel{\stackrel{\rightharpoonup}{\mathrm{N}}}{\mathrm{~N}}$ |
| 8 | － | 249 | 271 | － | 277 | 301 | － | 277 | 301 | － | 277 | 301 | － | 239 | 260 | － | 277 | 301 | － | 277 | 301 |  | 277 | 301 |
| 9 | － | 222 | 242 | － | 247 | 268 | － | 247 | 268 | － | 246 | 268 | － | 213 | 232 | － | 247 | 268 | － | 246 | 268 | － | 246 | 268 |
| 10 | － | 200 | 218 | － | 222 | 242 | － | 222 | 242 | － | 222 | 242 | － | 192 | 209 | － | 222 | 242 | － | 222 | 242 | － | 222 | 241 |
| 11 | － | 182 | 198 | － | 202 | 220 | － | 202 | 220 | － | 202 | 220 | － | 175 | 190 | － | 202 | 220 | － | 202 | 220 | － | 202 | 220 |
| 12 | － | 167 | 182 | － | 186 | 202 | － | 185 | 202 | － | 185 | 201 | － | 160 | 174 | － | 185 | 202 | － | 185 | 202 | － | 185 | 201 |
| 13 | － | 154 | 168 | － | 171 | 186 | － | 171 | 186 | － | 171 | 186 | － | 148 | 161 | － | 171 | 186 | － | 171 | 186 | － | 171 | 186 |
| 14 | － | 143 | 156 | － | 159 | 173 | － | 159 | 173 | － | 159 | 173 | － | 137 | 149 | － | 159 | 173 | － | 159 | 173 | － | 158 | 173 |
| 15 | 125 | 134 | 145 | － | 148 | 162 | － | 148 | 161 | － | 148 | 161 | － | 128 | 139 | － | 148 | 161 | － | 148 | 161 | － | 148 | 161 |
| 16 | 104 | 125 | 136 | － | 139 | 151 | － | 139 | 151 | － | 139 | 151 | 111 | 120 | 131 | － | 139 | 151 | － | 139 | 151 | － | 139 | 151 |
| 17 | 88 | 114 | 115 | － | 131 | 143 | － | 131 | 142 | － | 130 | 142 | 94 | 113 | 123 | － | 131 | 142 | － | 131 | 142 | － | 130 | 142 |
| 18 | 75 | 97 | 97 | － | 124 | 135 | － | 123 | 134 | － | 123 | 134 | 80 | 102 | 104 | － | 123 | 134 | － | 123 | 134 | － | 123 | 134 |
| 19 | 64 | 83 | 83 | 107 | 117 | 127 | － | 117 | 127 | － | 117 | 127 | 68 | 89 | 89 | 115 | 117 | 127 | － | 117 | 127 | － | 116 | 127 |
| 20 | 55 | 71 | 71 | 92 | 107 | 116 | － | 111 | 121 | － | 111 | 121 | 59 | 76 | 76 | 99 | 107 | 117 | － | 111 | 121 | － | 110 | 120 |
| 21 | 48 | 62 | 62 | 80 | 97 | 104 | － | 105 | 115 | － | 105 | 115 | 51 | 66 | 66 | 86 | 97 | 106 | － | 105 | 115 | － | 105 | 115 |
| 22 | 42 | 54 | 54 | 70 | 88 | 91 | － | 101 | 110 | － | 100 | 109 | 45 | 57 | 57 | 76 | 88 | 96 | － | 100 | 109 | － | 100 | 109 |
| 23 | － | － | － | 62 | 80 | 80 | 90 | 96 | 104 | － | 96 | 105 | 40 | 50 | 50 | 67 | 81 | 86 | － | 96 | 105 | － | 96 | 104 |
| 24 | － | － | － | 55 | 70 | 70 | 80 | 88 | 96 | － | 92 | 100 | － | － | － | 59 | 74 | 76 | 86 | 89 | 97 | － | 92 | 100 |
| 25 | － | － | － | 49 | 62 | 62 | 71 | 81 | 88 | － | 88 | 96 | － | － | － | 53 | 67 | 67 | 76 | 82 | 90 | － | 88 | 96 |
| 26 | － | － | － | 43 | 55 | 55 | 63 | 74 | 81 | － | 85 | 92 | － | － | － | 47 | 60 | 60 | 68 | 76 | 83 | － | 84 | 92 |
| 27 | － | － | － | － | － | － | 57 | 69 | 73 | 77 | 79 | 86 | － | － | － | 42 | 53 | 53 | 61 | 70 | 76 | － | 81 | 89 |
| 28 | － | － | － | － | － | － | 51 | 64 | 65 | 69 | 73 | 80 | － | － | － | － | － | － | 55 | 65 | 71 | 75 | 76 | 83 |
| 29 | － | － | － | － | － | － | 46 | 59 | 59 | 63 | 68 | 74 | － | － | － | － | － | － | 50 | 60 | 63 | 68 | 70 | 77 |
| 30 | － | － | － | － | － | － | 42 | 53 | 53 | 57 | 64 | 69 | － | － | － | － | － | － | 45 | 56 | 57 | 61 | 66 | 72 |

To Use PLF Table：
1．Select the span required（see General Note 3 below）．
2．Compare the design total load（plf）to the appropriate Total column and compare the design live load（plf）to the Live column．
3．Select a product that meets or exceeds both the design total and roof live loads．When no value is shown in the Live column，Total load will govern．

## General Notes：

1．Table values apply to uniformly loaded simple or multiple span joists．
2．Clear span is the clear distance between the face of supports．
3．Use the horizontal span dimension from the building plans to size joists for roofs that slope up to 2 ＂in 12 ＂．For roof slopes greater than 2 ＂in 12 ＂，multiply the horizontal span dimension by the appropriate Slope Factor from the table on page 29.
4．Roofs must be sloped at least $1 / 4$＂in 12 ＂to assure drainage．
5．Live load column is based on an $\mathrm{L} / 240$ deflection limit．
6．Total load column is based on an L／180 deflection limit．Use 115\％column for snow loads and 125\％for non－snow roof live loads．Check with local code（based on location of building）for snow load requirements．
7．Verify that the deflection criteria conform to local building code requirements
8．Minimum end bearing length is $1-3 / 4$＂．Minimum intermediate bearing length is $3-1 / 2^{\prime \prime}$ ．
9．Web stiffeners are not required for loads shown．Except for details $2 \mathrm{a}, 2 \mathrm{~d}, 2 \mathrm{f}, 2 \mathrm{~g}, 2 \mathrm{~h}$ and 2 k ．
10．This table does not account for added stiffness from glued or nailed sheathing．

RFPI 90

|  | RFPI 70 |  |  |  |  |  |  |  |  |  |  |  | RFPI 80S |  |  |  |  |  |  |  |  | RFPI 90 |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1／2＂ |  |  | 11－7／8＂ |  |  | 14＂ |  |  | 16＂ |  |  | 11－7／8＂ |  |  | 14＂ |  |  | 16＂ |  |  | 9－1／2＂ |  |  | 11－7／8＂ |  |  | 14＂ |  |  | 16＂ |  |  |
|  | Live | Total |  | Live Total |  |  | Live Total |  |  | Live | Total |  | Live Total |  |  | Live Total |  |  | Live Total |  |  | Live Total |  |  | Live Total |  |  | Live | Total |  | $\begin{gathered} \text { Live } \\ \stackrel{\circ}{\mathrm{I}} \\ \hline \end{gathered}$ | Total |  |
|  | $\stackrel{\circ}{\mathrm{I}}$ | $\begin{aligned} & \stackrel{\circ}{\circ} \\ & \underset{7}{2} \end{aligned}$ | $\begin{aligned} & \stackrel{\text { ®̈̈ㅜㅇ }}{1} \end{aligned}$ | $\stackrel{i}{~}$ | $\begin{aligned} & \stackrel{\circ}{\circ} \\ & \stackrel{\rightharpoonup}{2} \end{aligned}$ | 俞 | $\stackrel{\circ}{5}$ | $\begin{aligned} & \stackrel{\circ}{\circ} \\ & \stackrel{\rightharpoonup}{7} \end{aligned}$ | $\begin{aligned} & \stackrel{\circ}{\mathrm{N}} \\ & \underset{\sim}{0} \end{aligned}$ | $\stackrel{\text { O}}{5}$ | $\begin{aligned} & \stackrel{\circ}{\circ} \\ & \underset{7}{2} \end{aligned}$ | $\begin{aligned} & \stackrel{\circ}{\circ} \\ & \end{aligned}$ | $\stackrel{i}{5}$ | $\stackrel{\circ}{\stackrel{\circ}{7}}$ | 茴 | $\stackrel{\text { O}}{5}$ | $\begin{aligned} & \stackrel{\circ}{\circ} \\ & \underset{ت}{7} \end{aligned}$ | $\begin{aligned} & \stackrel{\circ}{\circ} \\ & \text { N- } \end{aligned}$ | $\stackrel{\text { O}}{5}$ | $\begin{aligned} & \stackrel{\circ}{0} \\ & \underset{-1}{2} \end{aligned}$ | $\begin{aligned} & \stackrel{\circ}{\mathrm{N}} \\ & \underset{\sim}{0} \end{aligned}$ | $\stackrel{\text { O}}{5}$ | $\begin{aligned} & \stackrel{\circ}{\circ} \\ & \underset{-1}{2} \end{aligned}$ | $\begin{aligned} & \stackrel{\circ}{\mathrm{N}} \\ & \hline \end{aligned}$ | $\stackrel{i}{5}$ | $\begin{aligned} & \stackrel{\circ}{\circ} \\ & \underset{7}{2} \end{aligned}$ | $\begin{aligned} & \stackrel{\circ}{\stackrel{\circ}{*}} \end{aligned}$ | $\stackrel{\circ}{\mathrm{J}}$ | $\begin{aligned} & \stackrel{\circ}{\circ} \\ & \stackrel{\rightharpoonup}{7} \end{aligned}$ | 茴 |  | $\begin{aligned} & \stackrel{\circ}{0} \\ & \underset{-1}{2} \end{aligned}$ | 茴 |
| 8 |  | 259 | 28 |  | 277 | 30 |  | 277 | 30 |  | 276 | 301 |  | 311 | 338 |  | 334 | 363 |  | 334 | 363 |  | 334 | 364 |  | 372 | 404 |  | 371 | 404 |  | 371 | 404 |
| 9 |  | 230 | 251 |  | 247 | 268 |  | 246 | 268 |  | 246 | 268 |  | 277 | 301 |  | 297 | 324 |  | 297 | 323 |  | 298 | 324 |  | 331 | 360 |  | 331 | 360 |  | 330 | 360 |
| 10 |  | 208 | 226 |  | 222 | 242 |  | 222 | 242 |  | 222 | 241 |  | 249 | 271 |  | 268 | 292 |  | 268 | 291 |  | 268 | 292 |  | 298 | 324 |  | 298 | 324 |  | 29 | 324 |
| 11 |  | 189 | 206 |  | 202 | 220 |  | 202 | 220 |  | 202 | 219 |  | 227 | 247 |  | 244 | 265 |  | 243 | 265 |  | 244 | 265 |  | 271 | 295 |  | 271 | 295 |  | 271 | 295 |
| 12 |  | 173 | 189 |  | 185 | 202 |  | 185 | 201 |  | 185 | 201 |  | 208 | 226 |  | 223 | 243 |  | 223 | 243 |  | 224 | 243 |  | 249 | 271 |  | 248 | 270 |  | 248 | 270 |
| 13 |  | 16 | 174 |  | 171 | 186 |  | 171 | 186 |  | 171 | 186 |  | 192 | 209 |  | 206 | 224 |  | 206 | 224 |  | 20 | 225 |  | 23 | 250 |  | 229 | 250 |  | 229 | 249 |
| 14 |  | 149 | 162 |  | 159 | 173 |  | 159 | 173 |  | 158 | 172 |  | 178 | 194 |  | 191 | 208 |  | 191 | 208 |  | 192 | 209 |  | 213 | 232 |  | 213 | 23 |  | 213 | 232 |
| 15 |  | 139 | 151 |  | 148 | 161 |  | 148 | 161 |  | 148 | 161 |  | 166 | 181 |  | 179 | 194 |  | 178 | 194 |  | 179 | 195 |  | 199 | 217 |  | 199 | 216 |  | 198 | 216 |
| 16 | 127 | 130 | 141 |  | 139 | 151 |  | 139 | 151 |  | 13 | 151 |  | 56 | 169 |  | 167 | 182 |  | 167 | 182 | － | 16 | 183 |  | 186 | 203 |  | 186 | 203 |  | 18 | 202 |
| 17 | 107 | 122 | 133 |  | 131 | 142 |  | 130 | 142 |  | 130 | 142 |  | 146 | 159 |  | 157 | 171 |  | 157 | 171 | 153 | 158 | 172 |  | 175 | 191 |  | 175 | 191 |  | 175 | 190 |
| 18 | 91 | 115 | 119 |  | 123 | 134 |  | 123 | 134 |  | 123 | 134 |  | 138 | 150 |  | 148 | 162 |  | 148 | 162 | 131 | 149 | 162 |  | 166 | 180 |  | 165 | 180 |  | 165 | 180 |
| 19 | 78 | 102 | 102 |  | 117 | 127 |  | 117 | 127 |  | 116 | 127 |  | 131 | 142 |  | 141 | 153 |  | 140 | 153 | 113 | 141 | 147 |  | 157 | 171 |  | 156 | 17 |  | 156 | 170 |
| 20 | 68 | 87 | 87 |  | 111 | 121 |  | 111 | 121 |  | 110 | 120 |  | 124 | 135 |  | 133 | 145 |  | 133 | 145 | 98 | 127 | 127 |  | 149 | 162 |  | 148 | 162 |  | 148 | 162 |
| 21 | 59 | 76 | 76 | 98 | 105 | 115 | － 1 | 105 | 115 |  | 105 | 114 | 116 | 118 | 129 | － | 127 | 138 | － | 127 | 138 | 85 | 110 | 110 | 141 | 142 | 154 |  | 14 | 15 |  | 14 | 154 |
| 22 | 51 | 66 | 66 | 86 | 101 | 110 |  | 100 | 109 |  | 100 | 109 | 102 | 113 | 123 |  | 121 | 132 |  | 121 | 132 | 75 | 96 | 96 | 124 | 135 | 147 |  | 135 | 14 |  | 13 | 147 |
| 23 | 45 | 58 | 58 | 76 | 96 | 98 | － | 96 | 105 |  | 96 | 104 | 90 | 108 | 116 |  | 116 | 126 |  | 115 | 126 | 66 | 84 | 84 | 109 | 129 | 141 |  | 129 | 140 |  | 129 | 14 |
| 24 | 40 | 51 | 51 | 67 | 87 | 87 |  | 92 | 100 |  | 92 | 100 | 80 | 103 | 103 |  | 111 | 121 |  | 110 | 120 | 59 | 74 | 74 | 97 | 124 | 125 |  | 123 | 134 |  | 123 | 134 |
| 25 |  |  |  | 60 | 77 | 77 | 87 | 88 | 96 |  | 88 | 96 | 71 | 91 | 91 | 103 | 10 | 116 |  | 106 | 115 | 52 | 66 | 66 | 86 | 111 | 111 |  | 118 | 12 |  | 118 | 129 |
| 26 |  |  |  | 53 | 68 | 68 | 78 | 85 | 92 |  | 84 | 92 | 64 | 81 | 81 | 92 | 102 | 111 |  | 102 | 111 | 47 | 58 | 58 | 77 | 99 | 99 | 111 | 114 | 12 |  | 113 | 12 |
| 27 |  |  |  | 48 | 61 | 61 | 70 | 81 | 89 |  | 81 | 88 | 57 | 72 | 72 | 83 | 98 | 106 |  | 98 | 107 | 42 | 52 | 52 | 70 | 89 | 89 | 100 | 109 | 119 |  | 109 | 119 |
| 28 |  |  |  | 43 | 55 | 55 | 63 | 78 | 81 |  | 78 | 85 | 51 | 65 | 65 | 74 | 93 | 95 |  | 94 | 103 |  |  |  | 63 | 80 | 80 | 91 | 105 | 115 |  | 105 | 11 |
| 29 |  |  |  |  |  |  | 57 | 73 | 73 |  | 75 | 82 | 47 | 58 | 58 | 67 | 86 | 86 | 91 | 91 | 99 |  |  |  | 57 | 72 | 72 | 82 | 101 | 105 |  | 101 | 11 |
| 30 |  |  |  |  |  |  | 52 | 66 | 66 | 70 | 73 | 79 | 42 | 52 | 52 | 61 | 77 | 77 | 82 | 88 | 96 |  |  |  | 52 | 65 | 65 | 75 | 95 | 95 | － | 98 | 10 |

11．Use appropriate software （e．g．Simpson Strong－Tie ${ }^{\text {® }}$ Component Solutions ${ }^{\text {TiM }}$ ） or engineering analysis to analyze multiple span joists if the length of any span is less than half the length of an adjacent span． 12．Use appropriate software or engineering analysis to analyze conditions outside of the scope of this table such as cantilevers and concentrated loads．
13．Provide lateral support at bearing points and continuous lateral support along the compression flange of each joist．
14．For double joists，double the table values and connect the joists per the detail on page 22. 15．For proper installation procedures，refer to the appropriate sections in this publication．

## Allowable Roof Clear Spans Snow Load 115\% Load Duration

Please refer to notes on page 34.


|  |  |  | Slope of 4/12 or less |  |  | Slopes over 4/12 up to 8/12 |  |  | Slope over 8/12 up to 12/12 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Joist Depth | Joist Series | 16" o.c. | 19.2" o.c. | 24" o.c. | 16" o.c. | 19.2" o.c. | 24" o.c. | 16" o.c. | 19.2" o.c. | 24" o.c. |
|  |  | RFPI 20 | 19'-3" | 18'-1" | 16' - 7" | 18'-2" | 17' - 1" | 15' - 9" | 16'-10" | 15'-10" | 14' - 8" |
|  |  | RFPI 40S | 20'-1" | 18'-4" | 16'-4" | 19'-1" | 17'-11" | 16' - 0" | 17' - 9" | 16' -8" | $15^{\prime}-5^{\prime \prime}$ |
|  |  | RFPI 400 | 20' - 3" | 19'-0" | 17'-7" | 19'-1" | 17'-11" | 16' - 7" | 17' - 9" | 16' - 8" | 15' - 5" |
|  | 9-1/2" | RFPI 40 | 21' - 0" | 19'-9" | 18'-3" | 19'-10" | 18'-7" | 17' - 3' | 18' - 5" | 17'-3" | 16' - 0" |
|  |  | RFPI 60S | 21' - 6" | 20'-2" | 18'-8" | 20' - 4" | 19'-1" | $17^{\prime}-8{ }^{\prime \prime}$ | 18'-10" | 17' - 8" | $16^{\prime}-5^{\prime \prime}$ |
|  |  | RFPI 70 | 22' - 6" | 21'-2" | 19'-7" | 21'-3" | 20' - 0" | 18' - 6" | 19' - 9" | 18'-7" | 17' - 2" |
|  |  | RFPI 90 | 25'-9" | 24'-2' | 22'-4" | 24' - 4" | 22'-10" | 21'-1" | 22'-7" | 21'-2" | 19'-7" |
|  |  | RFPI 20 | 23'-1" | 21'-2" | 18'-11" | 21'-9" | 20' - 5' | 18' - 4' | 20' - 3" | 19' - 0" | 17'-2" |
| - |  | RFPI 40S | 22'-11" | 20'-11" | 18' - 8" | 22'-4" | 20' - 5" | 18'-2" | 21' - 3" | 19'-9" | 17 - -8" |
| 4 |  | RFPI 400 | 24' - 3" | 22'-9" | 20'-7" | 22'-11" | 21' - 6" | 19'-11" | 21' - ${ }^{\prime \prime}$ | 20'-0" | 18' - 6" |
| IT | 11-7/8" | RFPI 40 | 25'-1" | 23'-7" | 21'-10" | 23' - 8" | 22'-3" | 20'-7" | 22' - 0" | 20'-8" | 19'-2" |
| $\bigcirc$ | 11-7/8 | RFPI 60S | 25'-9" | 24'-2" | 22' - 0" | 24' - 4' | 22'-10" | 21'-2" | 22'-7" | 21'-3" | 19'-8" |
| 6 |  | RFPI 70 | 27' - 0" | 25'-4" | 23' - 5" | 25' - 6" | 23'-11" | 22'-2" | 23' - 8" | 22'-3" | 20'-7" |
|  |  | RFPI 80S | 28' - 8' | 26'-11" | 24'-11" | 27'-1" | 25' - 5" | 23'-6" | 25'-2" | 23'-7" | 21'-10" |
|  |  | RFPI 90 | 30' - 9' | 28'-10" | 26' - 8" | 29'-0" | 27' - ${ }^{\prime \prime}$ | 25'-3" | 27' - 0 " | 25'-4" | 23' - ${ }^{\prime \prime}$ |
|  |  | RFPI 20 | 25' - 4" | 23'-1" | 19'-3" | 24' - 9" | 22'-7" | 18'-4" | 23'-1" | 21'-6" | 17'-2" |
| II |  | RFPI 40S | 25'-2" | 22'-11" | 20'-6" | 24'-7" | 22'-5" | 20' - 0" | 23'-9" | 21'-8" | 19'-4" |
| $\geq$ |  | RFPI 400 | 27'-7" | 25'-2" | 22'-4" | 26'-1" | 24' - 6" | 21'-4" | 24' - ${ }^{\prime \prime}$ | 22'-9" | 20' - 0" |
|  |  | RFPI 40 | 28' - 8" | 26'-9" | 23'-11" | 27' - 0" | 25' - 5" | 23'-4" | 25'-1" | 23'-7" | 21'-10" |
|  | 14 | RFPI 60S | 29'-5" | 27'-0" | 24' - 1' | 27' - 9" | 26' - 1" | 23'-7" | 25' - 9' | 24'-2" | 22'-3" |
|  |  | RFPI 70 | 30' - 9' | 28'-11" | 24'-11" | 29'-1" | 27' - 3" | 23'-9" | 27' - 0" | 25'-4" | 22'-3" |
|  |  | RFPI 80S | 32'-7" | 30'-7" | 28'-4" | 30'-10" | 28'-11" | 26'-9" | 28'-7" | 26'-10" | 24'-11" |
|  |  | RFPI 90 | 35' - 0" | 32'-10" | 30' - 5" | 33' - 0" | 31' - 0" | $28^{\prime}-8{ }^{\prime \prime}$ | 30' - 8' | 28' - 10" | $26^{\prime}-8^{\prime \prime}$ |
|  |  | RFPI 40S | 27' - 1' | 24'-8" | 22' - 1" | 26' - 5" | 24' - 2" | 21'-7" | 25' - 7" | 23'-4" | 20'-10" |
|  |  | RFPI 400 | 29'-6" | 26'-11" | 22'-4" | 28'-10" | 26'-4" | 21'-4" | 26'-11" | 25'-1" | 20' - 0" |
|  |  | RFPI 40 | 31'-4" | 28'-7" | 24'-11" | 30' - 0" | 27'-11" | 23'-9" | 27'-10" | 26'-2" | 22'-3" |
|  | $16 "$ | RFPI 60S | 31'-10" | 29'-1" | 24'-11" | 30'-10' | 28' - 5' | 23'-9" | 28'-7" | 26'-11" | 22'-3" |
|  |  | RFPI 70 | 34' - 2' | 31'-2" | 24'-11" | 32' - ${ }^{\prime \prime}$ | 29'-9" | 23'-9" | 29'-11" | 27'-10" | 22'-3" |
|  |  | RFPI 80S | 36' - 2' | 34' - 0" | 30' - 1' | 34' - 2' | 32' - 1" | 28'-8" | 31' - 9" | 29'-10' | 26'-11" |
|  |  | RFPI 90 | 38'-10" | 36'-5" | 33' - 6" | $36^{\prime}-8{ }^{\prime \prime}$ | 34' - ${ }^{\prime \prime}$ | 31'-10" | 34' - 0" | 32'-0" | 29'-7" |

## Allowable Roof Clear Spans Snow Load 115\% Load Duration

Please refer to notes on page 34.

|  |  |  | Slope of 4/12 or less |  |  | Slopes over 4/12 up to 8/12 |  |  | Slope over 8/12 up to 12/12 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Joist Depth | Joist Series | 16" o.c. | 19.2" o.c. | 24" o.c. | 16" o.c. | 19.2" o.c. | 24" o.c. | 16" o.c. | 19.2" o.c. | 24" o.c. |
|  |  | RFPI 20 | 18' - 0" | 16'-10" | 14' -5" | 17' - 0" | 16' - 0" | 13' - 10" | 15' - 10" | 14'-11" | 13'-1" |
|  |  | RFPI 40S | 18'-2" | 16'-7" | 14'-10" | 17'-10" | 16'-3" | 14' - 6" | 16' -8" | 15'-8" | 14'-1" |
|  |  | RFPI 400 | 18'-11" | 17'-9" | 16' - 5' | 17'-11" | 16' - 10" | 15' - 7" | 16' - 8' | 15' - 8" | 14' - 6" |
|  | 9-1/2" | RFPI 40 | 19'-7" | 18'-5" | 17' - 0" | 18' - 7" | $17^{\prime}-5 "$ | 16' - 1" | 17'-4" | 16'-3" | 15' - 0" |
|  |  | RFPI 60S | 20'-1" | 18'-10" | 17' - 5' | 19' - 0" | 17' - 10" | 16' - 6" | 17'-9" | 16'-8" | 15'-5" |
|  |  | RFPI 70 | 21' - 0" | 19'-9" | 18'-3" | 19'-11" | 18' - 8" | 17' - 4" | 18'-7" | $17^{\prime}-5^{\prime \prime}$ | 16' - 2" |
|  |  | RFPI 90 | 24' - 0" | 22'-6" | 20' - 10" | 22'-9" | 21'-4" | 19'-9" | 21'-3" | 19'-11" | 18'-5" |
|  |  | RFPI 20 | 21'-0" | 19'-2" | 15'-9" | 20' - 5" | 18' - 9" | 15'-1" | 19'-0" | 17'-10" | 14'-4" |
| - |  | RFPI 40S | 20' - 9" | 18'-11" | 16'-10" | 20' - 4' | 18' - 6" | 16' - 7" | 19'-9" | 18' - 0" | 16'-1" |
| 4 |  | RFPI 400 | 22' - 8" | 20'-10" | 18'-4" | 21'-6" | 20'-2" | 17' - 7" | 20'-0" | 18'-9" | 16'-8" |
| 1 | 11-7/8" | RFPI 40 | 23' - 6" | 22'-0" | 19'-9" | 22'-3" | 20' - 10" | 19'-4" | 20' - 8" | 19'-5" | 18'-0" |
| $\bigcirc$ | 11-7/8 | RFPI 60S | 24'-1" | 22'-3" | 19'-11" | 22'-10" | 21'-5" | 19'-6" | 21'-3" | 20'-0" | 18'-6" |
|  |  | RFPI 70 | 25'-2" | 23'-8" | 20' - 5" | 23'-10" | 22'-5" | 19'-7" | 22' - 3" | 20'-11" | 18'-7" |
|  |  | RFPI 80S | 26' - 9" | 25'-2" | 22'-11" | 25' - 4" | 23'-10" | 22' - 0" | 23' - 8" | 22'-3" | 20'-7" |
|  |  | RFPI 90 | 28' - 8' | 26'-11" | 24'-11" | 27'-2" | 25' - 6" | 23'-7" | 25' - 4" | 23'-10" | 22'-0" |
|  |  | RFPI 20 | 22'-11" | 19'-9" | 15'-9" | 22' - 6" | 18'-11" | 15' - 1" | 21'-7" | 17'-11" | 14'-4" |
| II |  | RFPI 40S | 22'-9" | 20'-9" | 18' - 6" | 22'-4" | 20' - 4" | 18'-2" | 21'-9" | 19'-10" | 17'-8" |
| $\geq$ |  | RFPI 400 | 25' - 0" | 22'-10" | 18'-4" | 24' - 5' | 22' - 1" | 17' - 7" | 22' - 9" | 20'-11" | 16'-8" |
|  | 14" | RFPI 40 | 26' - 6" | 24'-2" | 20' - 5" | 25'-4" | 23' - 9" | 19'-7" | 23' - 7" | 22'-2" | 18'-7" |
|  |  | RFPI 60S | 26' - 9' | 24' - 5" | 20' - 5" | 26' - 0" | 23'-11" | 19' - 7' | 24' - 3" | 22'-9" | 18'-7" |
| + |  | RFPI 70 | 28' - 9' | 25'-6" | 20' - 5" | 27'-3' | 24' - 7" | 19'-7" | 25' - 5" | 23'-3" | 18'-7" |
| V |  | RFPI 80S | 30' - 6" | 28'-7" | 24' - 8" | 28'-10" | 27' - 1" | 23' - 9" | 26'-11" | 25'-3" | 22'-6" |
|  |  | RFPI 90 | 32' - 8" | 30'-8" | 27' - 5" | 30'-11" | 29'-1" | 26' - 5" | 28'-10" | 27'-1" | 25' - ${ }^{\prime \prime}$ |
|  |  | RFPI 40S | 24' - 6" | 22'-4" | 20' - 0" | 24'-1" | 21'-11" | 19'-7" | 23'-5" | 21'-4" | 18'-7" |
|  |  | RFPI 400 | 26' - 9" | 23'-0" | 18'-4" | 26' - 3' | 22'-1" | 17'-7" | 25'-2" | 20'-11" | 16' - 8' |
|  |  | RFPI 40 | 28' - 5' | 25'-6" | 20' - 5" | 27'-10" | 24' - 7" | 19'-7" | 26'-2" | 23'-3" | 18'-7" |
|  | 16" | RFPI 60S | 28'-10" | 25'-6" | 20' - 5" | 28'-3" | 24'-7" | 19'-7" | 26'-11" | 23'-3" | 18'-7" |
|  |  | RFPI 70 | 30' - 8" | 25'-6" | 20' - ${ }^{\prime \prime}$ | 29'-6" | 24'-7" | 19'-7" | 28' - 0" | 23' - 3" | 18'-7" |
|  |  | RFPI 80S | 33'-10" | 30'-11" | 24' - 8" | 32'-0" | 29' - 8" | 23' - 9" | 29'-10" | 28' - 0" | 22'-6" |
|  |  | RFPI 90 | 36' - 3' | 34' - 0" | $27^{\prime}-5^{\prime \prime}$ | $34^{\prime}-4 \prime$ | 32' - 3" | $26^{\prime}-5^{\prime \prime}$ | 32' - 0" | 30' - 1" | 25' - 0' |



## Allowable Roof Clear Spans Non-Snow Load 125\% Load Duration

|  | Joist Depth | Joist Series | Slope of 4/12 or less |  |  | Slopes over 4/12 up to 8/12 |  |  | Slope over 8/12 up to 12/12 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | 16" o.c. | 19.2" o.c. | 24" o.c. | 16" o.c. | 19.2" o.c. | 24" o.c. | 16" o.c. | 19.2" o.c. | 24" o.c. |
|  | 9-1/2" | RFPI 20 | 22'-1" | 20' - 9" | 19'-3" | 20'-10" | 19'-7" | 18'-2" | 19'-4" | 18'-2" | 16' - 10" |
|  |  | RFPI 40S | 23'-4" | 21'-11" | 20' - 3" | 22' - 0 " | 20'-8" | 19'-1" | 20'-5" | 19'-2" | 17' - 9" |
|  |  | RFPI 400 | 23'-4" | 21'-11" | 20' - 3" | 22' - 0" | 20' - 8" | 19'-1" | 20'-5" | 19'-2" | 17' - 9" |
|  |  | RFPI 40 | 24'-2" | 22' - 8" | 21' - ${ }^{\prime \prime}$ | 22'-9" | 21'-5" | 19'-10" | 21'-2" | 19'-10" | 18' - 5" |
|  |  | RFPI 60S | 24'-9" | 23' - 3" | 21'-6" | 23'-4" | 21'-11" | 20' - 4" | 21'-8" | 20'-4" | 18'-10" |
|  |  | RFPI 70 | 25'-11" | 24' - 4" | 22' - 6" | 24' - 5" | 23'-0" | 21'-3" | 22'-8" | 21'-4" | 19'-9" |
|  |  | RFPI 90 | 29'-7" | 27' - 10' | 25' - 9" | 27'-11" | 26'-3" | 24'-4" | 25'-11" | 24'-4" | 22'-7" |
|  | 11-7/8" | RFPI 20 | 26'-6" | 24'-11" | 23'-1" | 25' - 0" | 23'-6" | 21'-9" | 23'-3" | 21'-10" | 20' - 3" |
| 0 |  | RFPI 40S | 27'-11' | 26' - 3" | 23'-10" | 26' - 4" | 24'-9" | 22'-11" | 24' - 5" | 22'-11" | 21' - 3" |
| 4 |  | RFPI 400 | 27'-11" | 26' - 3' | 24' - 3" | 26'-4" | 24'-9" | 22'-11" | 24'-5" | 22'-11" | 21'-3" |
| IT |  | RFPI 40 | 28'-10" | 27'-1" | 25'-1" | 27' - 3" | 25'-7" | 23' - 8" | 25'-3" | 23'-9" | 22' - 0" |
| - |  | RFPI 60S | 29'-8" | 27'-10" | 25' - 9" | 28' - 0" | 26'-3" | 24'-4" | 25'-11" | 24'-5" | 22'-7" |
| $0$ |  | RFPI 70 | 31'-0" | 29'-2" | 27' - 0" | 29' - 3" | 27'-6" | 25' - 6" | 27' - 2" | 25'-6" | 23' - 8" |
|  |  | RFPI 80S | 33'-0" | 31' - 0" | 28' - 8" | 31'-1" | 29'-3" | 27'-1" | 28'-11" | 27'-2" | 25'-2" |
|  |  | RFPI 90 | 35'-4" | 33' - 3" | 30' - 9" | 33' - ${ }^{\prime \prime}$ | 31'-4" | 29'-0" | 31'-0" | 29'-1" | 27' - 0" |
| $\stackrel{\odot}{N}$ | 14" | RFPI 20 | 30'-3' | 28'-5" | 26'-4" | 28'-7" | 26'-10" | 24'-10" | 26' - 6" | 24'-11" | 23'-1" |
|  |  | RFPI 40S | 31'-8" | 29'-4" | 26' - 3" | 29'-11" | 28'-1" | 25' - 7" | 27' - 9" | 26'-1" | 24'-2" |
|  |  | RFPI 400 | 31'-9" | 29'-10" | 27' - 8" | 30' - 0" | 28'-2" | 26'-1" | 27' - 10" | 26'-2" | 24'-3" |
|  |  | RFPI 40 | 32'-11" | 30'-11" | 28' - 8" | 31' - 0" | 29'-2" | 27' - 0" | 28' - 10" | 27'-1" | 25'-1" |
|  |  | RFPI 60S | 33'-9" | 31'-9" | 29'-5" | 31'-10" | 29'-11" | 27' - 9" | 29'-7" | 27'-9" | 25'-9" |
|  |  | RFPI 70 | 35'-4" | 33'-3" | 30' - 9" | 33' - 4' | 31'-4" | 29'-1" | 31' - 0" | 29'-1" | 27' - 0" |
|  |  | RFPI 80S | 37'-6" | 35' - 3" | 32'-7" | 35'-5" | 33'-3" | 30'-10" | 32'-10" | 30'-10" | 28'-7" |
|  |  | RFPI 90 | 40'-3" | 37' - 9" | 35' - 0" | 38' - 0" | 35'-8' | 33' - 0" | 35' - 3" | 33'-2" | 30' - 8" |
|  | $16 "$ | RFPI 40S | 34'-8" | 31'-7" | 28'-3" | 33'-2" | 30'-10" | 27'-7" | 30' - 9" | 28'-11" | 26' - 8" |
|  |  | RFPI 400 | 35'-4" | 33'-2" | 30' - 9" | 33' - 4' | 31'-4" | 29'-0" | 30'-11" | 29'-1" | 26'-11" |
|  |  | RFPI 40 | 36'-6" | 34'-4" | 31'-9" | $34^{\prime}-5^{\prime \prime}$ | 32'-5" | 30' - 0" | 32' - 0" | 30'-1" | 27' - 10" |
|  |  | RFPI 60S | 37'-6" | 35' - 3' | 32' - 8" | 35'-5" | 33'-3' | 30'-10" | 32'-10" | 30'-10" | 28'-7" |
|  |  | RFPI 70 | 39'-3' | 36'-11" | 34'-2" | 37' - 1' | 34'-10" | 32'-3" | 34' - 5" | 32'-4" | 29'-11" |
|  |  | RFPI 80S | 41'-7" | 39'-1" | 36'-2" | 39'-3" | 36'-11" | 34'-2" | 36' - 5" | 34'-3" | 31'-9" |
|  |  | RFPI 90 | 44'-7" | 41'-11" | 38'-10" | 42'-1" | 39'-7" | 36' - 8' | 39' - 1" | 36'-9" | 34' - 0" |



## Allowable Roof Clear Spans Non-Snow Load 125\% Load Duration

|  |  |  | Slope of 4/12 or less |  |  | Slopes over 4/12 up to 8/12 |  |  | Slope over 8/12 up to 12/12 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Joist Depth | Joist Series | 16" o.c. | 19.2" o.c. | 24" o.c. | 16" o.c. | 19.2" o.c. | 24" o.c. | 16" o.c. | 19.2" o.c. | 24" o.c. |
| $\bigcirc$ | 9-1/2" | RFPI 20 | 20' - 0" | 18'-9" | 17' - 4" | 18'-8" | 17' - 7" | 16'-3' | 17'-2" | 16' - 2" | 14'-11" |
|  |  | RFPI 40S | 21' - 0" | 19'-9" | 18' - 0" | 19'-8" | 18'-6" | 17'-2" | 18'-1" | 17' - 0" | 15' - 9" |
|  |  | RFPI 400 | 21' - 0" | 19'-9" | 18' - 3" | 19'-8" | 18' - 6" | 17' - 2" | 18'-1" | 17' - 0" | 15' - 9" |
|  |  | RFPI 40 | 21'-10" | 20'-6" | 18'-11" | 20'-5" | 19'-2" | 17'-9" | 18'-9" | $17^{\prime}-8{ }^{\prime \prime}$ | 16' - 4" |
|  |  | RFPI 60S | 22'-4" | 21'-0" | 19'-5" | 20'-11" | 19'-8" | 18' - 2" | 19'-3" | 18' - 1" | 16' - 9" |
|  |  | RFPI 70 | 23' - 5" | 22'-0" | 20' - 4' | 21'-11" | 20'-7" | 19'-1" | 20'-2" | 18'-11" | 17' - 6" |
|  |  | RFPI 90 | 26' - 9" | 25'-1" | 23'-2" | 25'-0" | 23' - 6" | 21'-9" | 23'-0" | 21'-8" | 20' - 0" |
|  | 11-7/8" | RFPI 20 | 23'-11" | 22'-6" | 20' - 10" | 22'-5" | 21'-1" | 19' - 6" | 20'-7" | 19'-4" | 17'-11" |
|  |  | RFPI 40S | 25'-2" | 23'-0" | 20' - 7" | 23'-7" | 22'-2" | 19'-10" | 21'-8" | 20' - 5" | 18'-11" |
| - |  | RFPI 400 | 25'-2" | 23'-8" | 21'-11" | 23'-7" | 22'-2" | 20' - 6" | 21'-8" | 20' - ${ }^{\prime \prime}$ | 18'-11" |
|  |  | RFPI 40 | 26'-1" | 24'-6" | 22' - 8" | 24'-5" | 22'-11" | 21' - 3" | 22'-6" | 21'-1" | 19'-6" |
|  |  | RFPI 60S | 26' - 9" | 25'-2" | 23' - 3' | 25'-1" | 23'-7" | 21'-10" | 23'-1" | 21'-8" | 20' - 1" |
| $\underset{N}{\mathbf{N}}$ |  | RFPI 70 | 28' - 0" | 26'-4" | 24' - 4' | 26'-3" | 24' - 8" | 22'-10" | 24'-2" | 22' - 8" | 21'-0" |
|  |  | RFPI 80S | 29'-9" | 27'-11" | 25'-10" | 27'-11" | 26'-2" | 24' - 3" | 25'-8" | 24' - 1" | 22'-4" |
|  |  | RFPI 90 | 31'-11" | 30'-0" | 27' - 9" | 29'-11" | 28'-1' | 26'-0" | 27'-6" | 25'-10" | 23'-11" |
| $\stackrel{O}{N}$ | 14" | RFPI 20 | 27'-4" | 25'-5" | 22'-9" | 25'-7" | 24'-1" | 21'-9" | 23'-7" | 22'-2" | 19'-10" |
|  |  | RFPI 40S | 27' - 8' | 25'-3" | 22'-7" | 26' - 9" | 24'-5" | 21'-10" | 24'-8" | 23'-2" | 20'-10" |
|  |  | RFPI 400 | 28' - 8" | 27'-0" | 24'-10" | 26'-11" | 25'-3" | 23'-5" | 24'-9" | 23'-3" | 21'-6" |
|  |  | RFPI 40 | 29'-9" | 27'-11" | 25' - 10" | 27' - 10" | 26'-2" | 24'-3" | 25'-7" | 24' - 1" | 22'-3" |
|  |  | RFPI 60S | 30' - 6" | 28'-8" | 26' - 6" | 28'-7" | 26'-10" | 24' - 10" | 26'-3" | 24' - 8" | 22'-10" |
|  |  | RFPI 70 | 31'-11" | 30' - 0' | 27'-9" | 29'-11" | 28'-1" | 26' - 0" | 27'-6" | 25'-10" | 23'-11" |
|  |  | RFPI 80S | 33' - 10" | 31' - 10" | 29'-5" | 31-9" | 29'-10" | 27' - 7" | 29'-2" | 27' - 5' | 25'-5" |
|  |  | RFPI 90 | 36' - 4" | 34'-1" | 31'-7" | 34' - 0" | 32' - 0" | 29'-7" | 31'-4" | 29'-5" | 27' - ${ }^{\prime \prime}$ |
|  | $16 "$ | RFPI 40S | 29'-10" | 27'-3" | 24' - 4" | 28'-10" | 26'-3" | 23'-6" | 27'-4" | 25'-1" | 22'-5" |
|  |  | RFPI 400 | 31'-11" | 29'-8" | 26' - 6" | 29'-10" | 28'-1" | 25' - 4" | 27'-6" | 25' - 10" | 23'-1" |
|  |  | RFPI 40 | 33' - 0" | 31'-0" | 28'-2" | 30'-11" | 29'-0" | 26'-11" | 28'-5" | 26' - 8" | 24'-9" |
|  |  | RFPI 60S | 33'-11" | 31'-10" | 28' - 7" | 31' - 9" | 29'-10" | 27' - 7" | 29'-2" | 27' - 5" | 25' - 5' |
|  |  | RFPI 70 | 35' - 6" | 33'-4" | 30'-2" | 33' - 3" | 31'-3" | 28'-2" | 30'-7" | 28'-9" | 25' - 8' |
|  |  | RFPI 80S | 37' - 7" | 35'-3" | 32' - 8" | 35' - 2" | 33'-1" | 30' - 7" | 32'-5" | 30' - 5" | 28'-2" |
|  |  | RFPI 90 | 40'-4" | 37'-10" | $35^{\prime}-0^{\prime \prime}$ | 37' - 9" | 35'-5" | 32'-10" | 34'-9" | 32 - 8" | 30' - 3' |



Notes:

1. Roofs must be sloped at least $1 / 4$ " in 12 " to assure drainage.
2. Deflection under live load is limited to L/240. Deflection under total load is limited to L/180. Verify that the deflection criteria conform to local building code requirements.
3. Table values apply to uniformly loaded simple or multiple span joists. Span is the horizontal distance from face to face of supports. Use appropriate software (e.g. Simpson Strong-Tie ${ }^{\circledR}$ Component Solutions ${ }^{\text {TM }}$ ) or engineering analysis to analyze multiple span joists if the length of any span is less than half the length of an adjacent span.
4. Minimum end bearing length is $1-3 / 4$ ". Minimum intermediate bearing length is 3-1/2".
5. Table values are based on cantilever lengths up to 2' max. Use member sizing software for longer cantilever lengths.
6. Web stiffeners are not required for spans shown, except for birdsmouth cuts and hangers.

## RigidRim ${ }^{\circledR}$ Rimboard Specifications

As a component of the Roseburg Framing System ${ }^{\circledR}$, RigidRim ${ }^{\circledR}$ Rimboard allows your customers to quickly frame the perimeter of their floor system and is one of the most cost-effective methods to properly transfer vertical and horizontal loads around the I-joist and directly into the supporting walls. RigidRim Rimboard is dimensionally stable and resists shrinking and warping. It also provides a smooth nailing surface for the attachment of exterior sheathing, siding and ledgers. Refer to page 18 for additional framing information. RigidRim Rimboard is currently available in the following materials, thicknesses and grades*:
$1-1 / 8^{\prime \prime}$ RigidRim $^{\circledR}$ OSB Rimboard
1-1/8" RigidRim $^{\circledR}$ Plus OSB Rimboard
$1-1 / 2^{\prime \prime}$ \& $1-3 / 4^{\prime \prime} 1.4 E$ RigidRim ${ }^{\circledR}$ LVL Rimboard
*Not all products are available in all markets. Contact your Roseburg EWP representative for availability.

The RigidRim OSB Rimboard products are available in lengths up to 24 ft , and the 1.4 E RigidRim LVL Rimboard is available in lengths up to 60 ft . All Rimboard products are available in all of the standard I-joist depths.
RigidRim Rimboard is manufactured in accordance with ANSI/APA PRR 410 Standard for Performance-Rated Engineered Wood Rim Boards which meets or exceeds the requirements given in the ICCES Acceptance Criteria for Wood-Based Rim Board Products, AC 124. Furthermore, the 1.4E LVL rimboard is included in ICC-ES code report ESR-1210. See Table 1 below for RigidRim design capacities. All RigidRim Rimboard products have been tested in the edgewise bending orientation and therefore may be designed for applications to support loads over window and door openings. See Table 2 below for allowable design properties for edgewise bending. See Table 3 below for allowable uniform loads for specified spans (see APA publication W345 Performance Rated Rim Boards ${ }^{\circledR}$ for additional information).


## TABLE 1: RIGIDRIM RIMBOARD DESIGN CAPACITIES (1) [2]|3]

|  | Rimboard Width (in) | Horizontal Load (plf) | Vertical Load (plf) | 1/2" Lag Screw Load (lbs) ${ }^{(4)}$ | Post Load (lbs) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| RigidRim ${ }^{\circledR}$ OSB | 1-1/8" | 180 (8d box or common) | 4,850 $/ 3,200^{6}$ | 350 | 3,500 ${ }^{7}$ |
| RigidRim ${ }^{\text {® }}$ Plus OSB | 1-1/8" | 200 (8d box or common) | 4,850 $/ 3,200^{6}$ | 350 | 3,500 ${ }^{7}$ |
| 1.4E RigidRim ${ }^{\text {® }}$ LVL | 1-1/2" | 215 (8d box or common) | 4,900 $/$ N.A. ${ }^{6}$ | 400 | 3,500 ${ }^{5}$ |
| 1.4E RigidRim ${ }^{\text {® }}$ LVL | 1-3/4" | 215 (8d box or common) | $5,500^{5} /$ N.A. ${ }^{6}$ | 400 | 3,500 ${ }^{5}$ |

1. All design properties assume rimboard nailing of 8 d nails at 6 " on-center. Additional nailing does not guarantee additional load capacity. Refer to APA document Y250 for additional load transfer details.
2. All design values, except Horizontal Load, are based on a 10 -year load duration (100\%) and should be adjusted for other load durations in accordance with the applicable code. Horizontal Load may not be adjusted for duration of load.
3. The 16 d (box or common) nails used to connect the bottom plate of a wall to the rimboard through the sheathing do not reduce the horizontal load capacity of the
rimboard provided that the 8 d nail spacing (sheathing to rimboard) is 6 " o.c. and the 16 d nail spacing (bottom plate to sheathing to rimboard) is in accordance with the prescriptive requirements of the applicable code.
4. Allowable load for lag screw installed perpendicular to wide face of rimboard.
5. Depth $\leq 16{ }^{\prime \prime}$
6. $16^{\prime \prime}<$ Depth $\leq 24$ ". Allowable load for intermediate depths can be found in APA publication W345.
7. Depth $\leq 24 "$

## TABLE 2: RIGIDRIM RIMBOARD EDGEWISE DESIGN PROPERTIES

|  | Flexural Stress | Modulus of Elasticity | Horizontal Shear | Compression Perpendicular to Grain ${ }^{(2)}$ |
| :---: | :---: | :---: | :---: | :---: |
| RigidRim $^{\circledR}$ OSB \& RigidRim |  |  |  |  |
| ® Plus OSB | $600 \mathrm{psi}^{(1)}$ | $0.55 \times 10^{6} \mathrm{psi}$ | 270 psi | 550 psi |
| $1.4 \mathrm{ERigidRim}{ }^{\circledR}$ LVL $^{2,250} \mathrm{psi}$ | $1.4 \times 10^{6} \mathrm{psi}$ | 200 psi | 560 psi |  |

$\begin{array}{lll}\text { (1) Allowable edgewise bending stress is applicable only to a span of } 4 \text { ' or less } & \text { (2)Compression Perpendicular to Grain value may not be increased for duration of load }\end{array}$
TABLE 3: ALLOWABLE UNIFORM LOAD FOR RIGIDRIM® OSB AND RIGIDRIM® PLUS OSB RIMBOARD USED AS HEADERS ${ }^{(11) 2|3| 3 \mid(4)}$

| Rimboard Size | Span |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 24" | 30" | 36" | 42" | 48" |
|  | Total Load (plf)/Minimum End Bearing (in) |  |  |  |  |
| 1-1/8"x 9-1/2" | 1,330 / 3.0 | 890 / 3.0 | 630 / 3.0 | 510 / 1.5 | 390 / 1.5 |
| $1-1 / 8$ "x 11-7/8" | 1,870 / 4.5 | 1,270 / 4.5 | 990 / 3.0 | 740 / 3.0 | 580 / 3.0 |
| 2 ply 1-1/8"x 14 " | 4,520 / 6.0 | 3,540 / 4.5 | 2,570 / 4.5 | 1,940 / 4.5 | 1,610 / 3.0 |
| 2 ply 1-1/8"x 16 " | 5,170 / 6.0 | 4,250 / 6.0 | 3,120 / 6.0 | 2,540 / 4.5 | 1,990 / 4.5 |

[^1]
## RigidLam ${ }^{\circledR}$ LVL Product Line

You've probably been building with traditional solid sawn lumber beams, headers, columns, studs and stair stringers for as long as you've been building. Now through advances in technology and design, there is a better choice - RigidLam LVL (Laminated Veneer Lumber) beams, headers, columns, studs and stair stringers. They are simply a better alternative than traditional solid sawn lumber pieces. Work with a stronger, stiffer, more consistent and more predictable building material. Compared with similar sized sections, our RigidLam LVL products can support heavier loads and allow greater spans than conventional lumber.

## MOISTURE REPELLENT SEALER

RigidLam LVL is coated with a wax-based moisture repellent sealer that is formulated specifically for LVL to provide temporary protection against moisture issues during normal storage and construction schedules. It is applied to all six sides of the LVL during the manufacturing process.


## STORAGE, HANDLING \& INSTALLATION

- Do not drop RigidLam LVL off the delivery truck. Best practice is use of a forklift or boom.
- RigidLam LVL should be stored lying flat and protected from the weather.
- Keep the material a minimum of 6 " above ground to minimize the absorption of ground moisture and allow circulation of air.
- Bundles should be supported every 10 ' or less.
- RigidLam LVL is for use in covered, dry conditions only. Protect from the weather on the jobsite both before and after installation.
- $1-1 / 2^{\prime \prime} \times 14$ " and deeper and $1-3 / 4$ " $\times 16^{\prime \prime}$ and deeper must be a minimum of two plies unless designed by a design professional for a specific application.
- RigidLam LVL headers and beams shall not be cut, notched or drilled except as shown below. Heel cuts may be possible. Contact your Roseburg Forest Products representative.
- It is permissible to rip RigidLam LVL to a non-standard depth provided it is structurally adequate for the applied loads. Use appropriate software (e.g. Simpson Strong-Tie ${ }^{\circledR}$ Component Solutions ${ }^{\text {TM }}$ ) or engineering analysis to analyze non-standard depths.
- Protect RigidLam LVL from direct contact with concrete or masonry.
- Ends of RigidLam LVL bearing in concrete or masonry pockets must have a minimum of $1 / 2$ " airspace on top, sides and end.
- RigidLam LVL is manufactured without camber and therefore may be installed with either edge up or down.
- Do not install damaged RigidLam LVL.
- Do not walk on beams until they are fully braced, or serious injuries may result.

See additional notes on page 7

## PERMISSIBLE HORIZONTAL ROUND HOLE LOCATION FOR RIGIDLAM® LVL BEAMS


$=$ Zone where horizontal holes are permitted for passage of wires, conduit, etc.

- For beam depths (d) of 4-3/8,5-1/2, and 7-1/4 inches, the maximum hole diameter is $1,1-1 / 8$, and $1-1 / 2$ inches, respectively.
- For deeper beams, the maximum hole diameter is 2 inches.
- Diagram applies for simple and multi-span applications with uniform loading.
- No more than 3 holes per span are permitted.
- Holes should not be cut in cantilevers.
- Note: Larger holes, more holes and/or holes that are located outside of the shaded area shown may be permissible as verified by appropriate software (e.g. Simpson Strong-Tie ${ }^{\circledR}$ Component Solutions ${ }^{T W}$ ) or engineering analysis.


## minimum nail spacing for rigidlam liv beams



| Nail Size | Minimum <br> Parallel <br> Spacing | Minimum <br> Parallel End <br> Distance | Minimum <br> Perpendicular <br> Spacing |
| :---: | :---: | :---: | :---: |
| 8d Box | $2 "$ | $1-1 / 2 "$ | $2 "$ |
| 8d Common | $3 "$ | $2 "$ | $2 "$ |
| 10d \& 12d Box | $3 "$ | $2 "$ | $2 "$ |
| 10d \& 12d Common | $4 "$ | $3 "$ | $3 "$ |
| 16d Sinker | $4 "$ | $3 "$ | $3 "$ |
| 16d Common | $6 "$ | $4 "$ | $3 "$ |

## Available RigidLam ${ }^{\circledR}$ LVL Sizes*


*Not all grades and/or sizes available in all markets. Contact your Roseburg EWP representative for availability.

## RigidLam ${ }^{\circledR}$ LVL Allowable Design Stresses ${ }^{1}$

|  |  | 1.4E LVL | 1.6E LVL | 2.1E LVL | 2.3E LVL |
| :---: | :---: | :---: | :---: | :---: | :---: |
| True Modulus of Elasticity (MOE) ${ }^{2}$ - Edgewise or Flatwise | $E(p s i)=$ | 1,400,000 | 1,600,000 | 2,100,000 | 2,300,000 |
| Apparent Modulus of Elasticity (MOE) ${ }^{2}$ - Edgewise or Flatwise | $E(p s i)=$ | 1,300,000 | 1,500,000 | 2,000,000 | 2,200,000 |
| Bending - Edgewise ${ }^{3,4}$ | $\mathrm{F}_{\mathrm{b}}$ edge (psi) $=$ | 2,250 | 2,250 | 3,100 | 3,100 |
| Bending - Flatwise ${ }^{5}$ | $\mathrm{F}_{\mathrm{b}} \mathrm{flat}(\mathrm{psi})=$ | 2,250 | 2,250 | 3,100 | 3,100 |
| Horizontal Shear - Edgewise | $\mathrm{F}_{\mathrm{V}}$ edge (psi) $=$ | 200 | 220 | 290 | 290 |
| Horizontal Shear - Flatwise | $\mathrm{F}_{\mathrm{V}}$ flat (psi) $=$ | 130 | 130 | 130 | 130 |
| Compression Perp. To Grain ${ }^{2}$ - Edgewise | $\mathrm{F}_{\mathrm{C} \text { perp }}$ edge ( psi ) $=$ | 560 | 575 | 750 | 750 |
| Compression Perp. To Grain ${ }^{2}$ - Flatwise | $\mathrm{F}_{\mathrm{C} \text { perp }} \mathrm{flat}_{(\mathrm{psi}}(\mathrm{C}$ | 650 | 650 | 650 | 650 |
| Compression Parallel to Grain | $\mathrm{F}_{\mathrm{Cpara}}(\mathrm{psi})=$ | 1,950 | 1,950 | 3,000 | 3,000 |
| Tension Parallel to Grain ${ }^{6}$ | $\mathrm{F}_{\mathrm{t}}(\mathrm{psi})=$ | 1,500 | 1,500 | 2,100 | 2,100 |
| MOE for stability calculations ${ }^{2}$ | $\mathrm{E}_{\text {min }}(\mathrm{psi})=$ | 704,639 | 805,301 | 1,056,958 | 1,157,620 |

1. These allowable design stresses apply to dry service conditions. RigidLam shall not be used for exterior conditions exposed to elements.
2. No increase is allowed for duration of load.
3. The tabulated values are based on a reference depth of 12 inches. For other depths, when loaded edgewise, the allowable bending stress (Fb) shall be modified by a depth factor, $\mathrm{Kd}=(12 / \mathrm{d})^{1 / 8}$ for Douglas-fir (DF) LVL (Mill \#1055) or $\mathrm{Kd}=(12 / \mathrm{d})^{1 / 5}$ for Southern Pine (SP) LVL (Mill \#1125), where dis the LVL depth in inches. For depths less than 3-1/2 inches, multiply the tabulated value by 1.17 for DF LVL or 1.28 for SP LVL.
4. A factor of 1.04 may be applied for repetitive members as defined in the National Design Specification for Wood Construction.
5. Tabulated $\mathrm{F}_{\mathrm{b}}$ flat values are based on a width of $1-3 / 4$. For other widths, when loaded flatwise, multiply $\mathrm{F}_{\mathrm{b}}$ flat by $(1.75 / \mathrm{t})^{1 / 5}$, where $t$ is the LVL width in inches. For widths less than $1-3 / 4$ ", use the tabulated value.
6. Tensile stress is based on a 4-foot gage length. For greater lengths, multiply $F_{t}$ by $(4 / L)^{1 / 9}$ where $L=$ length in feet. For lengths less than four feet, use the tabulated value.

## RigidLam ${ }^{\circledR}$ LVL Design Values (1-Ply 1-3/4" Edgewise)

| 1.6E Douglas-fir |  |  |  |  | 2.1E Douglas-fir |  |  |  | 2.3E Douglas-fir |  |  |  | 2.15 Southern Pine |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| RigidL_am LVL |  |  |  |  | RigidLam LVL |  |  |  | RigidLam LVL |  |  |  | RigidLam LVL |  |  |  |
| Depth (in) | Max. Vert. Shear (lbs) | Max. Moment (ft-lbs) | $\begin{aligned} & \mathrm{EL} \mathrm{x}^{10^{6}} \\ & \left(\mathrm{lbs}-\mathrm{in}^{2}\right) \end{aligned}$ | Approx. Weight (lbs/ft) | Max. Vert. Shear (lbs) | Max. Moment (ft-lbs) | $\begin{aligned} & \mathrm{El} \times 10^{6} \\ & \left(\mathrm{lbs}-\mathrm{in}^{2}\right) \end{aligned}$ | Approx. Weight (lbs/ft) | Max. Vert. <br> Shear (lbs) | Max. Moment (ft-lbs) | $\begin{aligned} & \mathrm{EL} \mathrm{x10} \mathbf{x}^{6} \\ & \left(\mathrm{lbs}-\mathrm{in}^{2}\right) \end{aligned}$ | Approx. Weight (lbs/ft) | Max. Vert. Shear (lbs) | Max. Moment (ft-lbs) | $\begin{aligned} & \text { El x10 } 0^{6} \\ & \left(\mathrm{lbs}-\mathrm{in}^{2}\right) \end{aligned}$ | Approx. Weight (lbs/ft) |
| 3-1/2 | 898 | 781 | 10 | 1.53 | 1,184 | 1,077 | 13 | 1.62 | 1,184 | 1,077 | 14 | 1.62 | 1,184 | 1,181 | 13 | 1.79 |
| 4-3/8 | 1,123 | 1,187 | 20 | 1.91 | 1,480 | 1,636 | 26 | 2.02 | 1,480 | 1,636 | 28 | 2.02 | 1,480 | 1,765 | 26 | 2.23 |
| 5-1/4 | 1,348 | 1,671 | 34 | 2.30 | 1,776 | 2,303 | 44 | 2.42 | 1,776 | 2,303 | 49 | 2.42 | 1,776 | 2,450 | 44 | 2.68 |
| 5-1/2 | 1,412 | 1,824 | 39 | 2.41 | 1,861 | 2,513 | 51 | 2.54 | 1,861 | 2,513 | 56 | 2.54 | 1,861 | 2,664 | 51 | 2.81 |
| 7 | 1,797 | 2,866 | 80 | 3.06 | 2,368 | 3,949 | 105 | 3.23 | 2,368 | 3,949 | 115 | 3.23 | 2,368 | 4,112 | 105 | 3.57 |
| 7-1/4 | 1,861 | 3,061 | 89 | 3.17 | 2,453 | 4,218 | 117 | 3.35 | 2,453 | 4,218 | 128 | 3.35 | 2,453 | 4,380 | 117 | 3.70 |
| 9-1/4 | 2,374 | 4,834 | 185 | 4.05 | 3,130 | 6,660 | 242 | 4.27 | 3,130 | 6,660 | 265 | 4.27 | 3,130 | 6,791 | 242 | 4.72 |
| 9-1/2 | 2,438 | 5,082 | 200 | 4.16 | 3,214 | 7,002 | 263 | 4.39 | 3,214 | 7,002 | 288 | 4.39 | 3,214 | 7,125 | 263 | 4.85 |
| 11-1/4 | 2,888 | 6,977 | 332 | 4.92 | 3,806 | 9,613 | 436 | 5.20 | 3,806 | 9,613 | 478 | 5.20 | 3,806 | 9,660 | 436 | 5.74 |
| 11-7/8 | 3,048 | 7,722 | 391 | 5.20 | 4,018 | 10,639 | 513 | 5.48 | 4,018 | 10,639 | 562 | 5.48 | 4,018 | 10,647 | 513 | 6.06 |
| 14 | 3,593 | 10,514 | 640 | 6.13 | 4,737 | 14,486 | 840 | 6.47 | 4,737 | 14,486 | 920 | 6.47 | 4,737 | 14,320 | 840 | 7.15 |
| 16 | 4,107 | 13,506 | 956 | 7.00 | 5,413 | 18,608 | 1,254 | 7.39 | 5,413 | 18,608 | 1,374 | 7.39 | 5,413 | 18,210 | 1,254 | 8.17 |
| 18 | 4,620 | 16,843 | 1,361 | 7.88 | 6,090 | 23,206 | 1,786 | 8.31 | 6,090 | 23,206 | 1,956 | 8.31 | 6,090 | 22,511 | 1,786 | 9.19 |
| 20 | 5,133 | 20,522 | 1,867 | 8.75 | 6,767 | 28,275 | 2,450 | 9.24 | 6,767 | 28,275 | 2,683 | 9.24 | 6,767 | 27,212 | 2,450 | 10.21 |
| 22 | 5,647 | 24,537 | 2,485 | 9.63 | 7,443 | 33,807 | 3,261 | 10.16 | 7,443 | 33,807 | 3,572 | 10.16 | 7,443 | 32,305 | 3,261 | 11.23 |
| 24 | 6,160 | 28,886 | 3,226 | 10.50 | 8,120 | 39,798 | 4,234 | 11.08 | 8,120 | 39,798 | 4,637 | 11.08 | 8,120 | 37,782 | 4,234 | 12.25 |

[^2]
## RigidLam ${ }^{\circledR}$ LVL Online Resources

## The following RigidLam LVL resources can be accessed online at www.roseburg.com:

- PLF (pounds per lineal foot) Tables for Douglas-fir and Southern Pine LVL
- Quick Reference Tables for Douglas-fir and Southern Pine LVL
- Floor beams
- 1-story garage door headers
- 2-story garage door headers
- 1-story window \& door headers
- 2-story window \& door headers
- RigidLam LVL Column Tables for Douglas-fir
- RigidLam LVL Bearing Length Requirements


# RigidLam ${ }^{\circledR}$ LVL Columns 

Douglas-fir LVL

ALLOWABLE AXIAL LOAD CAPACITY (LBS) FOR 1.6E RIGIDLAM®LVL COLUMNS

| Effective Column Length (ft.) | Column Size |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3-1/2" $\times 3-1 / 2^{\prime \prime}$ |  |  | 3-1/2" $\times$ 5-1/2" |  |  | 3-1/2" $\times 7-1 / 4$ " |  |  | 5-1/4" $\times 5-1 / 2^{\prime \prime}$ |  |  | 5-1/4" $\times 7-1 / 4^{\prime \prime}$ |  |  | 7" $\times$ 7-1/4" |  |  |
|  | $\begin{aligned} & \text { Floor } \\ & 100 \% \end{aligned}$ | $\begin{aligned} & \text { Roof } \\ & \text { Snow } \\ & 115 \% \end{aligned}$ | Roof <br> Live <br> 125\% | $\begin{aligned} & \text { Floor } \\ & \text { 100\% } \end{aligned}$ | $\begin{aligned} & \text { Roof } \\ & \text { Snow } \\ & 115 \% \end{aligned}$ | $\begin{aligned} & \text { Roof } \\ & \text { Live } \\ & \text { 125\% } \end{aligned}$ | $\begin{aligned} & \text { Floor } \\ & 100 \% \end{aligned}$ | $\begin{aligned} & \text { Roof } \\ & \text { Snow } \\ & \text { 115\% } \end{aligned}$ | $\begin{aligned} & \text { Roof } \\ & \text { Live } \\ & 125 \% \end{aligned}$ | $\begin{aligned} & \text { Floor } \\ & 100 \% \end{aligned}$ | $\begin{aligned} & \text { Roof } \\ & \text { Snow } \\ & 115 \% \end{aligned}$ | $\begin{aligned} & \text { Roof } \\ & \text { Live } \\ & \text { 125\% } \end{aligned}$ | Floor 100\% | $\begin{aligned} & \text { Roof } \\ & \text { Snow } \\ & 115 \% \end{aligned}$ | $\begin{aligned} & \text { Roof } \\ & \text { Live } \\ & 125 \% \end{aligned}$ | $\begin{aligned} & \text { Floor } \\ & \text { 100\% } \end{aligned}$ | $\begin{aligned} & \text { Roof } \\ & \text { Snow } \\ & \text { 115\% } \end{aligned}$ | $\begin{gathered} \text { Roof } \\ \text { Live } \\ 125 \% \end{gathered}$ |
| 6 | 8,555 | 9,110 | 9,425 | 13,455 | 14,315 | 14,815 | 17,735 | 18,870 | 19,530 | 26,535 | 29,405 | 31,170 | 34,975 | 38,760 | 41,090 | 50,400 | 56,800 | 60,905 |
| 7 | 7,160 | 7,530 | 7,745 | 11,250 | 11,835 | 12,170 | 14,830 | 15,600 | 16,045 | 24,270 | 26,535 | 27,885 | 31,990 | 34,980 | 36,760 | 48,020 | 53,710 | 57,300 |
| 8 | 6,015 | 6,275 | 6,430 | 9,450 | 9,870 | 10,105 | 12,460 | 13,010 | 13,320 | 21,900 | 23,615 | 24,605 | 28,870 | 31,130 | 32,435 | 45,385 | 50,320 | 53,370 |
| 9 | 5,095 | 5,290 | 5,400 | 8,010 | 8,315 | 8,485 | 10,560 | 10,960 | 11,190 | 19,575 | 20,865 | 21,615 | 25,805 | 27,505 | 28,490 | 42,545 | 46,710 | 49,230 |
| 10 | 4,360 | 4,510 | 4,590 | 6,855 | 7,085 | 7,220 | 9,035 | 9,340 | 9,515 | 17,425 | 18,410 | 18,990 | 22,970 | 24,270 | 25,030 | 39,560 | 42,995 | 45,035 |
| 11 | 3,770 | 3,880 | 3,945 | 5,920 | 6,100 | 6,200 | 7,805 | 8,040 | 8,175 | 15,510 | 16,290 | 16,745 | 20,445 | 21,475 | 22,075 | 36,570 | 39,350 | 40,975 |
| 12 | 3,285 | 3,375 | 3,425 | 5,160 | 5,300 | 5,385 | 6,805 | 6,990 | 7,095 | 13,845 | 14,475 | 14,840 | 18,255 | 19,085 | 19,565 | 33,645 | 35,905 | 37,220 |
| 13 | 2,885 | 2,955 | 3,000 | 4,535 | 4,650 | 4,715 | 5,980 | 6,130 | 6,215 | 12,410 | 12,915 | 13,215 | 16,360 | 17,025 | 17,425 | 30,870 | 32,725 | 33,810 |
| 14 | 2,555 | 2,610 | 2,645 | 4,015 | 4,105 | 4,160 | 5,290 | 5,410 | 5,480 | 11,170 | 11,585 | 11,835 | 14,725 | 15,275 | 15,600 | 28,320 | 29,870 | 30,770 |
| 15 |  |  |  |  |  |  |  |  |  | 10,090 | 10,445 | 10,645 | 13,300 | 13,765 | 14,035 | 26,005 | 27,320 | 28,070 |
| 16 |  |  |  |  |  |  |  |  |  | 9,155 | 9,455 | 9,625 | 12,070 | 12,460 | 12,685 | 23,920 | 25,035 | 25,680 |
| 17 |  |  |  |  |  |  |  |  |  | 8,340 | 8,590 | 8,735 | 10,995 | 11,325 | 11,520 | 22,045 | 23,000 | 23,555 |
| 18 |  |  |  |  |  |  |  |  |  | 7,620 | 7,840 | 7,965 | 10,050 | 10,335 | 10,500 | 20,355 | 21,185 | 21,665 |
| 19 |  |  |  |  |  |  |  |  |  | 6,995 | 7,180 | 7,285 | 9,220 | 9,465 | 9,605 | 18,850 | 19,575 | 19,985 |
| 20 |  |  |  |  |  |  |  |  |  | 6,435 | 6,600 | 6,695 | 8,485 | 8,700 | 8,825 | 17,490 | 18,120 | 18,480 |
| 21 |  |  |  |  |  |  |  |  |  | 5,940 | 6,085 | 6,165 | 7,830 | 8,020 | 8,125 | 16,265 | 16,820 | 17,140 |
| 22 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | 15,160 | 15,645 | 15,930 |
| 23 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | 14,150 | 14,590 | 14,840 |
| 24 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | 13,245 | 13,630 | 13,855 |
| 25 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | 12,420 | 12,765 | 12,965 |

ALLOWABLE AXIAL LOAD CAPACITY (LBS) FOR 2.1E RIGIDLAM®LVL COLUMNS

| Effective Column Length (ft.) | Column Size |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3-1/2" $\times 3-1 / 2^{\prime \prime}$ |  |  | 3-1/2" $\times$ 5-1/2" |  |  | 3-1/2" $\times 7$-1/4" |  |  | 5-1/4" $\times 5-1 / 2$ " |  |  | 5-1/4" $\times 7-1 / 4$ " |  |  | 7" $\times 7-1 / 4 "$ |  |  |
|  | Floor 100\% | Roof Snow 115\% | $\begin{gathered} \text { Roof } \\ \text { Live } \\ 125 \% \end{gathered}$ | Floor 100\% | Roof Snow 115\% | Roof Live 125\% | $\begin{aligned} & \text { Floor } \\ & \text { 100\% } \end{aligned}$ | Roof Snow $115 \%$ | $\begin{gathered} \text { Roof } \\ \text { Live } \\ 125 \% \end{gathered}$ | $\begin{aligned} & \text { Floor } \\ & \text { 100\% } \end{aligned}$ | Roof Snow $115 \%$ | $\begin{gathered} \text { Roof } \\ \text { Live } \\ 125 \% \end{gathered}$ | $\begin{aligned} & \text { Floor } \\ & \text { 100\% } \end{aligned}$ | Roof Snow $115 \%$ | $\begin{aligned} & \text { Roof } \\ & \text { Live } \\ & 125 \% \end{aligned}$ | $\begin{aligned} & \text { Floor } \\ & \text { 100\% } \end{aligned}$ | Roof Snow 115\% | Roof Live $125 \%$ |
| 6 | 11,585 | 12,280 | 12,685 | 18,205 | 19,300 | 19,935 | 23,995 | 25,440 | 26,280 | 37,155 | 40,960 | 43,285 | 48,975 | 53,995 | 57,055 | 71,625 | 80,475 | 86,125 |
| 7 | 9,615 | 10,085 | 10,360 | 15,110 | 15,850 | 16,280 | 19,920 | 20,895 | 21,465 | 33,560 | 36,465 | 38,195 | 44,240 | 48,070 | 50,345 | 67,730 | 75,460 | 80,290 |
| 8 | 8,040 | 8,375 | 8,570 | 12,635 | 13,165 | 13,470 | 16,655 | 17,355 | 17,755 | 29,925 | 32,090 | 33,355 | 39,445 | 42,300 | 43,970 | 63,480 | 70,035 | 74,045 |
| 9 | 6,795 | 7,040 | 7,185 | 10,680 | 11,065 | 11,295 | 14,075 | 14,585 | 14,885 | 26,495 | 28,135 | 29,090 | 34,925 | 37,090 | 38,345 | 58,980 | 64,375 | 67,610 |
| 10 | 5,800 | 5,990 | 6,100 | 9,120 | 9,420 | 9,585 | 12,025 | 12,415 | 12,635 | 23,445 | 24,715 | 25,450 | 30,910 | 32,580 | 33,545 | 54,365 | 58,730 | 61,305 |
| 11 | 5,005 | 5,150 | 5,235 | 7,865 | 8,095 | 8,225 | 10,370 | 10,670 | 10,845 | 20,805 | 21,805 | 22,380 | 27,430 | 28,740 | 29,505 | 49,815 | 53,355 | 55,425 |
| 12 | 4,355 | 4,470 | 4,540 | 6,845 | 7,030 | 7,130 | 9,025 | 9,265 | 9,400 | 18,515 | 19,325 | 19,790 | 24,410 | 25,470 | 26,085 | 45,545 | 48,400 | 50,105 |
| 13 | 3,825 | 3,915 | 3,970 | 6,010 | 6,155 | 6,240 | 7,925 | 8,115 | 8,225 | 16,565 | 17,215 | 17,600 | 21,835 | 22,695 | 23,200 | 41,630 | 43,985 | 45,370 |
| 14 | 3,380 | 3,455 | 3,500 | 5,315 | 5,430 | 5,500 | 7,005 | 7,160 | 7,250 | 14,880 | 15,425 | 15,735 | 19,615 | 20,330 | 20,740 | 38,055 | 40,040 | 41,195 |
| 15 |  |  |  |  |  |  |  |  |  | 13,430 | 13,885 | 14,150 | 17,705 | 18,305 | 18,650 | 34,855 | 36,535 | 37,525 |
| 16 |  |  |  |  |  |  |  |  |  | 12,170 | 12,550 | 12,775 | 16,045 | 16,545 | 16,840 | 31,995 | 33,425 | 34,255 |
| 17 |  |  |  |  |  |  |  |  |  | 11,080 | 11,400 | 11,585 | 14,605 | 15,030 | 15,275 | 29,445 | 30,665 | 31,385 |
| 18 |  |  |  |  |  |  |  |  |  | 10,115 | 10,395 | 10,555 | 13,335 | 13,700 | 13,915 | 27,165 | 28,225 | 28,835 |
| 19 |  |  |  |  |  |  |  |  |  | 9,275 | 9,515 | 9,650 | 12,225 | 12,540 | 12,720 | 25,125 | 26,045 | 26,575 |
| 20 |  |  |  |  |  |  |  |  |  | 8,530 | 8,735 | 8,855 | 11,240 | 11,515 | 11,675 | 23,285 | 24,095 | 24,565 |
| 21 |  |  |  |  |  |  |  |  |  | 7,870 | 8,055 | 8,155 | 10,375 | 10,615 | 10,750 | 21,635 | 22,345 | 22,755 |
| 22 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | 20,150 | 20,780 | 21,130 |
| 23 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | 18,800 | 19,360 | 19,685 |
| 24 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | 17,580 | 18,080 | 18,365 |
| 25 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | 16,475 | 16,920 | 17,175 |

## Notes:

1. Column is a single, one-piece member for dry-use applications only. DO NOT use these tables for multi-ply, built-up column applications.
2. Column is assumed to have adequate bracing in all directions at both ends.
3. Loads are calculated per the 2018 National Design Specification ${ }^{\circledR}$ for axial loads only.
4. For side-loaded columns, use appropriate design software or consult with a design professional.
5. Table assumes an eccentricity of $1 / 6$ of the smaller column dimension.
6. Table assumes column bearing to be steel or concrete. When bearing on a wood plate (with $F_{C}$ perp $=425 \mathrm{PSI}$ ), axial loads shall not exceed the load shown below for the given column size for all durations of load:

 | Load (lbs.) | 5,206 | 8,181 | 10,784 | 12,272 | 16,177 | 21,569 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- |

Note: The 2.1 E chart is the same as 2.0 E apparent because 2.1 E is $5 \%$ higher than 2.0 E and the multiplier for Emin went from 1.05 to 1.0 when switching to True MOE. This will not be the case for other MOE values. The values for 1.6 E true will be slightly higher than the values for 1.5 E apparent because 1.6 E is $6.67 \%$ higher than 1.5 E but Emin is still reduced by $5 \%$. The values for 2.3 E true will be slightly lower than the values for 2.2 E app since 2.3 is only $4.5 \%$ higher than 2.2. Douglas-fir and Southern pine values are the same.
1.6E RigidLam LVL Allowable Design Stresses ${ }^{(1)}$

True Modulus of Elasticity (MOE) E $=1,600,000$ psi $^{(2)}$
Bending (edgewise \& flatwise) $\quad \mathrm{F}_{\mathrm{b}}=2,250 \mathrm{psi}^{(3)(4)}$
Compression Parallel to Grain $\quad \mathrm{F}_{\mathrm{c}}=1,950 \mathrm{psi}$
2.1E RigidLam LVL Allowable Design Stresses ${ }^{(1)}$

True Modulus of Elasticity (MOE) $\mathrm{E}=2,100,000 \mathrm{psi}^{(2)}$
Bending (edgewise \& flatwise) $\quad \mathrm{F}_{\mathrm{b}}=3,100 \mathrm{psi}^{(3)(4)}$
Compression Parallel to Grain $\quad \mathrm{F}_{\mathrm{c}}=3,000 \mathrm{psi}$
(1) These allowable design stresses apply to dry service conditions.
(2) No increase is allowed for duration of load.
(3) Edgewise bending: For depths other than 12" multiply $\mathrm{F}_{\mathrm{b}}$ by $(12 / \mathrm{d})^{1 / 8}$, where $d=$ depth of member (inches).
(4) Flatwise bending: For thicknesses greater than 1-3/4" multiply $F_{b}$ by $(1.75 / \mathrm{t})^{1 / 5}$, where $\mathrm{t}=$ thickness of member (inches).

## Load Development

## CASE ONE: FLUSH BEAM



Typical Flush Beam Framing

Step 1 Determine the Trib Width (expressed in units of feet). In the example at right, the Trib Width $=20^{\prime}$.
Step 2 Determine the Live Load plf and Total Load plf on the Beam: $\mathrm{plf}_{\mathrm{LL}}=\left(\mathrm{psf}_{\mathrm{LL}}\right) \times($ Trib Width $)$. Here, $\mathrm{plf}_{\mathrm{LL}}=40 \mathrm{psf} \times 20^{\prime}=800 \mathrm{plf}_{\mathrm{LL}}$ $\mathrm{plf}_{\mathrm{TL}}=\left(\mathrm{psf}_{\mathrm{TL}}\right) \times($ Trib Width $)$. Here, $\mathrm{plf}_{\mathrm{TL}}=50 \mathrm{psf} \times 20^{\prime}=1,000 \mathrm{plf}_{\mathrm{TL}}$
Step 3 Use the appropriate PLF Table (pages 48-53), and match the span of the LVL beam with the left "Span" column of the table. Always round the beam span up to the next whole foot ( 14 ' for this example).


Step 4 For a span of $14^{\prime}$, going from left to right, find a beam that supports a LL equal to or greater than 800 plf and a TL equal to or greater than 1,000 plf. Both checks must be made to properly size the beam.
Step 5 A 2 ply 14 " RigidLam LVL will work ( $820>800$ and $1,155>1,000$ ) but a 3 ply 11-7/8," comes close. To check if the 3 ply 11-7/8," LVL works at the actual span of $13^{\prime}-6^{\prime \prime}$, interpolate the table between $12^{\prime}$ and 14 '. If you are not familiar with this, use the diagram as shown below to set up the interpolation as follows:

For LL $\frac{(1,194-771)}{\left(14^{\prime}-12^{\prime}\right)}=\frac{(?-771)}{\left(14^{\prime}-13.5^{\prime}\right)} \Rightarrow 211.5=\frac{(?-771)}{0.5} \Rightarrow(211.5 \times 0.5)+771=? \Rightarrow ?=876.75$ plf $>800$ plf $\underline{\text { OK }}$
The plf value for TL at $14^{\prime}$ is 1,139 plf and since this is greater than the required 1,000 plf, interpolation is not required for total load.

Therefore, an alternative solution would be a 3 ply 11-7/8," 2.1 E RigidLam LVL ( $877>800$ and $1,139>1,000$ )


CASE TWO: DROPPED BEAM


When the LVL beam is dropped and the I-joists are continuous over the beam, there is more load transferred to the beam. This is because the continuous I-joists increase the trib width of the beam (green shaded area).

If both spans of the I-joist are equal, there is $25 \%$ more load put onto the LVL beam. If both spans are not equal, like shown in the diagram to the right (Span $B>S p a n A$ ), there is even more load placed onto the LVL beam. The exact formula is complicated but fortunately there is a simple and safe way to size the LVL beam:

Step 1 Assume both spans of the I-joist to be the longest span. In the example to the right, this would be Span B ( 21.25 ft ).

Step 2 Calculate the plf on the LVL beam as if it were flush and increase by 25\%:

$$
\begin{aligned}
& \operatorname{plf}_{\mathrm{LL}}=40 \mathrm{psf} \times 21.25 \times 1.25=1,063 \mathrm{plf}_{\mathrm{LL}} \\
& \operatorname{plf}_{\mathrm{TL}}=50 \mathrm{psf} \times 21.25 \times 1.25=1,329 \mathrm{plf}_{\mathrm{TL}}
\end{aligned}
$$

Step 3 Use the longest span of the LVL beam (round up to the next whole foot, 16 ' for this example) and use the appropriate plf table. In this example, use a 2 ply $2.1 \mathrm{E} 18^{\prime \prime}$ RigidLam LVL beam ( $1,138>1,063 \& 1,389>1,329$ ).


This method will always be safe provided the long span of the I-joist (Span B) is not more than five times longer than the shorter span (Span A). When possible, use appropriate software (e.g. Simpson Strong-Tie ${ }^{\circledR}$ Component Solutions ${ }^{T M}$ ) or engineering analysis to determine solution.

# RigidLam LVL Bearing Details <br> Please refer to the RigidLam LVL Bearing Length Requirements document on the Roseburg website (www.roseburg.com). 




Bearing on Wood Column
Verify the required bearing length and the ability of the supporting column member to provide adequate strength.


Bearing on Exterior Wall Check for proper beam bearing length based on plate material.

Bearing on Steel Column Verify the required bearing length and the ability of the supporting column member to provide adequate strength.


## Pocket Construction

Provide 1/2" air space on top, sides and end of RigidLam LVL beams.


Provide moisture barrier between RigidLam LVL beams and concrete.

# Fastening Recommendations For Multiple Ply Members 

## Top Loaded Members - 2 \& 3 Ply

For 12" deep (or less) members, nail plies together with two rows of $16 \mathrm{~d} \times 3-1 / 2^{\prime \prime}$ com nails at 12" o.c. (add 1 row for 16d sinkers).
For 14 ", 16 " or $18^{\prime \prime}$ deep members, nail plies together



3-Ply
with three rows of $16 \mathrm{~d} \times 3-1 / 2^{\prime \prime}$ com. nails at 12 " o.c. (add one row for 16 d sinkers). For 20 ", $22^{\prime \prime}$ or 24 " deep members, nail plies together with four rows of $16 \mathrm{~d} \times 3-1 / 2^{\prime \prime}$ com. nails at 12 " o.c. (add one row for 16 d sinkers).

Top Loaded Members - 4 Ply
For 4-Ply Top Loaded members, it is recommended to connect the plies together with appropriate wood screws (see page 42 for approved wood screws).
The recommended fastener spacing is two rows at $24^{\prime \prime}$ o.c. for up to and including $16^{\prime \prime}$ deep members, and three rows at $24^{\prime \prime}$ o.c. for members up to and including $24^{\prime \prime}$ deep. If the fastener point penetrates a minimum of $75 \%$ of the 4th ply,
 they may be applied from one side of the beam; otherwise, the fasteners must be applied from both sides and staggered. Load must be applied evenly to all four plies; otherwise, use connections for side loaded members.

## Side Loaded Members

| MAXIMUM UNFORM LOAD APPLIED TO ETTHER OUTSIDE PIY - POUNDS PER INNEALFOOT |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| \# of 1-1/2" Plies | Nailed: 10d common (0.148" $\varnothing \times 3$ ') |  | Bolted: 1/2" $\varnothing$ Through Bolt |  |  |  |  |  |
|  | 2 rows at 12" o.c. | 3 rows at 12" o.c. | 2 rows at 24" o.c. |  | 2 rows at 12" o.c. |  | 3 rows at 12" o.c. |  |
|  | All Grades | All Grades | 1.4E / 1.6E | 2.1E / 2.3E | 1.4E/1.6E | 2.1E / 2.3E | 1.4E/1.6E | 2.1E / 2.3E |
| 2 | 465 | 700 | 395 | 435 | 795 | 870 | 1,190 | 1,305 |
| 3 | 350 | 525 | 295 | 325 | 595 | 650 | 895 | 980 |
| 4 | - | - | 265 | 290 | 530 | 580 | 795 | 870 |
| \# of 1-3/4" Plies | Nailed: 16 d common (0.162" ø X 3-1/2") |  | Bolted: 1/2" $\varnothing$ Through Bolt |  |  |  |  |  |
|  | 2 rows at 12" o.c. | 3 rows at 12" o.c. | 2 rows at 24" o.c. |  | 2 rows at 12" o.c. |  | 3 rows at 12" o.c. |  |
|  | All Grades | All Grades | 1.4E/1.6E | 2.1E / 2.3E | 1.4E / 1.6E | 2.1E / 2.3E | 1.4E/ 1.6E | 2.1E / 2.3E |
| 2 | 560 | 845 | 460 | 505 | 925 | 1,015 | 1,390 | 1,520 |
| 3 | 420 | 635 | 345 | 380 | 695 | 760 | 1,040 | 1,140 |
| 4 | - | - | 305 | 335 | 615 | 675 | 925 | 1,015 |
| (2) 3-1/2" | - | - | 820 | 860 | 1,640 | 1,720 | 2,465 | 2,580 |


| RIGIDLAM LVL EQUIVALENT SPECIFIC GRAVITY VALUES FOR FASTENER DESIGN |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Face |  |  | Edge |  |  |
|  | DF |  | SP | DF |  | SP |
|  | $\begin{gathered} \text { 1.4E \& } \\ \text { 1.6E LVL } \end{gathered}$ | $\begin{gathered} \text { 2.1E \& } \\ \text { 2.3E LVL } \end{gathered}$ | $\begin{gathered} \text { 1.6E \& } \\ \text { 2.1E LVL } \end{gathered}$ | $\begin{gathered} \text { 1.4E \& } \\ \text { 1.6E LVL } \end{gathered}$ | $\begin{gathered} \text { 2.1E \& } \\ \text { 2.3E LVL } \end{gathered}$ | $\begin{gathered} \text { 1.6E \& } \\ \text { 2.1E LVL } \end{gathered}$ |
| Withdrawal - nail | 0.50 | 0.50 | 0.50 | 0.47 | 0.50 | 0.43 |
| Dowel Bearing - nail | 0.50 | 0.50 | 0.55 | 0.50 | 0.50 | 0.49 |
| Dowel Bearing - bolt | 0.47 | 0.50 | 0.55 |  | ot applicab |  |

GRAVITY VALUES FOR FASTENER DESIGN

- Use appropriate software (e.g. Simpson Strong-Tie ${ }^{\circledR}$ Component Solutions ${ }^{\text {TM }}$ ) or beam/header Quick Reference Tables or PLF load tables to size the beam.
- The table values apply to common (A307) bolts. Bolt holes must be centered at least two inches from the top and bottom edges of the beam. Bolt holes must be the same diameter as the bolts. Washers must be used under the bolt heads and nuts.
Offset or stagger rows of bolt holes by one-half of the bolt spacing.
- The specified nailing applies to both sides of a three-ply beam.
- Seven inch wide beams may not be loaded from one side only. They must be loaded from both sides and/or top-loaded.
- The side loaded table values for nails may be doubled for 6 " o.c. spacing and tripled for 4" o.c. spacing.
- Duration of load factors (e.g. $115 \%, 125 \%$ etc...) may be applied to the table values.


## Fastening Recommendations For Multiple Ply LVL Members (ont)

- The wood screws listed are approved for use in connecting multiple plies of RigidLam ${ }^{\circledR}$ LVL together and may be used as an alternative to the nailing or bolting guidelines on the previous page.
- Pre-drilling of the LVL members is not required for the screws listed below.
- Carefully review and adhere to the design and


2 ply
 installation information available from each of the screw The diagrams above are for illustrative purposes only, screws may need to be applied to both sides. manufacturers listed below.

Refer to the manufacturers' information for the appropriate design and installation guidelines.

## Simpson SDW Wood Screws



| Model No. | L(in) | TL (in) | Head Stamp |
| :---: | :---: | :---: | :---: |
| SDW22338 | $3-3 / 8$ | $1-9 / 16$ | 3.37 |
| SDW22500 | 5 | $1-9 / 16$ | 5.00 |
| SDW22634 | $6-3 / 4$ | $1-9 / 16$ | 6.75 |

- Code Evaluation Report - IAPMO ER-0192
- For SDW design and installation information or hanger information, refer to the current Simpson Strong-Tie literature, www.strongtie.com or contact Simpson Strong-Tie at 800-999-5099.


## MiTek WSWH Washer Head Structural Wood Screws



| Model No. | L (in) | SH (in) | T (in) |
| :---: | :---: | :---: | :---: |
| WSWH338 | $3-3 / 8$ | $1-1 / 8$ | 2 |
| WSWH5 | 5 | $2-3 / 4$ | 2 |
| WSWH634 | $6-3 / 4$ | $4-1 / 2$ | 2 |

- Code Evaluation Report: ICC-ES ESR-2761
- For WSWH design and installation information or hanger information, refer to the current MiTek Structural Product Catalog, www.MiTek-us.com or contact MiTek at 800-328-5934


## FastenMaster FlatLOK"W Wood Screws



| Product | L (in) | TL (in) | Head Marking |
| :---: | :---: | :---: | :---: |
| FLL312 | $3-1 / 2$ | 2 | F3.5FL |
| FLOO5 | 5 | 2 | F5.0FL |
| FL634 | $6-3 / 4$ | 2 | F6.75FL |

- Code Evaluation Report - DrJ - TER 1501-08
- For FlatLOK design and installation information, refer to the current FastenMaster literature, www.fastenmaster.com or contact FastenMaster at 800-518-3569.


## LVL QUICK REFERENCE TABLES

## Floor Beams

Douglas-fir LVL and Southern Pine LVL

The tables below show the size of the beams needed to support various floor systems. The tables are valid for loads of one floor only, i.e., a second story floor or one story floor over a basement.
When floor joists span continuously from wall to wall (not cut at beam) these tables require that " B " be not less than $45 \%$, or greater than $55 \%$ of "A".


Tributary Area
(see page 40)

FLOOR BEAM - 1.6E RIGIDLAM LVL (40 PSF LL + 10 PSF DL)

| idth of | Beam Support Spacing (ft.) |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Building " A " (ft.) | 11 | 12 | 13 | 14 | 15 | 16 | 17 | 18 | 19 | 20 |
| 24 | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-14 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{gathered} 2-16 \text { * } \\ 3-14 \end{gathered}$ | $\begin{gathered} 2-18^{*} \\ 3-16 \end{gathered}$ | $\begin{gathered} 2-18 \\ 3-16 \end{gathered}$ | $\begin{gathered} 2-20 \text { * } \\ 3-18 \end{gathered}$ | $\begin{gathered} 2-20 \text { * } \\ 3-18 \end{gathered}$ |
| 28 | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-14 \\ & 3-14 \end{aligned}$ | $\begin{gathered} 2-16 \\ 3-14 \end{gathered}$ | $\begin{gathered} 2-16^{*} \\ 3-14 \end{gathered}$ | $\begin{gathered} 2-18 \text { * } \\ 3-16 \end{gathered}$ | $\begin{gathered} 2-18 \\ 3-16 \end{gathered}$ | $\begin{gathered} 2-20 \\ 3-18 \end{gathered}$ | 3-18 | 3-20 |
| 32 | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{gathered} 2-14^{*} \\ 3-11-7 / 8 \end{gathered}$ | $\begin{gathered} 2-16 \text { * } \\ 3-14 \end{gathered}$ | $\begin{gathered} 2-16 \text { * } \\ 3-14 \end{gathered}$ | $\begin{gathered} 2-18 \\ 3-16 \end{gathered}$ | $\begin{gathered} 2-18 \text { * } \\ 3-16 \end{gathered}$ | 3-18 | $3-18 \text { * }$ | $3-18 \text { * }$ | $3-20 \text { * }$ |
| 36 | $\begin{gathered} 2-14^{*} \\ 3-11-7 / 8 \end{gathered}$ | $\begin{gathered} 2-16 \\ 3-14 \end{gathered}$ | $\begin{gathered} 2-16 \text { * } \\ 3-14 \end{gathered}$ | $\begin{gathered} 2-18 \\ 3-14 \end{gathered}$ | $3-16$ | $3-16 \text { * }$ | $3-18 \text { * }$ | $3-18 \text { * }$ | $3-20 \text { * }$ | $3-20 \text { * }$ |
| 40 | $\begin{gathered} 2-16^{*} \\ 3-11-7 / 8 \end{gathered}$ | $\begin{gathered} 2-16 \text { * } \\ 3-14 \end{gathered}$ | $\begin{gathered} 2-18 * \\ 3-14 \end{gathered}$ | 3-16 | $3-16 \text { * }$ | $3-18 \text { * }$ | $3-18 \text { * }$ | $3-20 \text { * }$ | $3-20 \text { * }$ |  |
| 44 | $\begin{gathered} 2-16 \text { * } \\ 3-14 \end{gathered}$ | $\begin{gathered} 2-18 \text { * } \\ 3-14 \end{gathered}$ | $3-14 \text { * }$ | $3-16 \text { * }$ | $3-16 \text { * }$ | $3-18 \text { * }$ | $3-20 \text { * }$ | $3-20 \text { * }$ |  | - |

## FLOOR BEAM - 2.1E RIGIDLAM LVL (40 PSF LL + 10 PSF DL)

| Width of Building "A" (ft.) | Beam Support Spacing (ft.) |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 11 | 12 | 13 | 14 | 15 | 16 | 17 | 18 | 19 | 20 |
| 24 | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-18 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ |
| 28 | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-18 \\ & 3-14 \end{aligned}$ | $\begin{gathered} 2-18 \text { * } \\ 3-16 \end{gathered}$ | $\begin{gathered} 2-18^{*} \\ 3-16 \end{gathered}$ | $\begin{gathered} 2-20 \\ 3-18 \end{gathered}$ |
| 32 | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-14 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{gathered} 2-16 \text { * } \\ 3-14 \end{gathered}$ | $\begin{gathered} 2-18 \text { * } \\ 3-16 \end{gathered}$ | $\begin{gathered} 2-18 \text { * } \\ 3-16 \end{gathered}$ | $\begin{gathered} 2-20 \text { * } \\ 3-18 \end{gathered}$ | $\begin{gathered} 2-20 \text { * } \\ 3-18 \end{gathered}$ |
| 36 | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{gathered} 2-16 \\ 3-14 \end{gathered}$ | $\begin{gathered} 2-16 \\ 3-14 \end{gathered}$ | $\begin{gathered} 2-18 \\ 3-16 \end{gathered}$ | $\begin{gathered} 2-18 \\ 3-16 \end{gathered}$ | $\begin{gathered} 2-20 \\ 3-18 \end{gathered}$ | $\begin{gathered} 2-20 \\ 3-18 \end{gathered}$ | $3-18$ |
| 40 | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{gathered} 2-14 \text { * } \\ 3-14 \end{gathered}$ | $\begin{gathered} 2-16 \text { * } \\ 3-14 \end{gathered}$ | $\begin{gathered} 2-18 \text { * } \\ 3-14 \end{gathered}$ | $\begin{gathered} 2-18 \text { * } \\ 3-16 \end{gathered}$ | $\begin{gathered} 2-20 \text { * } \\ 3-16 \end{gathered}$ | $3-18$ | $3-18 \text { * }$ | $3-20 \text { * }$ |
| 44 | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{gathered} 2-14^{*} \\ 3-11-7 / 8 \end{gathered}$ | $\begin{gathered} 2-16 \text { * } \\ 3-14 \end{gathered}$ | $\begin{gathered} 2-16 \text { * } \\ 3-14 \end{gathered}$ | $\begin{gathered} 2-18 \text { * } \\ 3-16 \end{gathered}$ | $3-16$ | $3-18 \text { * }$ | $3-18 \text { * }$ | $3-20 \text { * }$ | $3-20 \text { * }$ |

* see note 3


## Notes:

1. Beam sizes are listed as the number of $1-3 / 4$ " wide plies by the LVL depth (e.g. 2-9-1/2" indicates two 1-3/4" plies by 9-1/2" deep).
2. Beam sizes are based on continuous floor joist spans and simple or continuous beam spans. If the floor joists are not continuous, it is permissible to consider a Total Floor Joist Span "A" that is equal to 0.8 times the actual " $A$ " dimension.
3. The minimum required end and intermediate bearing lengths (based on 575 PSI for 1.6E LVL and 750 PSI for 2.1 E LVL) are 3 " and $7-1 / 2^{\prime \prime}$ respectively unless the * symbol is shown. In that case, $4-1 / 2^{\prime \prime}$ and 10-1/2" end and intermediate bearing lengths are required.
4. All beams require support across their full width.
5. Beam sizes are based on residential floor loading of 40 PSF live load and 10 PSF dead load. The roof framing must be trusses supported at the exterior walls only
6. Deflection is limited to $\mathrm{L} / 360$ at live load and $\mathrm{L} / 240$ at total load.
7. Allowable loads shown for multiple ply LVL members are also applicable to factory glued LVL beams with the same thickness as the combined multiple plies.
8. The beam sizes shown are based on the controlling size for RigidLam LVL produced from Douglas-fir or Southern Pine veneer and therefore can be used for either species. Quick Reference tables separated by species are available on the Roseburg website.

## 1-Story Garage Door Headers <br> Douglas-fir LVL and Southern Pine LVL

The tables indicate the appropriate size header for various roof truss spans with 2 ' soffit. If the soffit is greater than 2 ', additional engineering is necessary.


## 1 STORY - 1.6E RIGIDLAM® LVL

| Roof Loading |  | Snow-115\% Load Duration |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 25 PSF LL + 20 PSF DL |  |  | 30 PSF LL + 20 PSF DL |  |  | 40 PSF LL + 20 PSF DL |  |  |
| Rough Opening (ft.) |  | 9'-3" | 16'-3" | 18'-3" | 9'-3" | 16'-3" | 18'-3" | 9'-3" | 16'-3" | 18'-3" |
| Roof Truss Span with 2' Soffit Assumed | 20 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \\ & \hline \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \\ & \hline \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-18 \\ & 3-14 \end{aligned}$ |
|  | 4 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 2-14 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \\ & \hline \end{aligned}$ | $\begin{aligned} & 2-18 \\ & 2-14 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \\ & \hline \end{aligned}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ |
|  | 28 | $2-9-1 / 2$ | 2-16 | 2-18 | $2-9-1 / 2$ | 2-16 | 2-18 | $2-9-1 / 2$ | 2-18 | $2-20 \text { * }$ |
|  | 32 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $2-16$ | $2-18$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | 2-18 | $2-20$ | $2-11-7 / 8$ | $2-20^{*}$ | $2-22 \text { * }$ |
|  | 36 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-18 \\ & 2-14 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & \text { 2-20 } \\ & 3-16 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-18 \\ & 3-14 \end{aligned}$ | $\begin{gathered} 2-20^{*} \\ 3-16 \end{gathered}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-20^{*} \\ 3-16 \end{gathered}$ | $\begin{gathered} 2-22^{*} \\ 3-18 \end{gathered}$ |
| Roof Loading |  | Non-Snow - 125\% Load Duration |  |  |  |  |  |  |  |  |
|  |  | 20 PSF LL + 15 PSF DL |  |  | 20 PSF LL + 20 PSF DL |  |  | 20 PSF LL + 25 PSF DL |  |  |
| Rough Opening (ft.) |  | 9'-3' | 16'-3" | 18'-3" | 9'-3' | 16'-3" | 18'-3" | 9'-3" | 16'-3" | 18'-3" |
| Roof Truss Span with 2' Soffit Assumed | 20 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ |
|  | 24 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ |
|  | 28 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-14 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ |
|  | 32 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-14 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ |
|  | 36 | $2-9-1 / 2$ | $2-14$ | 2-16 | $2-9-1 / 2$ | 2-16 | 2-18 | $2-9-1 / 2$ | 2-16 | 2-18 |

1 STORY - 2.1E RIGIDLAM® LVL

| Roof Loading |  | Snow - 115\% Load Duration |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 25 PSF LL + 20 PSF DL |  |  | 30 PSF LL + 20 PSF DL |  |  | 40 PSF LL + 20 PSF DL |  |  |
| Rough Opening (ft.) |  | 9'-3" | 16'-3" | 18'-3" | 9'-3" | 16'-3" | 18'-3" | 9'-3" | 16'-3" | 18'-3" |
| Roof Truss Span with 2' Soffit Assumed | 20 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \\ & \hline \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \\ & \hline \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ |
|  | 24 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \\ \hline \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-14 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \\ \hline \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ |
|  | 28 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \\ \hline \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-18 \\ & 3-14 \end{aligned}$ |
|  | 32 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \\ \hline \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \\ & \hline \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-18 \\ & 3-16 \\ & \hline \end{aligned}$ |
|  | 36 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-18 \\ & 3-14 \\ & \hline \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-18 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-20 \\ & 3-16 \end{aligned}$ |
| Roof Loading |  | Non-Snow - 125\% Load Duration |  |  |  |  |  |  |  |  |
|  |  | 20 PSF LL + 15 PSF DL |  |  | 20 PSF LL + 20 PSF DL |  |  | 20 PSF LL + 25 PSF DL |  |  |
| Rough Opening (ft.) |  | 9'-3" | 16'-3" | 18'-3" | 9'-3' | 16'-3" | 18'-3" | 9'-3" | 16'-3" | 18'-3" |
| Roof Truss Span with 2' Soffit Assumed | 20 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ |
|  | 24 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ |
|  | 28 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \\ & \hline \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \\ \hline \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ |
|  | 32 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \\ & \hline \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \\ & \hline \end{aligned}$ |
|  | 36 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-14 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ |

* see note 2


## Notes:

1. Header sizes are listed as the number of $1-3 / 4$ " wide plies by the LVL depth (e.g. 2-9-1/2" indicates two 1-3/4" plies by 9-1/2" deep)
2. The minimum required end bearing length (based on 575 PSI for 1.6 ELVL and 750 PS for 2.1 E LVL ) is $4-1 / 2^{\prime \prime}$ unless the *symbol is shown. In that case, 6 " is required.
3. All headers require support across their full width. Use $2 \times 4$ cripples for two-ply headers and $2 \times 6$ cripples for three-ply headers.
4. The roof framing is assumed to be trusses supported by the exterior walls only.
5. Deflection is limited to $L / 240$ at live load and $L / 180$ at total load.
6. Allowable loads shown for multiple ply LVL members are also applicable to single billet LVL members with the same width as the combined multiple plies.
7. The header sizes shown are based on the controlling size for RigidLam LVL produced from Douglas-fir or Southern Pine veneer and therefore can be used for either species Quick Reference tables separated by species are available on the Roseburg website.

## 2-Story Garage Door Headers <br> Douglas-fir LVL and Southern Pine LVL

The tables consider the combined loads from a wall, second story floor ( $1 / 4$ of total floor joist span) and various roof truss spans with a 2 ' soffit. Intermediate floor beam assumed. If the soffit exceeds 2 ', additional engineering will be necessary.

## 2 STORY - 1.6E RIGIDLAM® LVL

| Roof Loading |  | Snow-115\% Load Duration |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 25 PSF LL + 20 PSF DL |  |  | 30 PSF LL + 20 PSF DL |  |  | 40 PSF LL + 20 PSF DL |  |  |
| Rough Opening (ft.) |  | 9'-3" | 16'-3" | 18'-3" | 9'-3" | 16'-3" | 18'-3" | 9'-3" | 16'-3" | 18'-3" |
| Roof Truss Span with 2' Soffit Assumed | 20 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ | $\begin{aligned} & 2-20 \\ & 3-16 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ | $\begin{aligned} & 2-20 \\ & 3-18 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ | $\begin{gathered} 2-22 \text { * } \\ 3-18 \end{gathered}$ |
|  | 24 | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ | $\begin{gathered} 2-20 \\ 3-18 \end{gathered}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-20^{*} \\ 3-16 \end{gathered}$ | $\begin{gathered} 2-22 \\ 3-18 \end{gathered}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-20^{*} \\ 3-16 \end{gathered}$ | $\begin{gathered} 2-22^{*} \\ 3-18 \end{gathered}$ |
|  | 28 | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-20 \text { * } \\ 3-16 \end{gathered}$ | $\begin{gathered} 2-22^{*} \\ 3-18 \end{gathered}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-20^{*} \\ 3-16 \end{gathered}$ | $\begin{gathered} 2-24 \\ 3-18 \end{gathered}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{gathered} 2-22^{*} \\ 3-18 \end{gathered}$ | $3-20$ |
|  | 32 | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-20^{*} \\ 3-18 \end{gathered}$ | $3-20$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{gathered} 2-22^{*} \\ 3-18 \end{gathered}$ | $3-20$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $3-18$ | $3-22 \text { * }$ |
|  | 36 | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{gathered} 2-22 \text { * } \\ 3-18 \end{gathered}$ | $3-20 \text { * }$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | 3-18 | $3-20 \text { * }$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $3-20 \text { * }$ | $3-22 \text { * }$ |
| Roof Loading |  | Non-Snow - 125\% Load Duration |  |  |  |  |  |  |  |  |
|  |  | 20 PSF LL + 15 PSF DL |  |  | 20 PSF LL + 20 PSF DL |  |  | 20 PSF LL + 25 PSF DL |  |  |
| Rough Opening (ft.) |  | 9'-3" | 16'-3" | 18'-3" | 9'-3" | 16'-3" | 18'-3" | 9'-3" | 16'-3" | 18'-3" |
| Roof Truss Span with 2' Soffit Assumed | 20 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \\ & \hline \end{aligned}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-18 \\ & 3-16 \\ & \hline \end{aligned}$ | $\begin{aligned} & 2-20 \\ & 3-16 \end{aligned}$ |
|  | 24 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ | $\begin{aligned} & 2-20 \\ & 3-16 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ | $\begin{aligned} & 2-20 \\ & 3-18 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ | $\begin{gathered} 2-20^{*} \\ 3-18 \end{gathered}$ |
|  | 28 | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ | $\begin{gathered} 2-20 \\ 3-18 \end{gathered}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ | $\begin{gathered} 2-20 \text { * } \\ 3-18 \end{gathered}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-18^{*} \\ 3-16 \end{gathered}$ | $\begin{gathered} 2-22 \\ 3-18 \end{gathered}$ |
|  | 32 | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-18^{*} \\ 3-16 \end{gathered}$ | $\begin{gathered} 2-20^{*} \\ 3-18 \end{gathered}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-20^{*} \\ 3-16 \end{gathered}$ | $\begin{gathered} 2-22 \\ 3-18 \end{gathered}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-20 \\ 3-18 \end{gathered}$ | $3-20$ |
|  | 36 | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-20 \text { * } \\ 3-16 \end{gathered}$ | $\begin{gathered} 2-22^{*} \\ 3-18 \end{gathered}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-20^{*} \\ 3-18 \end{gathered}$ | $3-20$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{gathered} 2-22^{*} \\ 3-18 \end{gathered}$ | $3-20 \text { * }$ |

## 2 STORY - 2.1E RIGIDLAM® LVL

| Roof Loading |  | Snow - 115\% Load Duration |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 25 PSF LL + 20 PSF DL |  |  | 30 PSF LL + 20 PSF DL |  |  | 40 PSF LL + 20 PSF DL |  |  |
| Rough Opening (ft.) |  | 9'-3" | 16'-3" | 18'-3" | 9'-3' | 16'-3" | 18'-3" | 9'-3" | 16'-3" | 18'-3" |
| Roof Truss Span with 2' Soffit Assumed | 20 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \\ & \hline \end{aligned}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \\ & \hline \end{aligned}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ |
|  | 24 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-18 \\ & 3-16 \\ & \hline \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-18 \\ & 3-16 \\ & \hline \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-18 \\ & 3-16 \\ & \hline \end{aligned}$ | $\begin{aligned} & 2-20 \\ & 3-18 \end{aligned}$ |
|  | 28 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ | $\begin{aligned} & 2-20 \\ & 3-16 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ | $\begin{aligned} & 2-20 \\ & 3-18 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ | $\begin{gathered} 2-20 \text { * } \\ 3-18 \end{gathered}$ |
|  | 32 | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-18 \\ & 3-16 \\ & \hline \end{aligned}$ | $\begin{gathered} 2-20^{*} \\ 3-18 \end{gathered}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ | $\begin{gathered} 2-20^{*} \\ 3-18 \end{gathered}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-20^{*} \\ 3-16 \end{gathered}$ | $\begin{gathered} 2-22 \\ 3-18 \end{gathered}$ |
|  | 36 | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-18 \\ 3-16 \end{gathered}$ | $\begin{gathered} 2-22^{*} \\ 3-18 \end{gathered}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-20^{*} \\ 3-16 \end{gathered}$ | $\begin{gathered} 2-22 \\ 3-18 \end{gathered}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-20^{*} \\ 3-18 \end{gathered}$ | 3-20 |
| Roof Loading |  | Non-Snow-125\% Load Duration |  |  |  |  |  |  |  |  |
|  |  | 20 PSF LL + 15 PSF DL |  |  | 20 PSF LL + 20 PSF DL |  |  | 20 PSF LL + 25 PSF DL |  |  |
| Rough Opening (ft.) |  | 9'-3" | 16'-3" | 18'-3" | 9'-3" | 16'-3" | 18'-3" | 9'-3' | 16'-3" | 18'-3" |
| Roof Truss Span with 2' Soffit Assumed | 20 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \\ & \hline \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \\ & \hline \end{aligned}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ |
|  | 24 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \\ & \hline \end{aligned}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{array}{r} 2-18 \\ 3-16 \\ \hline \end{array}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \\ & \hline \end{aligned}$ | $\begin{aligned} & 2-18 \\ & 3-16 \\ & \hline \end{aligned}$ |
|  | 28 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ | $\begin{aligned} & 2-20 \\ & 3-16 \end{aligned}$ |
|  | 32 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-18 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-20 \\ & 3-16 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \\ \hline \end{gathered}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ | $\begin{aligned} & 2-20 \\ & 3-18 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ | $\begin{gathered} 2-20^{*} \\ 3-18 \end{gathered}$ |
|  | 36 | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ | $\begin{aligned} & 2-20 \\ & 3-18 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ | $\begin{gathered} 2-20^{*} \\ 3-18 \end{gathered}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-18 * \\ 3-16 \end{gathered}$ | $\begin{gathered} 2-20 \\ 3-18 \end{gathered}$ |

## * see note 3

## Notes:

1. Header sizes are listed as the number of $1-3 / 4$ " wide plies by the LVL depth (e.g. 2 - 9-1/2" indicates two 1-3/4" plies by 9-1/2" deep).
2. Header sizes are based on the assumption that the floor joists are supported in the middle of the building by a beam or wall.
3. The minimum required end bearing length (based on 575 PSI for 1.6 ELVL and 750 PSI for 2.1 ELVL ) is 4-1/2" unless the * symbol is shown. In that case, 6 " is required.
4. All headers require support across their full width. Use $2 \times 4$ cripples for two-ply headers and $2 \times 6$ cripples for three-ply headers.
5. Header sizes are based on residential floor loading of 40 PSF live load, 10 PSF dead load and 80 PLF wall dead load. The roof framing must be trusses supported by the exterior walls only.
erer wing
6. Deflection is limited to $\mathrm{L} / 360$ at live load and $\mathrm{L} / 240$ at total load.
7. Allowable loads shown for multiple ply LVL members are also applicable to single billet LVL members with the same width as the combined multiple plies.
8. The header sizes shown are based on the controlling size for RigidLam LVL produced from Douglas-fir or Southern Pine veneer and therefore can be used for either species Quick Reference tables separated by species are available on the Roseburg website.

## 1-Story Window \& Door Headers <br> Douglas-fir LVL and Southern Pine LVL

The tables indicate the appropriate size header for various roof truss spans with 2 ' soffit. If the soffit is greater than 2', additional engineering is necessary.

Tributary Area
(see page 40)


1 STORY - 1.6E RIGIDLAM® LVL

| Roof Loading |  | Snow - 115\% Load Duration |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 25 PSF LL + 20 PSF DL |  |  |  |  | 40 PSF LL + 20 PSF DL |  |  |  |  |
| Rough Opening (ft.) |  | 6'-0" | 8'-0" | 9'-0" | 10'-0" | 12'-0" | 6'-0" | 8'-0" | 9'-0" | 10'-0" | 12'-0" |
| Roof Truss <br> Span with <br> 2' Soffit <br> Assumed | 20 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ |
|  | 24 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-14 \\ & 3-14 \\ & \hline \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ |
|  | 28 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \\ & \hline \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-18 \\ & 3-14 \end{aligned}$ |
|  | 32 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ |
|  | 36 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ |
| Roof Loading |  | Non-Snow - 125\% Load Duration |  |  |  |  |  |  |  |  |  |
|  |  | 20 PSF LL + 15 PSF DL |  |  |  |  | 20 PSF LL + 25 PSF DL |  |  |  |  |
| Rough Opening (ft.) |  | 6'-0" | 8'-0" | 9'-0" | 10'-0" | 12'-0" | 6'-0" | 8'-0" | 9'-0" | 10'-0" | 12'-0" |
| Roof Truss Span with 2' Soffit Assumed | 20 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ |
|  | 24 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-14 \\ & 3-14 \end{aligned}$ |
|  | 28 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ |
|  | 32 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-14 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ |
|  | 36 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ |


| Roof Loading |  | Snow - 115\% Load Duration |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 25 PSF LL + 20 PSF DL |  |  |  |  | 40 PSF LL + 20 PSF DL |  |  |  |  |
| Rough Opening (ft.) |  | 6'-0" | 8'-0" | 9'-0" | 10'-0" | 12'-0" | 6'-0" | 8'-0" | 9'-0' | 10'-0" | 12'-0" |
| Roof Truss Span with 2' Soffit Assumed | 20 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ |
|  | 24 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-14 \\ & 3-14 \end{aligned}$ |
|  | 28 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ |
|  | 32 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ |
|  | 36 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ |
| Roof Loading |  | Non-Snow - 125\% Load Duration |  |  |  |  |  |  |  |  |  |


| Roof Loading |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 20 PSF LL + 15 PSF DL |  |  |  |  | 20 PSF LL + 25 PSF DL |  |  |  |  |
| Rough Opening (ft.) |  | 6'-0" | 8'-0" | 9'-0" | 10'-0" | 12'-0" | 6'-0" | 8'-0" | 9'-0" | 10'-0" | 12'-0" |
| Roof Truss Span with 2' Soffit Assumed | 20 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ |
|  | 24 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ |
|  | 28 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ |
|  | 32 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ |
|  | 36 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ |

* see note 2

Notes:

1. Header sizes are listed as the number of 1-3/4" thick plies by the header depth (e.g. 2-9-1/2" indicates two 1-3/4" plies by 9-1/2" deep).
2. The minimum required bearing length (based on 575 PSI for 1.6 LVL and 750 PSI for 2.1E LVL) is $4-1 / 2$ " unless the * symbol is shown. In that case, 6 " is required.
3. All headers require support across their full width. Use $2 \times 4$ cripples for two ply headers and $2 \times 6$ cripples for three ply headers.
4. The roof framing is assumed to be trusses supported by the exterior walls only.
5. Deflection is limited to $L / 240$ at live load and the lesser of $L / 180$ or $5 / 16$ " at total load.
6. Allowable loads shown for multiple ply LVL members are also applicable to single billet LVL members with the same width as the combined multiple plies.
7. The header sizes shown are based on the controlling size for RigidLam LVL produced from Douglas-fir or Southern Pine veneer and therefore can be used for either species. Quick Reference tables separated by species are available on the Roseburg website.

## LVL QUICK REFERENCE TABLES

## 2-Story Window \&: Door Headers

Douglas-fir LVL and Southern Pine LVL

The tables consider the combined loads from a wall, second story floor ( $1 / 4$ of total floor joist span) and various roof truss spans with a 2' soffit. Intermediate floor beam assumed. If the soffit exceeds $2^{2}$, additional engineering will be necessary.

Tributary Area
(see page 40)

2 STORY - 1.6E RIGIDLAM® LVL

| Roof Loading |  | Snow - 115\% Load Duration |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 25 PSF LL + 20 PSF DL |  |  |  |  | 40 PSF LL + 20 PSF DL |  |  |  |  |
| Rough Opening (ft.) |  | 6'-0" | 8'-0" | 9'-0" | 10'-0" | 12'-0" | 6'-0" | 8'-0" | 9'-0" | 10'-0" | 12'-0" |
| Roof Truss <br> Span with 2' Soffit Assumed | 20 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ |
|  | 24 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-14 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ |
|  | 28 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 2-14 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-20 \\ & 3-16 \end{aligned}$ |
|  | 32 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \\ & \hline \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{gathered} 2-20^{*} \\ 3-18 \end{gathered}$ |
|  | 36 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-20 \\ & 3-18 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{gathered} 2-16^{*} \\ 3-14 \end{gathered}$ | $\begin{gathered} 2-22 \\ 3-18 \end{gathered}$ |
| Roof Loading |  | Non-Snow-125\% Load Duration |  |  |  |  |  |  |  |  |  |
|  |  | 20 PSF LL + 15 PSF DL |  |  |  |  | 20 PSF LL + 25 PSF DL |  |  |  |  |
| Rough Opening (ft.) |  | 6'-0" | 8'-0" | 9'-0" | 10'-0" | 12'-0" | 6'-0" | 8'-0" | 9'-0" | 10'-0" | 12'-0" |
| Roof Truss <br> Span with 2' Soffit Assumed | 20 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ |
|  | 24 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ |
|  | 28 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ |
|  | 32 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ |
|  | 36 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-14 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-20 \\ & 3-18 \end{aligned}$ |


| Roof Loading |  | Snow-115\% Load Duration |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 25 PSF LL + 20 PSF DL |  |  |  |  | 40 PSF LL + 20 PSF DL |  |  |  |  |
| Rough Opening (ft.) |  | 6'-0" | 8'-0" | 9'-0" | 10'-0" | 12'-0" | 6'-0" | 8'-0" | 9'-0" | 10'-0" | 12'-0" |
| Roof Truss <br> Span with 2' Soffit Assumed | 20 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \\ & \hline \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \\ \hline \end{gathered}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ |
|  | 24 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \\ & \hline \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \\ & \hline \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ |
|  | 28 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-111 / 7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ |
|  | 32 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-18 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ |
|  | 36 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-18 \\ & 3-16 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-20 \\ & 3-16 \end{aligned}$ |
| Nonding Non-Snow-125\% Load Duration |  |  |  |  |  |  |  |  |  |  |  |
| Roof Loading |  | 20 PSF LL + 15 PSF DL |  |  |  |  | 20 PSF LL + 25 PSF DL |  |  |  |  |
| Rough Opening (ft.) |  | 6'-0" | 8'-0" | 9'-0" | 10'-0" | 12'-0" | 6'-0" | 8'-0" | 9'-0" | 10'-0" | 12'-0" |
| Roof Truss <br> Span with <br> 2' Soffit <br> Assumed | 20 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ |
|  | 24 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ |
|  | 28 | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11 / 7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{aligned} & 2-11-7 / 8 \\ & 3-11-7 / 8 \end{aligned}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{aligned} & 2-9-1 / 2 \\ & 3-9-1 / 2 \end{aligned}$ | $\begin{gathered} 2-11-7 / 8 \\ 3-9-1 / 2 \end{gathered}$ | $\begin{gathered} 2-14 \\ 3-11-7 / 8 \end{gathered}$ | $\begin{aligned} & 2-16 \\ & 3-14 \end{aligned}$ |
|  |  | 2-9-1/2 | 2-9-1/2 | 2-11-7/8 | 2-14 | 2-16 | 2-9-1/2 | 2-9-1/2 | 2-11-7/8 | 2-14 | 2-18 |
|  | 32 | 3-9-1/2 | 3-9-1/2 | 3-9-1/2 | 3-11-7/8 | 3-14 | 3-9-1/2 | 3-9-1/2 | 3-11-7/8 | 3-11-7/8 | 3-14 |
|  | 36 | 2-9-1/2 | 2-9-1/2 | 2-11-7/8 | 2-14 | 2-16 | 2-9-1/2 | 2-11-7/8 | 2-11-7/8 | 2-14 | 2-18 |
|  | 36 | 3-9-1/2 | 3-9-1/2 | 3-9-1/2 | 3-11-7/8 | 3-14 | 3-9-1/2 | 3-9-1/2 | 3-11-7/8 | 3-11-7/8 | 3-16 |

## * see note 3

## Notes:

1. Header sizes are listed as the number of 1-3/4" thick plies by the header depth (e.g. 2-9-1/2" indicates two 1-3/4" plies by 9-1/2" deep).
2. Header sizes are based on the assumption that the floor joists are supported in the middle of the building by a beam or wall.
3. The minimum required end bearing length (based on 575 PSI for 1.6 E LVL and 750 PSI for 2.1 ELVL ) is $4-1 / 2$ " unless the * symbol is shown. In that case, 6 " is required.
4. All headers require support across their full width. Use $2 \times 4$ cripples for two-ply headers and $2 \times 6$ cripples for three-ply headers.
5. Header sizes are based on residential floor loading of 40 PSF live load, 10 PSF dead load and 80 PLF wall dead load. The roof framing must be trusses supported by the exterior walls only.
6. Deflection is limited to $L / 360$ at live load and the lesser of $L / 240$ or $5 / 16$ " at total load.
7. Allowable loads shown for multiple ply LVL members are also applicable to single billet LVL members with the same width as the combined multiple plies.
8. The header sizes shown are based on the controlling size for RigidLam LVL produced from Douglas-fir or Southern Pine veneer and therefore can be used for either species. Quick Reference tables separated by species are available on the Roseburg website.

## PLF Tables Douglas－fir LLL and Southem Pine LVL

## 1－PLY 1－3／4＂2．1E RIGIDLAM® LVL－FLOOR（PLF）100\％LOAD DURATION

| Span（ft．） | Depth | 4－3／8＂ | 5－1／2＂ | 7－1／4＂ | 9－1／4＂ | 9－1／2＂ | 11－1／4＂ | 11－7／8＂ | 14＂ | 16＂ | 18＂ | 20＂ | 22＂ | 24＂ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 6 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ | $\begin{gathered} 166 \\ 247 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 321 \\ 478 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 693 \\ 776 \\ 1.8 / 4.5 \end{gathered}$ | $\begin{aligned} & 1,046 \\ & 2.4 / 6 \\ & \hline \end{aligned}$ | $\begin{gathered} 1,082 \\ 2.5 / 6.2 \end{gathered}$ | $\begin{gathered} 1,348 \\ 3.1 / 7.7 \end{gathered}$ | $\begin{gathered} 1,449 \\ 3.3 / 8.3 \end{gathered}$ | $\begin{gathered} 1,826 \\ 4.2 / 10.5 \end{gathered}$ |  |  |  |  |  |
| 8 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ | $\begin{gathered} 72 \\ 106 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 140 \\ 208 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 310 \\ 462 \\ 1.5 / 3.5 \end{gathered}$ | $\begin{gathered} 614 \\ 735 \\ 2.3 / 5.6 \end{gathered}$ | $\begin{gathered} 660 \\ 759 \\ 2.3 / 5.8 \end{gathered}$ | $\begin{gathered} 931 \\ 2.9 / 7.1 \end{gathered}$ | $\begin{gathered} 996 \\ 3.1 / 7.6 \end{gathered}$ | $\begin{gathered} 1,229 \\ 3.8 / 9.4 \end{gathered}$ |  |  |  |  |  |
| 10 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ | $\begin{gathered} 37 \\ 54 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 73 \\ 107 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 164 \\ 242 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 329 \\ 489 \\ 1.9 / 4.7 \end{gathered}$ | $\begin{gathered} 355 \\ 527 \\ 2 / 5.1 \end{gathered}$ | $\begin{gathered} 569 \\ 711 \\ 2.7 / 6.8 \end{gathered}$ | $\begin{gathered} 660 \\ 758 \\ 2.9 / 7.3 \end{gathered}$ | $\begin{gathered} 925 \\ 3.5 / 8.9 \end{gathered}$ |  |  |  |  |  |
| 12 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ |  | $\begin{gathered} 43 \\ 61 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 96 \\ 141 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 195 \\ 288 \\ 1.5 / 3.4 \end{gathered}$ | $\begin{gathered} 211 \\ 312 \\ 1.5 / 3.6 \end{gathered}$ | $\begin{gathered} 342 \\ 507 \\ 2.3 / 5.9 \end{gathered}$ | $\begin{gathered} 398 \\ 585 \\ 2.7 / 6.8 \end{gathered}$ | $\begin{gathered} 629 \\ 741 \\ 3.4 / 8.5 \end{gathered}$ |  |  |  |  |  |
| 14 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ |  |  | $\begin{gathered} 61 \\ 88 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 125 \\ 183 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 135 \\ 198 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 220 \\ 325 \\ 1.8 / 4.4 \end{gathered}$ | $\begin{gathered} 257 \\ 380 \\ 2.1 / 5.1 \end{gathered}$ | $\begin{aligned} & 410 \\ & 577 \\ & 3.1 / 7.8 \end{aligned}$ |  |  |  |  |  |
| 16 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ |  |  | $\begin{gathered} 41 \\ 58 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 85 \\ 122 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 92 \\ 132 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 150 \\ 219 \\ 1.5 / 3.4 \end{gathered}$ | $\begin{gathered} 175 \\ 257 \\ 1.6 / 4 \end{gathered}$ | $\begin{gathered} 281 \\ 414 \\ 2.6 / 6.4 \end{gathered}$ |  |  |  |  |  |
| 18 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ |  |  |  | $\begin{gathered} 60 \\ 85 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 65 \\ 92 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 106 \\ 154 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 124 \\ 181 \\ 1.5 / 3.2 \end{gathered}$ | $\begin{gathered} 201 \\ 294 \\ 2.1 / 5.2 \end{gathered}$ |  |  |  |  |  |
| 20 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ |  |  |  | $\begin{gathered} 44 \\ 61 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 47 \\ 66 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 78 \\ 111 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 92 \\ 131 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 148 \\ 215 \\ 1.7 / 4.2 \end{gathered}$ |  |  |  |  |  |
| 22 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ |  |  |  |  | $\begin{aligned} & 36 \\ & 49 \end{aligned}$ | $\begin{gathered} 59 \\ 83 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 69 \\ 98 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 112 \\ 161 \\ 1.5 / 3.5 \end{gathered}$ |  |  |  |  |  |
| 24 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ |  |  |  |  |  | $\begin{gathered} 46 \\ 63 \\ 1.5 / 3 \\ \hline \end{gathered}$ | $\begin{gathered} 54 \\ 74 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 87 \\ 123 \\ 1.5 / 3 \end{gathered}$ |  |  |  |  |  |
| 26 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ |  |  |  |  |  |  | $\begin{gathered} 42 \\ 57 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 69 \\ 96 \\ 1.5 / 3 \end{gathered}$ |  |  |  |  |  |
| 28 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ |  |  |  |  |  |  |  | $\begin{gathered} 55 \\ 76 \\ 1.5 / 3 \end{gathered}$ |  |  |  |  |  |
| 30 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ |  |  |  |  |  |  |  | $\begin{gathered} 45 \\ 60 \\ 1.5 / 3 \end{gathered}$ |  |  |  |  |  |

2－PLY 1－3／4＂2．1E RIGIDLAM® LVL－FLOOR（PLF）100\％LOAD DURATION

| Span（ft．） | Depth | 4－3／8＂ | 5－1／2＂ | 7－1／4＂ | 9－1／4＂ | 9－1／2＂ | 11－1／4＂ | 11－7／8＂ | 14＂ | 16＂ | 18＂ | 20＂ | 22＂ | 24＂ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 6 | LL | 333 | 641 | 1，385 |  |  |  |  |  |  |  |  |  |  |
|  | TL | 495 | 957 | 1，552 | 2，092 | 2，163 | 2，695 | 2，899 | 3，653 | 4，464 | 5，395 | 6，476 | 7，744 | 9，256 |
|  | BRG | 1.5 ／ 3 | 1.5 ／ 3 | 1.8 ／ 4.5 | 2.4 ／ 6 | 2.5 ／ 6.2 | $3.1 / 7.7$ | 3.3 ／ 8.3 | $4.2 / 10.5$ | $5.1 / 12.8$ | $6.2 / 15.5$ | 7.4 ／ 18.6 | 8.9 ／ 22.2 | 10.6 ／ 26.5 |
| 8 | LL | 144 | 281 | 621 | 1，228 | 1，321 |  |  |  |  |  |  |  |  |
|  | TL | 211 | 415 | 924 | 1，471 | 1，518 | 1，862 | 1，992 | 2，457 | 2，936 | 3，462 | 4，040 | 4，679 | 5，389 |
|  | BRG | $1.5 / 3$ | $1.5 / 3$ | 1.5 ／ 3.5 | $2.3 / 5.6$ | $2.3 / 5.8$ | 2．9／7．1 | $3.1 / 7.6$ | $3.8 / 9.4$ | 4.5 ／ 11.2 | $5.3 / 13.3$ | 6.2 ／ 15.5 | $7.2 / 17.9$ | $8.2 / 20.6$ |
| 10 | LL | 74 | 146 | 327 | 658 | 710 | 1，138 | 1，321 |  |  |  |  |  |  |
|  | TL | 107 | 214 | 484 | 978 | 1，055 | 1，421 | 1，515 | 1，849 | 2，186 | 2，546 | 2，932 | 3，348 | 3，797 |
|  | BRG | 1.5 ／ 3 | $1.5 / 3$ | 1.5 ／ 3 | 1.9 ／ 4.7 | 2 ／ 5.1 | 2.7 ／ 6.8 | $2.9 / 7.3$ | 3.5 ／ 8.9 | 4.2 ／ 10.5 | 4.9 ／ 12.2 | 5.6 ／ 14.1 | 6.4 ／ 16.1 | $7.3 / 18.2$ |
| 12 | LL | 43 | 85 | 193 | 391 | 422 | 684 | 796 | 1，258 |  |  |  |  |  |
|  | TL | 61 | 123 | 282 | 577 | 623 | 1，014 | 1，171 | 1，481 | 1，739 | 2，012 | 2，300 | 2，605 | 2，928 |
|  | BRG | 1.5 ／ 3 | 1.5 ／ 3 | 1.5 ／ 3 | 1.5 ／ 3.4 | 1.5 ／ 3.6 | $2.3 / 5.9$ | 2.7 ／ 6.8 | 3.4 ／ 8.5 | 44，661 | $4.6 / 11.6$ | $5.3 / 13.3$ | 44，727 | $6.7 / 16.9$ |
| 14 | LL |  | 54 | 123 | 250 | 270 | 441 | 514 | 820 | 1，189 | 1，639 |  |  |  |
|  | TL |  | 76 | 176 | 366 | 396 | 649 | 759 | 1，155 | 1，443 | 1，662 | 1，890 | 2，130 | 2，381 |
|  | BRG |  | 1.5 ／ 3 | 1.5 ／ 3 | 1.5 ／ 3 | 1.5 ／ 3 | 1.8 ／ 4.4 | 2.1 ／ 5.1 | 3.1 ／ 7.8 | 3.9 ／ 9.7 | 4.5 ／ 11.2 | $5.1 / 12.7$ | 5.7 ／ 14.3 | 6.4 ／ 16 |
| 16 | LL |  | 36 | 83 | 169 | 183 | 300 | 350 | 562 | 820 | 1，138 | 1，519 |  |  |
|  | TL |  | 49 | 117 | 245 | 265 | 438 | 513 | 829 | 1，122 | 1，389 | 1，604 | 1，800 | 2，006 |
|  | BRG |  |  | 1.5 ／ 3 | 1.5 ／ 3 | 1.5 ／ 3 | 1.5 ／ 3.4 | 1.6 ／ 4 | $2.6 / 6.4$ | 3.5 ／ 8.7 | 4.3 ／ 10.7 | 4.9 ／ 12.4 | 5.6 ／ 13.9 | $6.2 / 15.5$ |
| 18 | LL |  |  | 58 | 120 | 130 | 213 | 249 | 401 | 588 | 820 | 1，100 | 1，429 |  |
|  | TL |  |  | 80 | 170 | 185 | 308 | 361 | 587 | 865 | 1，093 | 1，323 | 1，558 | 1，731 |
|  | BRG |  |  | 1.5 ／ 3 | 1.5 ／ 3 | 1.5 ／ 3 | $1.5 / 3$ | 1.5 ／ 3.2 | 2.1 ／ 5.2 | $3 / 7.6$ | 3.8 ／ 9.5 | 4.6 ／ 11.5 | 5.4 ／ 13.6 | 44，727 |
| 20 | LL |  |  | 43 | 88 | 95 | 156 | 183 | 296 | 435 | 609 | 820 | 1，070 | 1，359 |
|  | TL |  |  | 57 | 122 | 133 | 223 | 262 | 429 | 636 | 882 | 1，068 | 1，270 | 1，487 |
|  | BRG |  |  | 1.5 ／ 3 | 1.5 ／ 3 | 1.5 ／ 3 | 1.5 ／ 3 | 1.5 ／ 3 | 1.7 ／ 4.2 | 2.5 ／ 6.2 | 3.4 ／ 8.6 | 4.1 ／ 10.4 | $4.9 / 12.3$ | $5.8 / 14.4$ |
| 22 | LL |  |  |  | 66 | 72 | 118 | 138 | 224 | 330 | 464 | 627 | 820 | 1，045 |
|  | TL |  |  |  | 90 | 98 | 166 | 195 | 322 | 479 | 677 | 879 | 1，045 | 1，224 |
|  | BRG |  |  |  | 1.5 ／ 3 | 1.5 ／ 3 | 1.5 ／ 3 | 1.5 ／ 3 | 1.5 ／ 3.5 | 2.1 ／ 5.2 | $2.9 / 7.3$ | 3.8 ／ 9.4 | 4.5 ／ 11.2 | 5.2 ／ 13.1 |
| 24 | LL |  |  |  | 51 | 55 | 91 | 107 | 174 | 257 | 361 | 489 | 641 | 820 |
|  | TL |  |  |  | 67 | 73 | 125 | 149 | 246 | 369 | 523 | 713 | 875 | 1，025 |
|  | BRG |  |  |  | 1.5 ／ 3 | 1.5 ／ 3 | 1.5 ／ 3 | 1.5 ／ 3 | $1.5 / 3$ | 1.8 ／ 4.4 | $2.5 / 6.2$ | 3.4 ／ 8.4 | $4.1 / 10.3$ | 4.8 ／ 12 |
| 26 | LL |  |  |  | 40 | 44 | 72 | 85 | 137 | 203 | 286 | 389 | 511 | 654 |
|  | TL |  |  |  | 51 | 56 | 97 | 115 | 192 | 289 | 411 | 562 | 742 | 870 |
|  | BRG |  |  |  | 1.5 ／ 3 | 1.5 ／ 3 | $1.5 / 3$ | 1.5 ／ 3 | $1.5 / 3$ | 1.5 ／ 3.8 | $2.1 / 5.3$ | 2.9 ／ 7.2 | 3.8 ／ 9.5 | 4.4 ／ 11.1 |
| 28 | LL |  |  |  |  |  | 58 | 68 | 110 | 164 | 231 | 314 | 413 | 530 |
|  | TL |  |  |  |  |  | 75 | 90 | 151 | 229 | 328 | 450 | 597 | 747 |
|  | BRG |  |  |  |  |  | 1.5 ／ 3 | 1.5 ／ 3 | 1.5 ／ 3 | 1.5 ／ 3.3 | 1.8 ／ 4.6 | 2.5 ／ 6.3 | 3.3 ／ 8.3 | 4.1 ／ 10.3 |
| 30 | LL |  |  |  |  |  | 47 | 55 | 90 | 134 | 189 | 257 | 338 | 435 |
|  | TL |  |  |  |  |  | 59 | 71 | 121 | 184 | 265 | 365 | 485 | 628 |
|  | BRG |  |  |  |  |  | 1.5 ／ 3 | 1.5 ／ 3 | 1.5 ／ 3 | 1.5 ／ 3 | 1.6 ／ 4 | 2.2 ／ 5.5 | 2.9 ／ 7.3 | 3.7 ／ 9.3 |

－The PLF load values in this table are based on the LVL member having lateral bracing at $24 "$ O．C．or less along its entire length．
－1－3／4＂LVL members 16 ＂and deeper and 1－1／2＂LVL members 14 ＂and deeper，must be a minimum of two plies unless designed by a design professional．Except for ledgers．
－Allowable PLF loads for single or multiple ply $1-1 / 2^{\prime \prime}$ wide LVL members can be obtained by multiplying the table values by 0.85 ．（Required bearing lengths are the same）
－This table may be used for either simple or multiple spans．
－Span is centerline of bearing to centerline of bearing．
－Loads shown can be applied to the beam in addition to its own weight．
－See pages 41 and 42 for details on attaching multiple ply members．
－Allowable loads shown for multiple ply LVL members are also applicable to factory single billet members with the same width as the combined multiple plies．
－The values shown are based on the lower allowable uniform load for RigidLam LVL produced from Douglas－fir or Southern Pine veneer and therefore can be used for either species．PLF tables separated by species are available on the Roseburg website．

## Key to Table：

LL＝Maximum live load - limits deflection to $\mathrm{L} / 360$
TL＝Maximum total load - limits deflections to L／240
$B R G=$ Required end／interior bearing length（inches），based on bearing stress of 750 PSI．

PLE Tables Douglas-fir LVL and Southern Pine LVL
3-PLY 1-3/4" 2.1E RIGIDLAM® LVL - FLOOR (PLF) 100\% LOAD DURATION

| Span (ft.) | Depth | 4-3/8" | 5-1/2" | 7-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-7/8" | 14" | 16" | 18" | 20" | 22" | 24" |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 10 | LL | 112 | 219 | 491 | 987 | 1,065 | 1,707 | 1,981 |  |  |  |  |  |  |
|  | TL | 161 | 321 | 726 | 1,467 | 1,582 | 2,132 | 2,273 | 2,774 | 3,279 | 3,819 | 4,398 | 5,022 | 5,695 |
|  | BRG | 1.5 / 3 | $1.5 / 3$ | $1.5 / 3$ | 1.9 / 4.7 | 2/5.1 | 2.7 / 6.8 | $2.9 / 7.3$ | 3.5 / 8.9 | $4.2 / 10.5$ | 4.9 / 12.2 | 5.6 / 14.1 | 6.4 / 16.1 | 7.3 / 18.2 |
| 12 | LL | 65 | 128 | 289 | 586 | 633 | 1,025 | 1,194 | 1,887 |  |  |  |  |  |
|  | TL | 91 | 184 | 422 | 865 | 935 | 1,521 | 1,756 | 2,222 | 2,609 | 3,017 | 3,449 | 3,907 | 4,392 |
|  | BRG | $1.5 / 3$ | $1.5 / 3$ | $1.5 / 3$ | 1.5 / 3.4 | 1.5 / 3.6 | $2.3 / 5.9$ | 2.7 / 6.8 | 3.4 / 8.5 | 44,661 | 4.6 / 11.6 | 5.3 / 13.3 | 44,727 | $6.7 / 16.9$ |
| 14 | LL | 41 | 81 | 184 | 375 | 405 | 661 | 771 | 1,230 | 1,783 | 2,459 |  |  |  |
|  | TL | 55 | 113 | 265 | 549 | 594 | 974 | 1,139 | 1,732 | 2,165 | 2,492 | 2,835 | 3,195 | 3,572 |
|  | BRG | 1.5 / 3 | $1.5 / 3$ | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.8 / 4.4 | 2.1 / 5.1 | $3.1 / 7.8$ | 3.9 / 9.7 | 4.5 / 11.2 | $5.1 / 12.7$ | 5.7 / 14.3 | $6.4 / 16$ |
| 16 | LL |  | 55 | 124 | 254 | 275 | 449 | 526 | 843 | 1,230 | 1,707 | 2,279 |  |  |
|  | TL |  | 73 | 175 | 367 | 397 | 657 | 770 | 1,243 | 1,683 | 2,083 | 2,405 | 2,701 | 3,008 |
|  | BRG |  | $1.5 / 3$ | $1.5 / 3$ | 1.5 / 3 | 1.5 / 3 | 1.5 / 3.4 | $1.6 / 4$ | $2.6 / 6.4$ | 3.5 / 8.7 | 4.3 / 10.7 | 4.9 / 12.4 | $5.6 / 13.9$ | $6.2 / 15.5$ |
| 18 | LL |  | 38 | 87 | 180 | 194 | 319 | 373 | 602 | 882 | 1,230 | 1,650 | 2,144 |  |
|  | TL |  | 49 | 120 | 255 | 277 | 461 | 542 | 881 | 1,298 | 1,640 | 1,985 | 2,338 | 2,597 |
|  | BRG |  |  | $1.5 / 3$ | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3.2 | $2.1 / 5.2$ | $3 / 7.6$ | 3.8 / 9.5 | 4.6 / 11.5 | 5.4 / 13.6 | 44,727 |
| 20 | LL |  |  | 64 | 132 | 142 | 234 | 275 | 444 | 652 | 913 | 1,230 | 1,605 | 2,039 |
|  | TL |  |  | 85 | 183 | 199 | 334 | 394 | 644 | 954 | 1,323 | 1,602 | 1,905 | 2,230 |
|  | BRG |  |  | $1.5 / 3$ | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | $1.5 / 3$ | $1.7 / 4.2$ | 2.5 / 6.2 | 3.4 / 8.6 | 4.1 / 10.4 | 4.9 / 12.3 | 5.8/14.4 |
| 22 | LL |  |  | 48 | 99 | 107 | 177 | 208 | 336 | 496 | 696 | 940 | 1,230 | 1,568 |
|  | TL |  |  | 61 | 135 | 147 | 248 | 293 | 483 | 719 | 1,016 | 1,319 | 1,568 | 1,837 |
|  | BRG |  |  | 1.5 / 3 | $1.5 / 3$ | 1.5 / 3 | 1.5 / 3 | $1.5 / 3$ | 1.5 / 3.5 | $2.1 / 5.2$ | $2.9 / 7.3$ | 3.8 / 9.4 | 4.5 / 11.2 | 5.2 / 13.1 |
| 24 | LL |  |  |  | 77 | 83 | 137 | 161 | 261 | 385 | 542 | 733 | 962 | 1,230 |
|  | TL |  |  |  | 101 | 110 | 188 | 223 | 370 | 553 | 785 | 1,069 | 1,312 | 1,537 |
|  | BRG |  |  |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.8 / 4.4 | 2.5 / 6.2 | 3.4 / 8.4 | 4.1 / 10.3 | 4.8 / 12 |
| 26 | LL |  |  |  | 60 | 65 | 108 | 127 | 206 | 305 | 430 | 583 | 766 | 981 |
|  | TL |  |  |  | 77 | 84 | 145 | 172 | 288 | 433 | 617 | 844 | 1,113 | 1,305 |
|  | BRG |  |  |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3.8 | 2.1 / 5.3 | $2.9 / 7.2$ | 3.8 / 9.5 | 4.4 / 11.1 |
| 28 | LL |  |  |  | 49 | 53 | 87 | 102 | 166 | 245 | 346 | 470 | 619 | 795 |
|  | TL |  |  |  | 59 | 64 | 113 | 135 | 227 | 344 | 492 | 675 | 895 | 1,120 |
|  | BRG |  |  |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3.3 | 1.8 / 4.6 | 2.5 / 6.3 | 3.3 / 8.3 | $4.1 / 10.3$ |
| 30 | LL |  |  |  |  | 43 | 71 | 83 | 135 | 200 | 283 | 385 | 508 | 652 |
|  | TL |  |  |  |  | 50 | 89 | 106 | 181 | 276 | 397 | 547 | 728 | 942 |
|  | BRG |  |  |  |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.6 / 4 | 2.2 / 5.5 | $2.9 / 7.3$ | 3.7 / 9.3 |
| 32 | LL |  |  |  |  |  | 58 | 69 | 112 | 166 | 234 | 319 | 421 | 542 |
|  | TL |  |  |  |  |  | 70 | 85 | 146 | 224 | 324 | 448 | 598 | 776 |
|  | BRG |  |  |  |  |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3.6 | $1.9 / 4.9$ | 2.6 / 6.4 | 3.3 / 8.3 |
| 34 | LL |  |  |  |  |  | 49 | 57 | 93 | 139 | 196 | 267 | 353 | 455 |
|  | TL |  |  |  |  |  | 56 | 68 | 119 | 183 | 267 | 370 | 496 | 645 |
|  | BRG |  |  |  |  |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3.2 | 1.7 / 4.3 | 2.3 / 5.7 | 2.9 / 7.4 |
| 36 | LL |  |  |  |  |  |  | 48 | 79 | 117 | 166 | 226 | 299 | 385 |
|  | TL |  |  |  |  |  |  | 54 | 97 | 151 | 221 | 308 | 414 | 541 |
|  | BRG |  |  |  |  |  |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3.9 | 2 / 5.1 | 2.6 / 6.6 |

4-PLY 1-3/4" 2.1E RIGIDLAM® LVL - FLOOR (PLF) 100\% LOAD DURATION

| Span (ft.) | Depth | 4-3/8" | 5-1/2" | 7-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-7/8" | 14" | 16" | 18" | 20" | 22" | 24" |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 10 | LL | 149 | 293 | 655 | 1,316 | 1,419 | 2,277 | 2,642 |  |  |  |  |  |  |
|  | TL | 214 | 428 | 968 | 1,955 | 2,110 | 2,843 | 3,031 | 3,699 | 4,371 | 5,092 | 5,865 | 6,696 | 7,593 |
|  | BRG | 1.5 / 3 | $1.5 / 3$ | 1.5 / 3 | 1.9 / 4.7 | $2 / 5.1$ | 2.7 / 6.8 | 2.9 / 7.3 | 3.5 / 8.9 | 4.2 / 10.5 | 4.9/12.2 | $5.6 / 14.1$ | $6.4 / 16.1$ | 7.3 / 18.2 |
| 12 | LL | 87 | 171 | 385 | 782 | 844 | 1,367 | 1,592 | 2,517 |  |  |  |  |  |
|  | TL | 121 | 245 | 563 | 1,154 | 1,247 | 2,028 | 2,342 | 2,963 | 3,479 | 4,023 | 4,599 | 5,209 | 5,856 |
|  | BRG | $1.5 / 3$ | 1.5 / 3 | 1.5 / 3 | 1.5 / 3.4 | 1.5 / 3.6 | 2.3 / 5.9 | 2.7 / 6.8 | 3.4 / 8.5 | 44,661 | $4.6 / 11.6$ | $5.3 / 13.3$ | 44,727 | 6.7 / 16.9 |
| 14 | LL | 55 | 108 | 245 | 500 | 540 | 881 | 1,029 | 1,640 | 2,378 | 3,279 |  |  |  |
|  | TL | 73 | 151 | 353 | 731 | 791 | 1,299 | 1,519 | 2,309 | 2,887 | 3,323 | 3,780 | 4,260 | 4,763 |
|  | BRG | 1.5 / 3 | $1.5 / 3$ | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.8 / 4.4 | $2.1 / 5.1$ | $3.1 / 7.8$ | 3.9 / 9.7 | 4.5 / 11.2 | $5.1 / 12.7$ | 5.7 / 14.3 | 6.4 / 16 |
| 16 | LL |  | 73 | 165 | 339 | 366 | 599 | 701 | 1,124 | 1,640 | 2,277 | 3,038 |  |  |
|  | TL |  | 98 | 233 | 489 | 530 | 876 | 1,027 | 1,657 | 2,244 | 2,777 | 3,207 | 3,601 | 4,011 |
|  | BRG |  | $1.5 / 3$ | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3.4 | 1.6 / 4 | 2.6 / 6.4 | 3.5 / 8.7 | 4.3 / 10.7 | 4.9 / 12.4 | $5.6 / 13.9$ | 6.2 / 15.5 |
| 18 | LL |  | 51 | 117 | 240 | 259 | 425 | 498 | 802 | 1,176 | 1,640 | 2,200 | 2,858 |  |
|  | TL |  | 66 | 160 | 340 | 369 | 615 | 723 | 1,175 | 1,731 | 2,187 | 2,647 | 3,117 | 3,462 |
|  | BRG |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3.2 | $2.1 / 5.2$ | 3/7.6 | 3.8 / 9.5 | 4.6 / 11.5 | 5.4 / 13.6 | 44,727 |
| 20 | LL |  |  | 85 | 176 | 190 | 312 | 366 | 592 | 870 | 1,218 | 1,640 | 2,139 | 2,718 |
|  | TL |  |  | 113 | 244 | 265 | 446 | 525 | 859 | 1,272 | 1,764 | 2,136 | 2,539 | 2,974 |
|  | BRG |  |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.7 / 4.2 | 2.5 / 6.2 | 3.4 / 8.6 | 4.1 / 10.4 | 4.9 / 12.3 | $5.8 / 14.4$ |
| 22 | LL |  |  | 64 | 132 | 143 | 236 | 277 | 448 | 661 | 928 | 1,253 | 1,640 | 2,091 |
|  | TL |  |  | 82 | 180 | 196 | 331 | 391 | 644 | 959 | 1,355 | 1,758 | 2,091 | 2,449 |
|  | BRG |  |  | $1.5 / 3$ | $1.5 / 3$ | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3.5 | $2.1 / 5.2$ | $2.9 / 7.3$ | 3.8 / 9.4 | 4.5 / 11.2 | 5.2 / 13.1 |
| 24 | LL |  |  | 50 | 102 | 111 | 183 | 214 | 348 | 513 | 722 | 978 | 1,283 | 1,640 |
|  | TL |  |  | 60 | 135 | 147 | 251 | 297 | 493 | 737 | 1,047 | 1,426 | 1,750 | 2,050 |
|  | BRG |  |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.8 / 4.4 | $2.5 / 6.2$ | 3.4 / 8.4 | 4.1/10.3 | 4.8 / 12 |
| 26 | LL |  |  |  | 81 | 87 | 144 | 169 | 275 | 407 | 573 | 777 | 1,021 | 1,308 |
|  | TL |  |  |  | 102 | 112 | 193 | 229 | 384 | 577 | 823 | 1,125 | 1,484 | 1,739 |
|  | BRG |  |  |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3.8 | $2.1 / 5.3$ | $2.9 / 7.2$ | 3.8 / 9.5 | 4.4 / 11.1 |
| 28 | LL |  |  |  | 65 | 70 | 116 | 136 | 221 | 327 | 462 | 627 | 826 | 1,060 |
|  | TL |  |  |  | 78 | 86 | 151 | 180 | 303 | 458 | 656 | 900 | 1,194 | 1,493 |
|  | BRG |  |  |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3.3 | 1.8 / 4.6 | 2.5 / 6.3 | 3.3 / 8.3 | 4.1 / 10.3 |
| 30 | LL |  |  |  | 53 | 57 | 94 | 111 | 180 | 267 | 378 | 513 | 677 | 870 |
|  | TL |  |  |  | 60 | 66 | 118 | 142 | 242 | 368 | 530 | 729 | 970 | 1,256 |
|  | BRG |  |  |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.6 / 4 | 2.2 / 5.5 | 2.9 / 7.3 | 3.7 / 9.3 |
| 32 | LL |  |  |  |  | 47 | 78 | 91 | 149 | 221 | 312 | 425 | 561 | 722 |
|  | TL |  |  |  |  | 51 | 94 | 113 | 195 | 299 | 432 | 597 | 797 | 1,034 |
|  | BRG |  |  |  |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3.6 | 1.9 / 4.9 | 2.6 / 6.4 | 3.3 / 8.3 |
| 34 | LL |  |  |  |  |  | 65 | 76 | 124 | 185 | 261 | 356 | 471 | 606 |
|  | TL |  |  |  |  |  | 75 | 90 | 158 | 244 | 355 | 494 | 661 | 860 |
|  | BRG |  |  |  |  |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3.2 | 1.7 / 4.3 | 2.3 / 5.7 | 2.9 / 7.4 |
| 36 | LL |  |  |  |  |  | 55 | 64 | 105 | 156 | 221 | 301 | 398 | 513 |
|  | TL |  |  |  |  |  | 59 | 72 | 129 | 201 | 295 | 411 | 553 | 721 |
|  | BRG |  |  |  |  |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3.9 | 2 / 5.1 | 2.6 / 6.6 |

Refer to notes on previous page.

## PLF Tables Dougas－fir LVL and Southem Pine LVL

## 1－PLY 1－3／4＂2．1E RIGIDLAM® LVL－ROOF SNOW（PLF）11．5\％LOAD DURATION

| Span（ft．） | Depth | 4－3／8＂ | 5－1／2＂ | 7－1／4＂ | 9－1／4＂ | 9－1／2＂ | 11－1／4＂ | 11－7／8＂ | 14＂ | 16＂ | 18＂ | 20＂ | 22＂ | 24＂ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 6 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ | $\begin{gathered} 250 \\ 331 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 481 \\ 639 \\ 1.5 / 3.7 \end{gathered}$ | $\begin{gathered} 893 \\ 2 / 5.1 \end{gathered}$ | $\begin{gathered} 1,203 \\ 2.8 / 6.9 \end{gathered}$ | $\begin{gathered} 1,245 \\ 2.9 / 7.1 \end{gathered}$ | $\begin{gathered} 1,551 \\ 3.6 / 8.9 \end{gathered}$ | $\begin{gathered} 1,668 \\ 3.8 / 9.6 \end{gathered}$ | $\begin{gathered} 2,101 \\ 4.8 / 12 \end{gathered}$ |  |  |  |  |  |
| 8 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ | $\begin{gathered} 108 \\ 142 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 211 \\ 278 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 466 \\ 603 \\ 1.9 / 4.7 \end{gathered}$ | $\begin{gathered} 846 \\ 2.6 / 6.5 \\ \hline \end{gathered}$ | $\begin{gathered} 873 \\ 2.7 / 6.7 \end{gathered}$ | $\begin{gathered} 1,072 \\ 3.3 / 8.2 \\ \hline \end{gathered}$ | $\begin{gathered} 1,146 \\ 3.5 / 8.8 \\ \hline \end{gathered}$ | $\begin{gathered} 1,414 \\ 4.3 / 10.8 \\ \hline \end{gathered}$ |  |  |  |  |  |
| 10 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ | $\begin{gathered} 56 \\ 72 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 110 \\ 143 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 246 \\ 324 \\ 1.5 / 3.1 \end{gathered}$ | $\begin{gathered} 494 \\ 609 \\ 2.4 / 6 \end{gathered}$ | $\begin{gathered} 532 \\ 640 \\ 2.5 / 6.2 \end{gathered}$ | $\begin{gathered} 818 \\ 3.1 / 7.8 \\ \hline \end{gathered}$ | $\begin{gathered} 872 \\ 3.3 / 8.4 \end{gathered}$ | $\begin{gathered} 1,064 \\ 4.1 / 10.2 \end{gathered}$ |  |  |  |  |  |
| 12 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ |  | $\begin{gathered} 64 \\ 83 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 144 \\ 189 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 293 \\ 386 \\ 1.8 / 4.5 \end{gathered}$ | $\begin{gathered} 317 \\ 417 \\ 1.9 / 4.8 \end{gathered}$ | $\begin{gathered} 513 \\ 609 \\ 2.8 / 7.1 \end{gathered}$ | $\begin{gathered} 597 \\ 674 \\ 3.1 / 7.8 \end{gathered}$ | $\begin{gathered} 853 \\ 3.9 / 9.8 \end{gathered}$ |  |  |  |  |  |
| 14 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ |  | $\begin{gathered} 41 \\ 51 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 92 \\ 119 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 188 \\ 245 \\ 1.5 / 3.3 \end{gathered}$ | $\begin{gathered} 203 \\ 265 \\ 1.5 / 3.6 \end{gathered}$ | $\begin{gathered} 330 \\ 435 \\ 2.3 / 5.9 \end{gathered}$ | $\begin{gathered} 386 \\ 494 \\ 2.7 / 6.7 \end{gathered}$ | $\begin{gathered} 615 \\ 665 \\ 3.6 / 9 \end{gathered}$ |  |  |  |  |  |
| 16 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ |  |  | $\begin{gathered} 62 \\ 79 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 127 \\ 165 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 137 \\ 178 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 225 \\ 294 \\ 1.8 / 4.6 \end{gathered}$ | $\begin{gathered} 263 \\ 344 \\ 2.1 / 5.3 \end{gathered}$ | $\begin{gathered} 421 \\ 507 \\ 3.1 / 7.8 \end{gathered}$ |  |  |  |  |  |
| 18 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ |  |  | $\begin{gathered} 44 \\ 55 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 90 \\ 115 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 97 \\ 125 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 160 \\ 207 \\ 1.5 / 3.6 \end{gathered}$ | $\begin{gathered} 187 \\ 243 \\ 1.7 / 4.3 \end{gathered}$ | $\begin{gathered} 301 \\ 394 \\ 2.8 / 6.9 \end{gathered}$ |  |  |  |  |  |
| 20 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ |  |  |  | $\begin{gathered} 66 \\ 83 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 71 \\ 90 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 117 \\ 150 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 137 \\ 177 \\ 1.5 / 3.5 \end{gathered}$ | $\begin{gathered} 222 \\ 289 \\ 2.3 / 5.6 \end{gathered}$ |  |  |  |  |  |
| 22 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ |  |  |  | $\begin{gathered} 50 \\ 61 \\ 1.5 / 3 \\ \hline \end{gathered}$ | $\begin{gathered} 54 \\ 67 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 89 \\ 112 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 104 \\ 132 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 168 \\ 217 \\ 1.9 / 4.7 \end{gathered}$ |  |  |  |  |  |
| 24 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ |  |  |  |  | $\begin{gathered} 42 \\ 51 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 68 \\ 86 \\ 1.5 / 3 \\ \hline \end{gathered}$ | $\begin{gathered} 80 \\ 101 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 130 \\ 167 \\ 1.6 / 4 \end{gathered}$ |  |  |  |  |  |
| 26 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ |  |  |  |  |  | $\begin{gathered} 54 \\ 66 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 63 \\ 79 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 103 \\ 130 \\ 1.5 / 3.4 \end{gathered}$ |  |  |  |  |  |
| 28 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ |  |  |  |  |  | $\begin{gathered} 43 \\ 52 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 51 \\ 62 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 83 \\ 103 \\ 1.5 / 3 \end{gathered}$ |  |  |  |  |  |
| 30 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ |  |  |  |  |  |  |  | $\begin{gathered} 68 \\ 83 \\ 1.5 / 3 \end{gathered}$ |  |  |  |  |  |

2－PLY 1－3／4＂2．1E RIGIDLAM® LVL－ROOF SNOW（PLF）11．5\％LOAD DURATION

| Span（ft．） | Depth | 4－3／8＂ | 5－1／2＂ | 7－1／4＂ | 9－1／4＂ | 9－1／2＂ | 11－1／4＂ | 11－7／8＂ | 14＂ | 16＂ | 18＂ | 20＂ | 22＂ | 24＂ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 6 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ | $\begin{gathered} 499 \\ 661 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 962 \\ 1,277 \\ 1.5 / 3.7 \end{gathered}$ | $\begin{aligned} & 1,786 \\ & 2 / 5.1 \end{aligned}$ | $\begin{gathered} 2,407 \\ 2.8 / 6.9 \end{gathered}$ | $\begin{gathered} 2,489 \\ 2.9 / 7.1 \end{gathered}$ | $\begin{gathered} 3,101 \\ 3.6 / 8.9 \end{gathered}$ | $\begin{gathered} 3,335 \\ 3.8 / 9.6 \end{gathered}$ | $\begin{gathered} 4,203 \\ 4.8 / 12 \end{gathered}$ | $\begin{gathered} 5,136 \\ 5.9 / 14.7 \end{gathered}$ | $\begin{gathered} 6,207 \\ 7.1 / 17.8 \end{gathered}$ | $\begin{gathered} 7,450 \\ 8.5 / 21.3 \end{gathered}$ | $\begin{gathered} 8,910 \\ 10.2 / 25.5 \end{gathered}$ | $\begin{gathered} 10,648 \\ 512.2 / 30.5 \end{gathered}$ |
| 8 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ | $\begin{gathered} 216 \\ 283 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 421 \\ 556 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 931 \\ 1,206 \\ 1.9 / 4.7 \end{gathered}$ | $\begin{gathered} 1,693 \\ 2.6 / 6.5 \end{gathered}$ | $\begin{gathered} 1,747 \\ 2.7 / 6.7 \end{gathered}$ | $\begin{gathered} 2,143 \\ 3.3 / 8.2 \\ \hline \end{gathered}$ | $\begin{gathered} 2,292 \\ 3.5 / 8.8 \end{gathered}$ | $\begin{gathered} 2,828 \\ 4.3 / 10.8 \\ \hline \end{gathered}$ | $\begin{gathered} 3,379 \\ 5.2 / 12.9 \end{gathered}$ | $\begin{gathered} 3,984 \\ 6.1 / 15.2 \end{gathered}$ | $\begin{gathered} 4,649 \\ 7.1 / 17.8 \\ \hline \end{gathered}$ | $\begin{gathered} 5,384 \\ 8.2 / 20.6 \end{gathered}$ | $\begin{gathered} 6,201 \\ 9.5 / 23.7 \end{gathered}$ |
| 10 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ | $\begin{gathered} 112 \\ 144 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 219 \\ 287 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 491 \\ 647 \\ 1.5 / 3.1 \end{gathered}$ | $\begin{gathered} 987 \\ 1,217 \\ 2.4 / 6 \end{gathered}$ | $\begin{gathered} 1,065 \\ 1,280 \\ 2.5 / 6.2 \end{gathered}$ | $\begin{gathered} 1,636 \\ 3.1 / 7.8 \end{gathered}$ | $\begin{gathered} 1,745 \\ 3.3 / 8.4 \end{gathered}$ | $\begin{gathered} 2,129 \\ 4.1 / 10.2 \end{gathered}$ | $\begin{gathered} 2,516 \\ 4.8 / 12.1 \end{gathered}$ | $\begin{gathered} 2,930 \\ 5.6 / 14 \end{gathered}$ | $\begin{gathered} 3,375 \\ 6.5 / 16.2 \end{gathered}$ | $\begin{gathered} 3,854 \\ 7.4 / 18.5 \end{gathered}$ | $\begin{gathered} 4,370 \\ 8.4 / 20.9 \end{gathered}$ |
| 12 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ | $\begin{gathered} 65 \\ 82 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 128 \\ 165 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 289 \\ 378 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 586 \\ 772 \\ 1.8 / 4.5 \end{gathered}$ | $\begin{gathered} 633 \\ 834 \\ 1.9 / 4.8 \end{gathered}$ | $\begin{gathered} 1,025 \\ 1,219 \\ 2.8 / 7.1 \end{gathered}$ | $\begin{gathered} 1,194 \\ 1,348 \\ 3.1 / 7.8 \end{gathered}$ | $\begin{gathered} 1,706 \\ 3.9 / 9.8 \end{gathered}$ | $\begin{gathered} 2,003 \\ 4.6 / 11.5 \end{gathered}$ | $\begin{gathered} 2,316 \\ 5.3 / 13.3 \end{gathered}$ | $\begin{gathered} 2,648 \\ 6.1 / 15.2 \end{gathered}$ | $\begin{gathered} 2,999 \\ 6.9 / 17.3 \end{gathered}$ | $\begin{gathered} 3,371 \\ 7.8 / 19.4 \end{gathered}$ |
| 14 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ | $\begin{gathered} 41 \\ 50 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 81 \\ 103 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 184 \\ 238 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 375 \\ 491 \\ 1.5 / 3.3 \end{gathered}$ | $\begin{gathered} 405 \\ 531 \\ 1.5 / 3.6 \end{gathered}$ | $\begin{gathered} 661 \\ 870 \\ 2.3 / 5.9 \end{gathered}$ | $\begin{gathered} 771 \\ 987 \\ 2.7 / 6.7 \end{gathered}$ | $\begin{aligned} & 1,230 \\ & 1,330 \\ & 3.6 / 9 \end{aligned}$ | $\begin{gathered} 1,662 \\ 4.5 / 11.2 \end{gathered}$ | $\begin{gathered} 1,914 \\ 5.2 / 12.9 \end{gathered}$ | $\begin{gathered} 2,177 \\ 5.9 / 14.6 \end{gathered}$ | $\begin{gathered} 2,453 \\ 6.6 / 16.5 \end{gathered}$ | $\begin{gathered} 2,742 \\ 7.4 / 18.4 \end{gathered}$ |
| 16 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ |  | $\begin{gathered} 55 \\ 67 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 124 \\ 158 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 254 \\ 329 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 275 \\ 356 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 449 \\ 588 \\ 1.8 / 4.6 \end{gathered}$ | $\begin{gathered} 526 \\ 689 \\ 2.1 / 5.3 \end{gathered}$ | $\begin{gathered} 843 \\ 1,015 \\ 3.1 / 7.8 \end{gathered}$ | $\begin{gathered} 1,230 \\ 1,293 \\ 44,661 \end{gathered}$ | $\begin{gathered} 1,600 \\ 4.9 / 12.3 \end{gathered}$ | $\begin{gathered} 1,847 \\ 5.7 / 14.2 \end{gathered}$ | $\begin{gathered} 2,074 \\ 6.4 / 16 \\ \hline \end{gathered}$ | $\begin{gathered} 2,310 \\ 7.1 / 17.8 \end{gathered}$ |
| 18 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ |  |  | $\begin{gathered} 87 \\ 109 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 180 \\ 230 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 194 \\ 249 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 319 \\ 414 \\ 1.5 / 3.6 \end{gathered}$ | $\begin{gathered} 373 \\ 486 \\ 1.7 / 4.3 \end{gathered}$ | $\begin{gathered} 602 \\ 788 \\ 2.8 / 6.9 \end{gathered}$ | $\begin{gathered} 882 \\ 1,018 \\ 3.5 / 8.9 \end{gathered}$ | $\begin{aligned} & 1,230 \\ & 1,260 \\ & 4.4 / 11 \end{aligned}$ | $\begin{gathered} 1,525 \\ 5.3 / 13.2 \end{gathered}$ | $\begin{gathered} 1,796 \\ 6.2 / 15.6 \end{gathered}$ | $\begin{gathered} 1,995 \\ 6.9 / 17.3 \end{gathered}$ |
| 20 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ |  |  | $\begin{gathered} 64 \\ 78 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 132 \\ 166 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 142 \\ 180 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 234 \\ 301 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 275 \\ 354 \\ 1.5 / 3.5 \end{gathered}$ | $\begin{gathered} 444 \\ 577 \\ 2.3 / 5.6 \end{gathered}$ | $\begin{gathered} 652 \\ 821 \\ 3.2 / 8 \end{gathered}$ | $\begin{gathered} 913 \\ 1,017 \\ 3.9 / 9.9 \end{gathered}$ | $\begin{gathered} 1,230 \\ 1,231 \\ 4.8 / 11.9 \end{gathered}$ | $\begin{gathered} 1,464 \\ 5.7 / 14.2 \end{gathered}$ | $\begin{gathered} 1,713 \\ 6.6 / 16.6 \end{gathered}$ |
| 22 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ |  |  | $\begin{gathered} 48 \\ 57 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 99 \\ 123 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 107 \\ 134 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 177 \\ 225 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 208 \\ 265 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 336 \\ 434 \\ 1.9 / 4.7 \end{gathered}$ | $\begin{gathered} 496 \\ 644 \\ 2.8 / 6.9 \end{gathered}$ | $\begin{gathered} 696 \\ 837 \\ 3.6 / 9 \end{gathered}$ | $\begin{gathered} 940 \\ 1,014 \\ 4.3 / 10.8 \end{gathered}$ | $\begin{gathered} 1,206 \\ 5.1 / 12.9 \end{gathered}$ | $\begin{gathered} 1,412 \\ 44,727 \end{gathered}$ |
| 24 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ |  |  |  | $\begin{gathered} 77 \\ 93 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 83 \\ 101 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 137 \\ 171 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 161 \\ 202 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 261 \\ 333 \\ 1.6 / 4 \end{gathered}$ | $\begin{gathered} 385 \\ 497 \\ 2.3 / 5.9 \end{gathered}$ | $\begin{gathered} 542 \\ 701 \\ 3.3 / 8.2 \end{gathered}$ | $\begin{gathered} 733 \\ 849 \\ 4 / 9.9 \end{gathered}$ | $\begin{gathered} 962 \\ 1,009 \\ 4.7 / 11.8 \end{gathered}$ | $\begin{gathered} 1,182 \\ 5.5 / 13.8 \end{gathered}$ |
| 26 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ |  |  |  | $\begin{gathered} 60 \\ 71 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 65 \\ 78 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 108 \\ 133 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 127 \\ 157 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 206 \\ 261 \\ 1.5 / 3.4 \end{gathered}$ | $\begin{gathered} 305 \\ 390 \\ 44,597 \end{gathered}$ | $\begin{gathered} 430 \\ 555 \\ 2.8 / 7.1 \end{gathered}$ | $\begin{gathered} 583 \\ 720 \\ 3.7 / 9.2 \end{gathered}$ | $\begin{gathered} 766 \\ 857 \\ 4.4 / 10.9 \end{gathered}$ | $\begin{gathered} 981 \\ 1,004 \\ 5.1 / 12.7 \end{gathered}$ |
| 28 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ |  |  |  | $\begin{gathered} 49 \\ 55 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 53 \\ 60 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 87 \\ 104 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 102 \\ 124 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 166 \\ 207 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 245 \\ 311 \\ 1.7 / 4.4 \end{gathered}$ | $\begin{gathered} 346 \\ 443 \\ 2.5 / 6.2 \end{gathered}$ | $\begin{gathered} 470 \\ 607 \\ 3.3 / 8.4 \end{gathered}$ | $\begin{gathered} 619 \\ 736 \\ 4 / 10.1 \\ \hline \end{gathered}$ | $\begin{gathered} 795 \\ 862 \\ 4.7 / 11.8 \end{gathered}$ |
| 30 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ |  |  |  |  |  | $\begin{gathered} 71 \\ 83 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 83 \\ 99 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 135 \\ 166 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 200 \\ 251 \\ 1.5 / 3.8 \end{gathered}$ | $\begin{gathered} 283 \\ 359 \\ 2.2 / 5.4 \end{gathered}$ | $\begin{gathered} 385 \\ 493 \\ 2.9 / 7.3 \end{gathered}$ | $\begin{gathered} 508 \\ 638 \\ 3.8 / 9.4 \end{gathered}$ | $\begin{gathered} 652 \\ 748 \\ 4.4 / 11 \end{gathered}$ |

－The PLF load values in this table are based on the LVL member having lateral bracing at 24＂O．C．or less along its entire length．
－1－3／4＂LVL members 16 ＂and deeper and 1－1／2＂LVL members 14 ＂and deeper，must be a minimum of two plies unless designed by a design professional．Except for ledgers．
－Allowable PLF loads for single or multiple ply $1-1 / 2^{\prime \prime}$ wide LVL members can be obtained by multiplying the table values by 0.85 ．（Required bearing lengths are the same）
－This table may be used for either simple or multiple spans．
－Span is centerline of bearing to centerline of bearing．
－Loads shown can be applied to the beam in addition to its own weight．
－See pages 41 and 42 for details on attaching multiple ply members．
－Allowable loads shown for multiple ply LVL members are also applicable to single billet LVL members with the same width as the combined multiple plies．
－The values shown are based on the lower allowable uniform load for RigidLam LVL produced from Douglas－fir or Southern Pine veneer and therefore can be used for either species．PLF tables separated by species are available on the Roseburg website．

## Key to Table：

LL＝Maximum live load - limits deflection to $\mathrm{L} / 240$
TL＝Maximum total load - limits deflections to L／180
$B R G=$ Required end／interior bearing length（inches），based on bearing stress of 750 PSI ．

PLF Tables Douglas fir LVL and Southem Pine LVL
3-PLY 1-3/4" 2.1E RIGIDLAM® LVL - ROOF SNOW (PLF) 11.5\% LOAD DURATION

| Span (ft.) | Depth | 4-3/8" | 5-1/2" | 7-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-7/8" | 14" | 16" | 18" | 20" | 22" | 24" |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 10 | LL | 168 | 329 | 737 | 1,481 | 1,597 |  |  |  |  |  |  |  |  |
|  | TL | 217 | 430 | 971 | 1,826 | 1,920 | 2,455 | 2,617 | 3,193 | 3,774 | 4,396 | 5,063 | 5,781 | 6,555 |
|  | BRG | 1.5 / 3 | 1.5 / 3 | 1.5 / 3.1 | 2.4 / 6 | 2.5 / 6.2 | 3.1 / 7.8 | 3.3 / 8.4 | 4.1 / 10.2 | 4.8 / 12.1 | 5.6 / 14 | $6.5 / 16.2$ | 7.4 / 18.5 | 8.4 / 20.9 |
| 12 | LL | 98 | 192 | 433 | 879 | 950 | 1,538 | 1,791 |  |  |  |  |  |  |
|  | TL | 123 | 248 | 567 | 1,158 | 1,251 | 1,828 | 2,023 | 2,559 | 3,004 | 3,474 | 3,971 | 4,498 | 5,057 |
|  | BRG | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.8 / 4.5 | 1.9 / 4.8 | 2.8 / 7.1 | 3.1 / 7.8 | 3.9 / 9.8 | 4.6 / 11.5 | $5.3 / 13.3$ | $6.1 / 15.2$ | $6.9 / 17.3$ | 7.8/19.4 |
| 14 | LL | 62 | 122 | 276 | 563 | 608 | 991 | 1,157 | 1,845 |  |  |  |  |  |
|  | TL | 76 | 154 | 356 | 736 | 796 | 1,304 | 1,481 | 1,995 | 2,494 | 2,870 | 3,265 | 3,679 | 4,113 |
|  | BRG | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3.3 | 1.5 / 3.6 | 2.3 / 5.9 | 2.7 / 6.7 | 3.6 / 9 | 4.5 / 11.2 | $5.2 / 12.9$ | 5.9 / 14.6 | $6.6 / 16.5$ | 7.4 / 18.4 |
| 16 | LL |  | 82 | 186 | 381 | 412 | 674 | 788 | 1,264 | 1,845 |  |  |  |  |
|  | TL |  | 101 | 237 | 494 | 535 | 882 | 1,033 | 1,522 | 1,939 | 2,399 | 2,771 | 3,111 | 3,465 |
|  | BRG |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.8 / 4.6 | 2.1 / 5.3 | 3.1 / 7.8 | 44,661 | $4.9 / 12.3$ | 5.7 / 14.2 | 6.4 / 16 | 7.1 / 17.8 |
| 18 | LL |  | 58 | 131 | 269 | 291 | 479 | 560 | 902 | 1,322 | 1,845 |  |  |  |
|  | TL |  | 68 | 164 | 345 | 374 | 621 | 729 | 1,182 | 1,527 | 1,890 | 2,287 | 2,693 | 2,992 |
|  | BRG |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3.6 | 1.7 / 4.3 | 2.8 / 6.9 | 3.5 / 8.9 | 4.4 / 11 | $5.3 / 13.2$ | 6.2 / 15.6 | 6.9 / 17.3 |
| 20 | LL |  |  | 96 | 197 | 214 | 352 | 412 | 666 | 979 | 1,370 | 1,845 |  |  |
|  | TL |  |  | 117 | 249 | 270 | 451 | 531 | 866 | 1,232 | 1,526 | 1,847 | 2,195 | 2,570 |
|  | BRG |  |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3.5 | 2.3 / 5.6 | 3.2 / 8 | 3.9 / 9.9 | 4.8 / 11.9 | 5.7 / 14.2 | 6.6 / 16.6 |
| 22 | LL |  |  | 72 | 149 | 161 | 266 | 311 | 504 | 743 | 1,044 | 1,410 |  |  |
|  | TL |  |  | 85 | 184 | 200 | 337 | 397 | 651 | 967 | 1,256 | 1,521 | 1,808 | 2,118 |
|  | BRG |  |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.9 / 4.7 | 2.8 / 6.9 | 3.6 / 9 | $4.3 / 10.8$ | $5.1 / 12.9$ | 44,727 |
| 24 | LL |  |  | 56 | 115 | 125 | 205 | 241 | 391 | 578 | 813 | 1,100 | 1,443 |  |
|  | TL |  |  | 63 | 139 | 152 | 257 | 303 | 500 | 746 | 1,051 | 1,273 | 1,514 | 1,774 |
|  | BRG |  |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.6 / 4 | 2.3 / 5.9 | 3.3 / 8.2 | 4 / 9.9 | 4.7 / 11.8 | $5.5 / 13.8$ |
| 26 | LL |  |  |  | 91 | 98 | 162 | 190 | 309 | 457 | 645 | 874 | 1,149 | 1,472 |
|  | TL |  |  |  | 107 | 116 | 199 | 236 | 391 | 585 | 832 | 1,080 | 1,285 | 1,506 |
|  | BRG |  |  |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3.4 | 44,597 | 2.8 / 7.1 | 3.7 / 9.2 | 4.4 / 10.9 | $5.1 / 12.7$ |
| 28 | LL |  |  |  | 73 | 79 | 130 | 153 | 249 | 368 | 520 | 706 | 929 | 1,192 |
|  | TL |  |  |  | 83 | 90 | 156 | 186 | 310 | 466 | 665 | 910 | 1,104 | 1,293 |
|  | BRG |  |  |  | 1.5 / 3 | $1.5 / 3$ | 1.5 / 3 | 1.5 / 3 | $1.5 / 3$ | 1.7 / 4.4 | 2.5 / 6.2 | 3.3 / 8.4 | 4/10.1 | 4.7 / 11.8 |
| 30 | LL |  |  |  | 59 | 64 | 106 | 125 | 203 | 301 | 425 | 578 | 761 | 979 |
|  | TL |  |  |  | 65 | 71 | 124 | 148 | 249 | 376 | 539 | 739 | 957 | 1,122 |
|  | BRG |  |  |  | $1.5 / 3$ | 1.5 / 3 | $1.5 / 3$ | 1.5 / 3 | 1.5 / 3 | 1.5 / 3.8 | 2.2 / 5.4 | $2.9 / 7.3$ | 3.8 / 9.4 | 4.4 / 11 |
| 32 | LL |  |  |  | 49 | 53 | 88 | 103 | 168 | 249 | 352 | 479 | 632 | 813 |
|  | TL |  |  |  | 51 | 56 | 100 | 119 | 202 | 307 | 441 | 607 | 808 | 982 |
|  | BRG |  |  |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3.4 | 1.9 / 4.8 | 2.6 / 6.5 | 3.4 / 8.6 | 4.1 / 10.3 |
| 34 | LL |  |  |  |  |  | 73 | 86 | 140 | 208 | 294 | 401 | 529 | 682 |
|  | TL |  |  |  |  |  | 80 | 96 | 165 | 253 | 365 | 504 | 672 | 865 |
|  | BRG |  |  |  |  |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.7 / 4.2 | 2.3 / 5.8 | $3 / 7.6$ | 3.9 / 9.7 |
| 36 | LL |  |  |  |  |  | 62 | 72 | 118 | 176 | 249 | 339 | 448 | 578 |
|  | ${ }_{\text {TL }}$ |  |  |  |  |  | 65 | 78 | 136 | 210 | 304 | 421 | 564 | 733 |
|  | BRG |  |  |  |  |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3.8 | $2.1 / 5.2$ | 2.7 / 6.8 | 3.5 / 8.8 |

4-PLY 1-3/4" 2.1E RIGIDLAM® LVL - ROOF SNOW (PLF) 115\% LOAD DURATION

| Span (ft.) | Depth | 4-3/8" | 5-1/2" | 7-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-7/8" | 14" | 16" | 18" | 20" | 22" | 24" |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 10 | LL | 223 | 439 | 982 | 1,974 | 2,129 |  |  |  |  |  |  |  |  |
|  | TL | 289 | 574 | 1,295 | 2,435 | 2,560 | 3,273 | 3,489 | 4,258 | 5,032 | 5,861 | 6,750 | 7,707 | 8,740 |
|  | BRG | 1.5 / 3 | $1.5 / 3$ | 1.5 / 3.1 | 2.4 / 6 | 2.5 / 6.2 | $3.1 / 7.8$ | 3.3 / 8.4 | 4.1 / 10.2 | 4.8 / 12.1 | $5.6 / 14$ | 6.5 / 16.2 | $7.4 / 18.5$ | 8.4 / 20.9 |
| 12 | LL | 130 | 256 | 578 | 1,173 | 1,266 | 2,051 | 2,389 |  |  |  |  |  |  |
|  | TL | 165 | 331 | 756 | 1,544 | 1,669 | 2,437 | 2,697 | 3,412 | 4,005 | 4,632 | 5,295 | 5,997 | 6,742 |
|  | BRG | 1.5 / 3 | $1.5 / 3$ | 1.5 / 3 | $1.8 / 4.5$ | 1.9 / 4.8 | $2.8 / 7.1$ | $3.1 / 7.8$ | 3.9 / 9.8 | 4.6 / 11.5 | 5.3 / 13.3 | 6.1 / 15.2 | 6.9 / 17.3 | 7.8 / 19.4 |
| 14 | LL | 82 | 162 | 368 | 750 | 811 | 1,322 | 1,543 | 2,460 |  |  |  |  |  |
|  | TL | 101 | 205 | 475 | 981 | 1,062 | 1,739 | 1,975 | 2,660 | 3,325 | 3,827 | 4,354 | 4,905 | 5,485 |
|  | BRG | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3.3 | 1.5 / 3.6 | 2.3 / 5.9 | 2.7 / 6.7 | 3.6 / 9 | 4.5 / 11.2 | 5.2 / 12.9 | 5.9 / 14.6 | $6.6 / 16.5$ | 7.4 / 18.4 |
| 16 | LL | 55 | 109 | 248 | 508 | 549 | 899 | 1,051 | 1,686 | 2,460 |  |  |  |  |
|  | TL | 65 | 134 | 316 | 658 | 713 | 1,176 | 1,377 | 2,030 | 2,585 | 3,199 | 3,694 | 4,148 | 4,620 |
|  | BRG | 1.5 / 3 | $1.5 / 3$ | $1.5 / 3$ | 1.5 / 3 | 1.5 / 3 | 1.8 / 4.6 | $2.1 / 5.3$ | $3.1 / 7.8$ | 44,661 | 4.9 / 12.3 | 5.7 / 14.2 | 6.4 / 16 | $7.1 / 17.8$ |
| 18 | LL |  | 77 | 175 | 359 | 389 | 638 | 747 | 1,203 | 1,763 | 2,460 |  |  |  |
|  | TL |  | 91 | 218 | 460 | 499 | 828 | 972 | 1,576 | 2,036 | 2,520 | 3,050 | 3,591 | 3,989 |
|  | BRG |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | $1.5 / 3.6$ | 1.7 / 4.3 | $2.8 / 6.9$ | 3.5 / 8.9 | 4.4 / 11 | $5.3 / 13.2$ | 6.2 / 15.6 | $6.9 / 17.3$ |
| 20 | LL |  | 56 | 128 | 263 | 285 | 469 | 549 | 887 | 1,305 | 1,827 | 2,460 |  |  |
|  | TL |  | 64 | 156 | 332 | 360 | 602 | 708 | 1,155 | 1,643 | 2,034 | 2,463 | 2,927 | 3,427 |
|  | BRG |  | $1.5 / 3$ | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | $1.5 / 3.5$ | $2.3 / 5.6$ | 3.2 / 8 | 3.9 / 9.9 | 4.8 / 11.9 | 5.7 / 14.2 | $6.6 / 16.6$ |
| 22 | LL |  |  | 96 | 199 | 215 | 354 | 415 | 672 | 991 | 1,392 | 1,880 |  |  |
|  | TL |  |  | 114 | 246 | 267 | 449 | 529 | 868 | 1,289 | 1,675 | 2,028 | 2,411 | 2,824 |
|  | BRG |  |  | $1.5 / 3$ | $1.5 / 3$ | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.9 / 4.7 | 2.8 / 6.9 | 3.6 / 9 | 4.3 / 10.8 | $5.1 / 12.9$ | 44,727 |
| 24 | LL |  |  | 74 | 153 | 166 | 274 | 321 | 521 | 770 | 1,083 | 1,467 | 1,924 |  |
|  | TL |  |  | 84 | 186 | 202 | 342 | 404 | 667 | 994 | 1,401 | 1,698 | 2,019 | 2,365 |
|  | BRG |  |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.6 / 4 | 2.3 / 5.9 | 3.3 / 8.2 | 4 / 9.9 | 4.7 / 11.8 | 5.5 / 13.8 |
| 26 | LL |  |  | 59 | 121 | 131 | 216 | 254 | 412 | 610 | 859 | 1,166 | 1,532 | 1,963 |
|  | TL |  |  | 63 | 142 | 155 | 265 | 314 | 521 | 780 | 1,109 | 1,441 | 1,714 | 2,008 |
|  | BRG |  |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | $1.5 / 3$ | 1.5 / 3 | 1.5 / 3.4 | 44,597 | $2.8 / 7.1$ | 3.7 / 9.2 | 4.4 / 10.9 | 5.1 / 12.7 |
| 28 | LL |  |  |  | 97 | 105 | 174 | 204 | 331 | 491 | 693 | 941 | 1,239 | 1,590 |
|  | TL |  |  |  | 110 | 121 | 208 | 247 | 413 | 622 | 887 | 1,214 | 1,471 | $1,724$ |
|  | BRG |  |  |  | $1.5 / 3$ | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.7 / 4.4 | 2.5 / 6.2 | 3.3 / 8.4 | 4/10.1 | $4.7 / 11.8$ |
| 30 | LL |  |  |  | 79 | 86 | 141 | 166 | 270 | 401 | 566 | 770 | 1,015 | 1,305 |
|  | TL |  |  |  | $86$ | $95$ | $166$ | $197$ | $332$ | $502$ | $718$ | $986$ | $1,276$ | $1,496$ |
|  | BRG |  |  |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | $1.5 / 3.8$ | $2.2 / 5.4$ | $2.9 / 7.3$ | $3.8 / 9.4$ | $4.4 / 11$ |
| 32 | LL |  |  |  | 65 | 71 | 117 | 137 | 223 | 331 | 469 | 638 | 842 | 1,083 |
|  | $\overline{\mathrm{TL}}$ |  |  |  | $68$ | 75 | 133 | 159 | 269 | 409 | 588 | $810$ | 1,078 | 1,309 |
|  | BRG |  |  |  | 1.5 / 3 | $1.5 / 3$ | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3.4 | $1.9 / 4.8$ | 2.6 / 6.5 | 3.4 / 8.6 | 4.1 / 10.3 |
| 34 | LL |  |  |  | 54 | 59 | 97 | 114 | 187 | 277 | 392 | 534 | 706 | 909 |
|  | TL |  |  |  | 54 | 59 | 107 | 128 | 220 | 337 | 486 | 672 | 896 | 1,154 |
|  | BRG |  |  |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.7 / 4.2 | 2.3 / 5.8 | $3 / 7.6$ | 3.9 / 9.7 |
| 36 | LL |  |  |  |  |  | 82 | 97 | 158 | 234 | 331 | 452 | 597 | 770 |
|  | TL |  |  |  |  |  | 87 | 105 | 181 | 279 | 405 | 562 | 752 | 978 |
|  | BRG |  |  |  |  |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3.8 | 2.1 / 5.2 | 2.7 / 6.8 | 3.5 / 8.8 |

Refer to notes on previous page.

## 1－PLY 1－3／4＂2．1E RIGIDLAM® LVL－ROOF NON－SNOW（PLF）125\％LOAD DURATION

| Span（ft．） | Depth | 4－3／8＂ | 5－1／2＂ | 7－1／4＂ | 9－1／4＂ | 9－1／2＂ | 11－1／4＂ | 11－7／8＂ | 14＂ | 16＂ | 18＂ | 20＂ | 22＂ | 24＂ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 6 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ | $\begin{gathered} 250 \\ 331 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 481 \\ 639 \\ 1.5 / 3.7 \end{gathered}$ | $\begin{gathered} 971 \\ 2.2 / 5.6 \end{gathered}$ | $\begin{aligned} & 1,308 \\ & 3 / 7.5 \\ & \hline \end{aligned}$ | $\begin{gathered} 1,353 \\ 3.1 / 7.8 \end{gathered}$ | $\begin{gathered} 1,686 \\ 3.9 / 9.7 \end{gathered}$ | $\begin{gathered} 1,813 \\ 4.2 / 10.4 \end{gathered}$ | $\begin{gathered} 2,285 \\ 5.2 / 13.1 \end{gathered}$ |  |  |  |  |  |
| 8 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ | $\begin{gathered} 108 \\ 142 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 211 \\ 278 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 466 \\ 617 \\ 1.9 / 4.7 \end{gathered}$ | $\begin{gathered} 920 \\ 2.8 / 7 \\ \hline \end{gathered}$ | $\begin{gathered} 950 \\ 2.9 / 7.3 \end{gathered}$ | $\begin{gathered} 1,165 \\ 3.6 / 8.9 \\ \hline \end{gathered}$ | $\begin{gathered} 1,246 \\ 3.8 / 9.5 \end{gathered}$ | $\begin{gathered} 1,537 \\ 4.7 / 11.8 \\ \hline \end{gathered}$ |  |  |  |  |  |
| 10 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ | $\begin{gathered} 56 \\ 72 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 110 \\ 143 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 246 \\ 324 \\ 1.5 / 3.1 \end{gathered}$ | $\begin{gathered} 494 \\ 653 \\ 2.5 / 6.3 \end{gathered}$ | $\begin{gathered} 532 \\ 696 \\ 2.7 / 6.8 \end{gathered}$ | $\begin{gathered} 854 \\ 890 \\ 3.4 / 8.5 \end{gathered}$ | $\begin{gathered} 949 \\ 3.6 / 9.1 \end{gathered}$ | $\begin{gathered} 1,158 \\ 4.4 / 11.1 \end{gathered}$ |  |  |  |  |  |
| 12 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ |  | $\begin{gathered} 64 \\ 83 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 144 \\ 189 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 293 \\ 386 \\ 1.8 / 4.5 \end{gathered}$ | $\begin{gathered} 317 \\ 417 \\ 1.9 / 4.8 \end{gathered}$ | $\begin{gathered} 513 \\ 663 \\ 3.1 / 7.7 \end{gathered}$ | $\begin{gathered} 597 \\ 733 \\ 3.4 / 8.5 \end{gathered}$ | $\begin{gathered} 928 \\ 4.3 / 10.7 \end{gathered}$ |  |  |  |  |  |
| 14 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ |  | $\begin{gathered} 41 \\ 51 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 92 \\ 119 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 188 \\ 245 \\ 1.5 / 3.3 \end{gathered}$ | $\begin{gathered} 203 \\ 265 \\ 1.5 / 3.6 \end{gathered}$ | $\begin{gathered} 330 \\ 435 \\ 2.3 / 5.9 \end{gathered}$ | $\begin{gathered} 386 \\ 508 \\ 2.7 / 6.9 \end{gathered}$ | $\begin{gathered} 615 \\ 723 \\ 3.9 / 9.7 \end{gathered}$ |  |  |  |  |  |
| 16 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ |  |  | $\begin{gathered} 62 \\ 79 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 127 \\ 165 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 137 \\ 178 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 225 \\ 294 \\ 1.8 / 4.6 \end{gathered}$ | $\begin{gathered} 263 \\ 344 \\ 2.1 / 5.3 \end{gathered}$ | $\begin{gathered} 421 \\ 552 \\ 3.4 / 8.5 \end{gathered}$ |  |  |  |  |  |
| 18 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ |  |  | $\begin{gathered} 44 \\ 55 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 90 \\ 115 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 97 \\ 125 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 160 \\ 207 \\ 1.5 / 3.6 \end{gathered}$ | $\begin{gathered} 187 \\ 243 \\ 1.7 / 4.3 \end{gathered}$ | $\begin{gathered} 301 \\ 394 \\ 2.8 / 6.9 \end{gathered}$ |  |  |  |  |  |
| 20 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ |  |  |  | $\begin{gathered} 66 \\ 83 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 71 \\ 90 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 117 \\ 150 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 137 \\ 177 \\ 1.5 / 3.5 \end{gathered}$ | $\begin{gathered} 222 \\ 289 \\ 2.3 / 5.6 \end{gathered}$ |  |  |  |  |  |
| 22 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ |  |  |  | $\begin{gathered} 50 \\ 61 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 54 \\ 67 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 89 \\ 112 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 104 \\ 132 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 168 \\ 217 \\ 1.9 / 4.7 \end{gathered}$ |  |  |  |  |  |
| 24 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ |  |  |  |  | $\begin{gathered} 42 \\ 51 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 68 \\ 86 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 80 \\ 101 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 130 \\ 167 \\ 1.6 / 4 \end{gathered}$ |  |  |  |  |  |
| 26 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ |  |  |  |  |  | $\begin{gathered} 54 \\ 66 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 63 \\ 79 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 103 \\ 130 \\ 1.5 / 3.4 \end{gathered}$ |  |  |  |  |  |
| 28 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ |  |  |  |  |  | $\begin{gathered} 43 \\ 52 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 51 \\ 62 \\ 1.5 / 3 \end{gathered}$ | $\begin{gathered} 83 \\ 103 \\ 1.5 / 3 \end{gathered}$ |  |  |  |  |  |
| 30 | $\begin{gathered} \mathrm{LL} \\ \mathrm{TL} \\ \mathrm{BRG} \end{gathered}$ |  |  |  |  |  |  |  | $\begin{gathered} 68 \\ 83 \\ 1.5 / 3 \end{gathered}$ |  |  |  |  |  |

2－PLY 1－3／4＂2．1E RIGIDLAM® LVL－ROOF NON－SNOW（PLF）125\％LOAD DURATION

| Span（ft．） | Depth | 4－3／8＂ | 5－1／2＂ | 7－1／4＂ | 9－1／4＂ | 9－1／2＂ | 11－1／4＂ | 11－7／8＂ | 14＂ | 16＂ | 18＂ | 20＂ | 22＂ | 24＂ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 6 | LL | 499 | 962 |  |  |  |  |  |  |  |  |  |  |  |
|  | TL | 661 | 1，277 | 1，942 | 2，617 | 2，706 | 3，372 | 3，627 | 4，570 | 5，584 | 6，748 | 8，100 | 9，686 | 11，576 |
|  | BRG | 1.5 ／ 3 | 1.5 ／ 3.7 | 2.2 ／ 5.6 | 3／7．5 | $3.1 / 7.8$ | $3.9 / 9.7$ | 4.2 ／ 10.4 | $5.2 / 13.1$ | $6.4 / 16$ | 7．7／19．3 | 9.3 ／ 23.2 | 11.1 ／ 27.7 | 13．3／ 33.1 |
| 8 | LL | 216 | 421 | 931 |  |  |  |  |  |  |  |  |  |  |
|  | TL | 283 | 556 | 1，235 | 1，841 | 1，900 | 2，331 | 2，492 | 3，075 | 3，675 | 4，332 | 5，055 | 5，854 | 6，742 |
|  | BRG | $1.5 / 3$ | $1.5 / 3$ | 1．9／4．7 | $2.8 / 7$ | $2.9 / 7.3$ | 3．6／8．9 | 3.8 ／ 9.5 | 4.7 ／ 11.8 | $5.6 / 14.1$ | $6.6 / 16.6$ | 7.7 ／ 19.3 | 9／22．4 | $10.3 / 25.8$ |
| 10 | LL | 112 | 219 | 491 | 987 | 1，065 | 1，707 |  |  |  |  |  |  |  |
|  | TL | 144 | 287 | 647 | 1，307 | 1，392 | 1，780 | 1，897 | 2，315 | 2，736 | 3，187 | 3，670 | 4，191 | 4，752 |
|  | BRG | 1.5 ／ 3 | 1.5 ／ 3 | 1.5 ／ 3.1 | 2.5 ／ 6.3 | 2.7 ／ 6.8 | 3.4 ／ 8.5 | 3.6 ／ 9.1 | 4.4 ／ 11.1 | $5.2 / 13.1$ | $6.1 / 15.3$ | 7／17．6 | 8／20．1 | $9.1 / 22.7$ |
| 12 | LL | 65 | 128 | 289 | 586 | 633 | 1，025 | 1，194 |  |  |  |  |  |  |
|  | TL | 82 | 165 | 378 | 772 | 834 | 1，325 | 1，467 | 1，855 | 2，178 | 2，519 | 2，880 | 3，261 | 3，666 |
|  | BRG | 1.5 ／ 3 | 1.5 ／ 3 | 1.5 ／ 3 | 1.8 ／ 4.5 | 1.9 ／ 4.8 | 3.1 ／ 7.7 | 3.4 ／ 8.5 | 4.3 ／ 10.7 | 5／12．5 | $5.8 / 14.5$ | 6.6 ／ 16.6 | 7.5 ／ 18.8 | 8.4 ／ 21.1 |
| 14 | LL | 41 | 81 | 184 | 375 | 405 | 661 | 771 | 1，230 | 1，783 |  |  |  |  |
|  | TL | 50 | 103 | 238 | 491 | 531 | 870 | 1，016 | 1，447 | 1，808 | 2，082 | 2，368 | 2，668 | 2，983 |
|  | BRG | 1.5 ／ 3 | 1.5 ／ 3 | 1.5 ／ 3 | 1.5 ／ 3.3 | 1.5 ／ 3.6 | 2.3 ／ 5.9 | 2.7 ／ 6.9 | 3.9 ／ 9.7 | 4.9 ／ 12.2 | 5.6 ／ 14 | 6.4 ／ 15.9 | 7.2 ／ 17.9 | 44，793 |
| 16 | LL |  | 55 | 124 | 254 | 275 | 449 | 526 | 843 | 1，230 | 1，707 |  |  |  |
|  | TL |  | 67 | 158 | 329 | 356 | 588 | 689 | 1，104 | 1，406 | 1，740 | 2，010 | 2，256 | 2，513 |
|  | BRG |  | 1.5 ／ 3 | 1.5 ／ 3 | 1.5 ／ 3 | 1.5 ／ 3 | 1.8 ／ 4.6 | $2.1 / 5.3$ | 3.4 ／ 8.5 | $4.3 / 10.8$ | 5.4 ／ 13.4 | $6.2 / 15.5$ | $6.9 / 17.4$ | 7.7 ／ 19.3 |
| 18 | LL |  |  | 87 | 180 | 194 | 319 | 373 | 602 | 882 | 1，230 | 1，650 |  |  |
|  | TL |  |  | 109 | 230 | 249 | 414 | 486 | 788 | 1，108 | 1，371 | 1，659 | 1，954 | 2，170 |
|  | BRG |  |  | 1.5 ／ 3 | 1.5 ／ 3 | 1.5 ／ 3 | $1.5 / 3.6$ | 1.7 ／ 4.3 | $2.8 / 6.9$ | 3.9 ／ 9.6 | 4.8 ／ 11.9 | 5.8 ／ 14.4 | $6.8 / 16.9$ | 7.5 ／ 18.8 |
| 20 | LL |  |  | 64 | 132 | 142 | 234 | 275 | 444 | 652 | 913 | 1，230 |  |  |
|  | TL |  |  | 78 | 166 | 180 | 301 | 354 | 577 | 853 | 1，107 | 1，340 | 1，593 | 1，865 |
|  | BRG |  |  | 1.5 ／ 3 | 1.5 ／ 3 | 1.5 ／ 3 | $1.5 / 3$ | 1.5 ／ 3.5 | 2.3 ／ 5.6 | 3.3 ／ 8.3 | 4.3 ／ 10.7 | 5.2 ／ 13 | $6.2 / 15.4$ | $7.2 / 18$ |
| 22 | LL |  |  | 48 | 99 | 107 | 177 | 208 | 336 | 496 | 696 | 940 | 1，230 |  |
|  | TL |  |  | 57 | 123 | 134 | 225 | 265 | 434 | 644 | 909 | 1，104 | 1，312 | 1，537 |
|  | BRG |  |  | 1.5 ／ 3 | 1.5 ／ 3 | 1.5 ／ 3 | 1.5 ／ 3 | 1.5 ／ 3 | 1.9 ／ 4.7 | 2.8 ／ 6.9 | 3.9 ／ 9.7 | 4.7 ／ 11.8 | $5.6 / 14$ | 6.5 ／ 16.4 |
| 24 | LL |  |  |  | 77 | 83 | 137 | 161 | 261 | 385 | 542 | 733 | 962 | 1，230 |
|  | TL |  |  |  | 93 | 101 | 171 | 202 | 333 | 497 | 704 | 924 | 1，099 | 1，287 |
|  | BRG |  |  |  | 1.5 ／ 3 | 1.5 ／ 3 | 1.5 ／ 3 | 1.5 ／ 3 | 1.6 ／ 4 | 2.3 ／ 5.9 | 3.3 ／ 8.3 | 4.3 ／ 10.8 | $5.1 / 12.8$ | 44，727 |
| 26 | LL |  |  |  | 60 | 65 | 108 | 127 | 206 | 305 | 430 | 583 | 766 | 981 |
|  | TL |  |  |  | 71 | 78 | 133 | 157 | 261 | 390 | 555 | 757 | 933 | 1，093 |
|  | BRG |  |  |  | 1.5 ／ 3 | 1.5 ／ 3 | 1.5 ／ 3 | $1.5 / 3$ | 1.5 ／ 3.4 | 44，597 | 2.8 ／ 7.1 | 3.8 ／ 9.6 | 4.7 ／ 11.8 | $5.5 / 13.8$ |
| 28 | LL |  |  |  | 49 | 53 | 87 | 102 | 166 | 245 | 346 | 470 | 619 | 795 |
|  | TL |  |  |  | 55 | 60 | 104 | 124 | 207 | 311 | 443 | 607 | 802 | 939 |
|  | BRG |  |  |  | 1.5 ／ 3 | 1.5 ／ 3 | 1.5 ／ 3 | 1.5 ／ 3 | 1.5 ／ 3 | 1.7 ／ 4.4 | 2.5 ／ 6.2 | 3.3 ／ 8.4 | 4.4 ／ 11 | 5.1 ／ 12.9 |
| 30 | LL |  |  |  |  |  | 71 | 83 | 135 | 200 | 283 | 385 | 508 | 652 |
|  | TL |  |  |  |  |  | 83 | 99 | 166 | 251 | 359 | 493 | 654 | 815 |
|  | BRG |  |  |  |  |  | 1.5 ／ 3 | 1.5 ／ 3 | 1.5 ／ 3 | 1.5 ／ 3.8 | 2.2 ／ 5.4 | 2.9 ／ 7.3 | 3.9 ／ 9.7 | 4.8 ／ 12 |

－The PLF load values in this table are based on the LVL member having lateral bracing at 24＂O．C．or less along its entire length．
－1－3／4＂LVL members 16 ＂and deeper and 1－1／2＂LVL members 14 ＂and deeper，must be a minimum of two plies unless designed by a design professional．Except for ledgers．
－Allowable PLF loads for single or multiple ply $1-1 / 2^{\prime \prime}$ wide LVL members can be obtained by multiplying the table values by 0.85 ．（Required bearing lengths are the same）
－This table may be used for either simple or multiple spans．
－Span is centerline of bearing to centerline of bearing．
－Loads shown can be applied to the beam in addition to its own weight．
－See pages 41 and 42 for details on attaching multiple ply members．
－Allowable loads shown for multiple ply LVL members are also applicable to single billet LVL members with the same width as the combined multiple plies．
－The values shown are based on the lower allowable uniform load for RigidLam LVL produced from Douglas－fir or Southern Pine veneer and therefore can be used for either species．PLF tables separated by species are available on the Roseburg website．

## Key to Table：

LL＝Maximum live load - limits deflection to $\mathrm{L} / 240$
TL＝Maximum total load - limits deflections to L／180
BRG $=$ Required end／interior bearing length（inches），based on bearing stress of 750 PSI．

PLF Tables Douglas fir LVL and Southem Pine LVL
3-PLY 1-3/4" 2.1E RIGIDLAM® LVL - ROOF NON-SNOW (PLF) 125\% LOAD DURATION

| Span (ft.) | Depth | 4-3/8" | 5-1/2" | 7-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-7/8" | 14" | 16" | 18" | 20" | 22" | 24" |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 10 | LL | 168 | 329 | 737 | 1,481 | 1,597 | 2,561 |  |  |  |  |  |  |  |
|  | TL | 217 | 430 | 971 | 1,960 | 2,088 | 2,670 | 2,846 | 3,473 | 4,104 | 4,780 | 5,506 | 6,286 | 7,128 |
|  | BRG | 1.5 / 3 | 1.5 / 3 | 1.5 / 3.1 | 2.5 / 6.3 | 2.7 / 6.8 | 3.4 / 8.5 | 3.6 / 9.1 | 4.4 / 11.1 | 5.2 / 13.1 | $6.1 / 15.3$ | 7/17.6 | $8 / 20.1$ | $9.1 / 22.7$ |
| 12 | LL | 98 | 192 | 433 | 879 | 950 | 1,538 | 1,791 |  |  |  |  |  |  |
|  | TL | 123 | 248 | 567 | 1,158 | 1,251 | 1,988 | 2,200 | 2,783 | 3,267 | 3,779 | 4,319 | 4,892 | 5,500 |
|  | BRG | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.8 / 4.5 | 1.9 / 4.8 | 3.1 / 7.7 | 3.4 / 8.5 | 4.3 / 10.7 | $5 / 12.5$ | $5.8 / 14.5$ | $6.6 / 16.6$ | 7.5 / 18.8 | 8.4 / 21.1 |
| 14 | LL | 62 | 122 | 276 | 563 | 608 | 991 | 1,157 | 1,845 | 2,675 |  |  |  |  |
|  | TL | 76 | 154 | 356 | 736 | 796 | 1,304 | 1,525 | 2,170 | 2,713 | 3,122 | 3,552 | 4,002 | 4,474 |
|  | BRG | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3.3 | 1.5 / 3.6 | 2.3 / 5.9 | 2.7 / 6.9 | 3.9 / 9.7 | 4.9 / 12.2 | 5.6 / 14 | $6.4 / 15.9$ | 7.2 / 17.9 | 44,793 |
| 16 | LL |  | 82 | 186 | 381 | 412 | 674 | 788 | 1,264 | 1,845 | 2,561 |  |  |  |
|  | TL |  | 101 | 237 | 494 | 535 | 882 | 1,033 | 1,657 | 2,110 | 2,610 | 3,014 | 3,384 | 3,770 |
|  | BRG |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.8 / 4.6 | 2.1 / 5.3 | 3.4 / 8.5 | 4.3 / 10.8 | 5.4 / 13.4 | $6.2 / 15.5$ | $6.9 / 17.4$ | 7.7 / 19.3 |
| 18 | LL |  | 58 | 131 | 269 | 291 | 479 | 560 | 902 | 1,322 | 1,845 | 2,475 |  |  |
|  | TL |  | 68 | 164 | 345 | 374 | 621 | 729 | 1,182 | 1,662 | 2,057 | 2,489 | 2,930 | 3,255 |
|  | BRG |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3.6 | 1.7 / 4.3 | 2.8 / 6.9 | 3.9 / 9.6 | 4.8 / 11.9 | 5.8 / 14.4 | $6.8 / 16.9$ | 7.5 / 18.8 |
| 20 | LL |  |  | 96 | 197 | 214 | 352 | 412 | 666 | 979 | 1,370 | 1,845 |  |  |
|  | TL |  |  | 117 | 249 | 270 | 451 | 531 | 866 | 1,280 | 1,661 | 2,010 | 2,389 | 2,797 |
|  | BRG |  |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3.5 | 2.3 / 5.6 | 3.3 / 8.3 | 4.3 / 10.7 | $5.2 / 13$ | $6.2 / 15.4$ | $7.2 / 18$ |
| 22 | LL |  |  | 72 | 149 | 161 | 266 | 311 | 504 | 743 | 1,044 | 1,410 | 1,845 |  |
|  | TL |  |  | 85 | 184 | 200 | 337 | 397 | 651 | 967 | 1,364 | 1,656 | 1,969 | 2,305 |
|  | BRG |  |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.9 / 4.7 | 2.8 / 6.9 | 3.9/9.7 | 4.7 / 11.8 | $5.6 / 14$ | 6.5 / 16.4 |
| 24 | LL |  |  | 56 | 115 | 125 | 205 | 241 | 391 | 578 | 813 | 1,100 | 1,443 | 1,845 |
|  | TL |  |  | 63 | 139 | 152 | 257 | 303 | 500 | 746 | 1,056 | 1,387 | 1,649 | 1,931 |
|  | BRG |  |  | 1.5 / 3 | 1.5 / 3 | $1.5 / 3$ | 1.5 / 3 | 1.5 / 3 | 1.6 / 4 | 2.3 / 5.9 | 3.3 / 8.3 | $4.3 / 10.8$ | $5.1 / 12.8$ | 44,727 |
| 26 | LL |  |  |  | 91 | 98 | 162 | 190 | 309 | 457 | 645 | 874 | 1,149 | 1,472 |
|  | TL |  |  |  | 107 | 116 | 199 | 236 | 391 | 585 | 832 | 1,135 | 1,400 | 1,640 |
|  | BRG |  |  |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3.4 | 44,597 | 2.8 / 7.1 | 3.8 / 9.6 | 4.7 / 11.8 | $5.5 / 13.8$ |
| 28 | LL |  |  |  | 73 | 79 | 130 | 153 | 249 | 368 | 520 | 706 | 929 | 1,192 |
|  | TL |  |  |  | 83 | 90 | 156 | 186 | 310 | 466 | 665 | 910 | 1,202 | 1,409 |
|  | BRG |  |  |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | $1.5 / 3$ | 1.7 / 4.4 | 2.5 / 6.2 | 3.3 / 8.4 | $4.4 / 11$ | $5.1 / 12.9$ |
| 30 | LL |  |  |  | 59 | 64 | 106 | 125 | 203 | 301 | 425 | 578 | 761 | 979 |
|  | TL |  |  |  | 65 | 71 | 124 | 148 | 249 | 376 | 539 | 739 | 982 | 1,223 |
|  | BRG |  |  |  | 1.5 / 3 | 1.5 / 3 | $1.5 / 3$ | 1.5 / 3 | 1.5 / 3 | 1.5 / 3.8 | $2.2 / 5.4$ | $2.9 / 7.3$ | 3.9 / 9.7 | 4.8 / 12 |
| 32 | LL |  |  |  | 49 | 53 | 88 | 103 | 168 | 249 | 352 | 479 | 632 | 813 |
|  | TL |  |  |  | 51 | 56 | 100 | 119 | 202 | 307 | 441 | 607 | 808 | 1,047 |
|  | BRG |  |  |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3.4 | 1.9 / 4.8 | 2.6 / 6.5 | 3.4 / 8.6 | 4.4 / 11 |
| 34 | LL |  |  |  |  |  | 73 | 86 | 140 | 208 | 294 | 401 | 529 | 682 |
|  | TL |  |  |  |  |  | 80 | 96 | 165 | 253 | 365 | 504 | 672 | 872 |
|  | BRG |  |  |  |  |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.7 / 4.2 | 2.3 / 5.8 | $3 / 7.6$ | 3.9 / 9.8 |
| 36 | LL |  |  |  |  |  | 62 | 72 | 118 | 176 | 249 | 339 | 448 | 578 |
|  | TL |  |  |  |  |  | 65 | 78 | 136 | 210 | 304 | 421 | 564 | 733 |
|  | BRG |  |  |  |  |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3.8 | $2.1 / 5.2$ | 2.7 / 6.8 | 3.5 / 8.8 |

4-PLY 1-3/4" 2.1E RIGIDLAM® LVL - ROOF NON-SNOW (PLF) 125\% LOAD DURATION

| Span (ft.) | Depth | 4-3/8" | 5-1/2" | 7-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-7/8" | 14" | 16" | 18" | 20" | 22" | 24" |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 10 | LL | 223 | 439 | 982 | 1,974 | 2,129 | 3,415 |  |  |  |  |  |  |  |
|  | TL | 289 | 574 | 1,295 | 2,614 | 2,784 | 3,559 | 3,795 | 4,630 | 5,472 | 6,374 | 7,341 | 8,381 | 9,504 |
|  | BRG | 1.5 / 3 | $1.5 / 3$ | 1.5 / 3.1 | 2.5 / 6.3 | 2.7 / 6.8 | 3.4 / 8.5 | 3.6 / 9.1 | 4.4 / 11.1 | 5.2 / 13.1 | $6.1 / 15.3$ | $7 / 17.6$ | $8 / 20.1$ | 9.1 / 22.7 |
| 12 | LL | 130 | 256 | 578 | 1,173 | 1,266 | 2,051 | 2,389 |  |  |  |  |  |  |
|  | TL | 165 | 331 | 756 | 1,544 | 1,669 | 2,651 | 2,933 | 3,711 | 4,357 | 5,038 | 5,759 | 6,523 | 7,333 |
|  | BRG | 1.5 / 3 | $1.5 / 3$ | 1.5 / 3 | 1.8 / 4.5 | 1.9 / 4.8 | 3.1 / 7.7 | 3.4 / 8.5 | 4.3 / 10.7 | $5 / 12.5$ | $5.8 / 14.5$ | 6.6 / 16.6 | 7.5 / 18.8 | 8.4 / 21.1 |
| 14 | LL | 82 | 162 | 368 | 750 | 811 | 1,322 | 1,543 | 2,460 | 3,567 |  |  |  |  |
|  | TL | 101 | 205 | 475 | 981 | 1,062 | 1,739 | 2,033 | 2,894 | 3,617 | 4,163 | 4,736 | 5,336 | 5,966 |
|  | BRG | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3.3 | 1.5 / 3.6 | $2.3 / 5.9$ | 2.7 / 6.9 | 3.9 / 9.7 | 4.9 / 12.2 | $5.6 / 14$ | $6.4 / 15.9$ | 7.2 / 17.9 | 44,793 |
| 16 | LL | 55 | 109 | 248 | 508 | 549 | 899 | 1,051 | 1,686 | 2,460 | 3,415 |  |  |  |
|  | TL | 65 | 134 | 316 | 658 | 713 | 1,176 | 1,377 | 2,209 | 2,813 | 3,481 | 4,019 | 4,512 | 5,026 |
|  | BRG | 1.5 / 3 | $1.5 / 3$ | $1.5 / 3$ | 1.5 / 3 | 1.5 / 3 | 1.8 / 4.6 | $2.1 / 5.3$ | 3.4 / 8.5 | 4.3 / 10.8 | 5.4 / 13.4 | 6.2 / 15.5 | 6.9 / 17.4 | 7.7 / 19.3 |
| 18 | LL |  | 77 | 175 | 359 | 389 | 638 | 747 | 1,203 | 1,763 | 2,460 | 3,300 |  |  |
|  | TL |  | 91 | 218 | 460 | 499 | 828 | 972 | 1,576 | 2,216 | 2,742 | 3,319 | 3,907 | 4,340 |
|  | BRG |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | $1.5 / 3.6$ | 1.7 / 4.3 | $2.8 / 6.9$ | 3.9 / 9.6 | 4.8 / 11.9 | 5.8 / 14.4 | $6.8 / 16.9$ | 7.5 / 18.8 |
| 20 | LL |  | 56 | 128 | 263 | 285 | 469 | 549 | 887 | 1,305 | 1,827 | 2,460 |  |  |
|  | TL |  | 64 | 156 | 332 | 360 | 602 | 708 | 1,155 | 1,707 | 2,214 | 2,680 | 3,186 | 3,729 |
|  | BRG |  | $1.5 / 3$ | $1.5 / 3$ | $1.5 / 3$ | 1.5 / 3 | 1.5 / 3 | 1.5 / 3.5 | $2.3 / 5.6$ | 3.3 / 8.3 | 4.3 / 10.7 | 5.2 / 13 | 6.2 / 15.4 | 7.2 / 18 |
| 22 | LL |  |  | 96 | 199 | 215 | 354 | 415 | 672 | 991 | 1,392 | 1,880 | 2,460 |  |
|  | TL |  |  | 114 | 246 | 267 | 449 | 529 | 868 | 1,289 | 1,819 | 2,208 | 2,625 | 3,073 |
|  | BRG |  |  | $1.5 / 3$ | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.9 / 4.7 | 2.8 / 6.9 | 3.9 / 9.7 | 4.7 / 11.8 | 5.6 / 14 | $6.5 / 16.4$ |
| 24 | LL |  |  | 74 | 153 | 166 | 274 | 321 | 521 | 770 | 1,083 | 1,467 | 1,924 | 2,460 |
|  | TL |  |  | 84 | 186 | 202 | 342 | 404 | 667 | 994 | 1,408 | 1,849 | 2,198 | 2,575 |
|  | BRG |  |  | $1.5 / 3$ | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.6 / 4 | 2.3 / 5.9 | 3.3 / 8.3 | 4.3 / 10.8 | 5.1 / 12.8 | 44,727 |
| 26 | LL |  |  | 59 | 121 | 131 | 216 | 254 | 412 | 610 | 859 | 1,166 | 1,532 | 1,963 |
|  | TL |  |  | 63 | 142 | 155 | 265 | 314 | 521 | 780 | 1,109 | 1,513 | 1,867 | 2,187 |
|  | BRG |  |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | $1.5 / 3$ | 1.5 / 3 | 1.5 / 3.4 | 44,597 | $2.8 / 7.1$ | 3.8 / 9.6 | 4.7 / 11.8 | 5.5 / 13.8 |
| 28 | LL |  |  |  | 97 | 105 | 174 | 204 | 331 | 491 | 693 | 941 | 1,239 | 1,590 |
|  | TL |  |  |  | 110 | 121 | 208 | 247 | 413 | 622 | 887 | 1,214 | 1,603 | 1,879 |
|  | BRG |  |  |  | $1.5 / 3$ | $1.5 / 3$ | $1.5 / 3$ | 1.5 / 3 | 1.5 / 3 | 1.7 / 4.4 | 2.5 / 6.2 | 3.3 / 8.4 | 4.4 / 11 | $5.1 / 12.9$ |
| 30 | LL |  |  |  | 79 | 86 | 141 | 166 | 270 | 401 | 566 | 770 | 1,015 | 1,305 |
|  | $\mathrm{TL}$ |  |  |  | $86$ | $95$ | $166$ | $197$ | $332$ | $502$ | $718$ | $986$ | $1,309$ | $1,630$ |
|  | BRG |  |  |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3.8 | 2.2 / 5.4 | 2.9 / 7.3 | $3.9 / 9.7$ | $4.8 / 12$ |
| 32 | LL |  |  |  | 65 | 71 | 117 | 137 | 223 | 331 | 469 | 638 | 842 | 1,083 |
|  | $\overline{\mathrm{TL}}$ |  |  |  | $68$ | 75 | 133 | 159 | 269 | 409 | 588 | 810 | 1,078 | 1,396 |
|  | BRG |  |  |  | 1.5 / 3 | $1.5 / 3$ | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3.4 | $1.9 / 4.8$ | 2.6 / 6.5 | 3.4 / 8.6 | 4.4 / 11 |
| 34 | LL |  |  |  | 54 | 59 | 97 | 114 | 187 | 277 | 392 | 534 | 706 | 909 |
|  | TL |  |  |  | 54 | 59 | 107 | 128 | 220 | 337 | 486 | 672 | 896 | 1,163 |
|  | BRG |  |  |  | $1.5 / 3$ | $1.5 / 3$ | 1.5 / 3 | 1.5 / 3 | $1.5 / 3$ | 1.5 / 3 | 1.7 / 4.2 | $2.3 / 5.8$ | $3 / 7.6$ | 3.9 / 9.8 |
| 36 | LL |  |  |  |  |  | 82 | 97 | 158 | 234 | 331 | 452 | 597 | 770 |
|  | TL |  |  |  |  |  | 87 | 105 | 181 | 279 | 405 | 562 | 752 | 978 |
|  | BRG |  |  |  |  |  | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3 | 1.5 / 3.8 | $2.1 / 5.2$ | 2.7 / 6.8 | 3.5 / 8.8 |

Refer to notes on previous page.

## RigidLam ${ }^{\circledR}$ LVL Studs*

*Currently, only Douglas-fir (Mill \#1055) RigidLam LVL, grades 1.6E through 2.1E, have been qualified for use in conventional or engineered stud wall construction.
Although conventional construction methods have allowed builders to meet the needs of homeowners, they are constantly being challenged with the need for straighter, stronger and taller wall framing components. Roseburg Forest Products RigidLam ${ }^{\circledR}$ LVL studs are an answer to the needs of both homeowners and builders. RigidLam studs are manufactured to the industry's highest standards and unlike solid-sawn lumber, RigidLam studs are straight, strong, and stiff, resulting in a faster installation time, fewer callbacks, and straight walls that give homeowners peace of mind.

## FIRE RATED STUD WALL APPLICATIONS

Conventional Stud Wall Construction: RigidLam studs are permitted to be used in fire-resistance-rated conventional wall construction and are considered to be a direct replacement for solid-sawn lumber, having the same dimensions, in any fire-resistance-rated wall assembly listed in Table 721.1 (2) of the IBC. A minimum of 2.5 pcf of mineral wool insulation must be placed in the stud cavity.

Engineered Stud Wall Construction: See APA Product Report PR-L289 for additional limitations and design value adjustments when using RigidLam studs in fire-resistance-rated engineered wall construction. PR-L289 can be found on the Roseburg website (www.roseburg.com) in the Engineered Wood Products section or on the APA website (www.apawood.org).

## CONVENTIONAL CONSTRUCTION

Based on testing conducted in accordance with ICC Evaluation Service Acceptance Criteria for Wood-Based Studs, AC202, RigidLam LVL studs are considered to be alternatives to sawn lumber studs complying with Section 2308.5 of the IBC, and Section R602 of the IRC.

## CONVENTIONAL WALL CONSTRUCTION - STUD NOTCHING AND HOLE GUIDELINES

Maximum hole diameter shall not exceed $40 \%$ of stud depth (60\% for non-bearing walls)


Notch depth "d" must not exceed $25 \%$ of stud depth ( $40 \%$ for non-bearing walls)


5/8" minimum distance to edge of hole

Double stud required where hole is between $40 \%$ and $60 \%$ of stud depth (no more than two double studs in a row with such holes). Not required for non-bearing walls.

## engineered construction

For building applications that fall outside the scope of conventional construction, RigidLam LVL studs may be used provided they are designed in accordance with accepted engineering practice. RigidLam LVL studs are available in 1.6 E and 2.1 E grades in widths of $1-1 / 2$ " and $1-3 / 4$ ".

## RIGIDLAM® LVL STUD ALLOWABLE DESIGN STRESSES VS. SOLID-SAWN LUMBER ${ }^{(1)(a)}$

| 2x4 |  | Joist (edgewise) |  |  | Plank (flatwise) |  |  | Axial |  | MOE |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | $F_{\text {b }}$ | $\mathrm{F}_{\mathrm{v}}$ | $\mathrm{Fc}_{\perp}{ }^{(2)}$ | $\mathrm{F}_{\mathrm{b}}$ | $\mathrm{F}_{\mathrm{v}}$ | $\mathrm{Fc}_{\perp}{ }^{(2)}$ | $\mathrm{F}_{\mathrm{c}}$ | $\mathrm{F}_{\mathrm{t}}$ |  |
| Species | Grade | (psi) | (psi) | (psi) | (psi) | (psi) | (psi) | (psi) | (psi) | (psi) |
| RigidLam LVL Stud | 1.6E | 2,730 ${ }^{(4)}$ | 220 | 575 | 2,250 | 130 | 650 | 1,950 | 1,500 ${ }^{(3)}$ | 1,600,000 |
| RigidLam LVL Stud | 2.1E | 3,761 ${ }^{(4)}$ | 290 | 750 | 3,100 | 130 | 650 | 3,000 | 2,100 ${ }^{(3)}$ | 2,100,000 |
| Douglas-fir ${ }^{(b)}$ | No. 2 | $1,553^{(c)}$ | 180 | 625 | $1,485{ }^{\text {(d) }}$ | 180 | 625 | 1,553 ${ }^{(\mathrm{e})}$ | $863{ }^{(\text {e) }}$ | 1,600,000 |
| Spruce-Pine-Fir ${ }^{(\mathrm{b})}$ | No. 2 | 1,509 ${ }^{\text {c }}$ ) | 135 | 425 | $1,444{ }^{(d)}$ | 135 | 425 | 1,323 ${ }^{(\mathrm{e})}$ | $675^{(\mathrm{e})}$ | 1,400,000 |
|  |  |  |  |  |  |  |  |  |  |  |
| 6 |  |  | (edge |  |  | $k$ (flat |  |  |  | OEF |
| 6 |  | $F_{\text {b }}$ | $\mathrm{F}_{\mathrm{v}}$ | $\mathrm{Fc}_{ \pm}{ }^{(2)}$ | $F_{\text {b }}$ | $\mathrm{F}_{\mathrm{v}}$ | $\mathrm{Fc}_{\perp}{ }^{(2)}$ | $\mathrm{F}_{\mathrm{c}}$ | $F_{\text {t }}$ | , |
| Species | Grade | (psi) | (psi) | (psi) | (psi) | (psi) | (psi) | (psi) | (psi) | (psi) |
| RigidLam LVL Stud | 1.6E | 2,580 ${ }^{(4)}$ | 220 | 575 | 2,250 | 130 | 650 | 1,950 | 1,500 ${ }^{(3)}$ | 1,600,000 |
| RigidLam LVL Stud | 2.1E | 3,554 ${ }^{(4)}$ | 290 | 750 | 3,100 | 130 | 650 | 3,000 | 2,100 ${ }^{(3)}$ | 2,100,000 |
| Douglas-fir ${ }^{(b)}$ | No. 2 | 1,346 ${ }^{(c)}$ | 180 | 625 | 1,346 ${ }^{\text {d) }}$ | 180 | 625 | 1,485 ${ }^{(\mathrm{e})}$ | $748{ }^{(\mathrm{e})}$ | 1,600,000 |
| Spruce-Pine-Fir ${ }^{(\mathrm{b})}$ | No. 2 | 1,308 ${ }^{(c)}$ | 135 | 425 | $1,308^{\text {d }}$ | 135 | 425 | 1,265 ${ }^{(\text {e) }}$ | $585{ }^{(\text {e) }}$ | 1,400,000 |

## RigidLam LVL Notes

1. These allowable design stresses apply to dry service conditions
2. Duration of Load increases not allowed
3. Tabulated values are based on a 4 ft length. For lengths greater than 4 ft , multiply by $(4 / \text { Length })^{1 / 9}$. For lengths less than 4 ft , use the table values.
4. Bending values have been multiplied by $(12 / \mathrm{d})^{1 / 8}$ and a repetitive member factor of 1.04

## Solid-Sawn Notes

a. These allowable design stresses apply to dry service conditions
b. Solid-sawn design values taken from 2018 National Design Specification
c. $F_{b}$ has been adjusted for repetitive member use and size factor increases
d. $F_{b}$ has been adjusted for size factor increases and flat-use increases
e. $F_{c}$ and $F_{t}$ have been adjusted for size factor increases

## ENGINEERED WALL CONSTRUCTION - RIGIDLAM STUD HOLE AND NOTCHING GUIDELINES

Notches: A notch up to $40 \%$ of the depth of the stud may be placed anywhere along the stud provided the reduced section is accounted for using standard engineering analysis and the allowable bending and/or tension stress is reduced by $30 \%$ to account for the stress concentrations that occur at the corners of the notch.

Holes: A hole with a maximum diameter of $30 \%$ of the depth of the stud may be placed anywhere along the stud at the centerline of the stud depth without further engineering analysis for lateral bending considerations. For other conditions, holes may be placed anywhere along the stud provided the reduced section is accounted for using standard engineering analysis.

## CONVENTIONAL AND ENGINEERED WALL CONSTRUCTION - RIGIDLAM LVL NAILING RESTRICTIONS

## Nailing Restrictions for Single <br> Stud at Adjoining Panel Edges

## Nailing Restrictions for Double <br> Studs at Adjoining Panel Edges



| 1.4E RigidLam LVL |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 1-1/2" Wide |  |  |  |  |  |
| Gross Stringer Depth | Tread Width |  |  |  |  |
|  | 36" |  | 42" | 44" | 48" |
|  | 2 Stringers | 3 Stringers | 3 Stringers | 3 Stringers | 3 Stringers |
| 40 psf Live Load and 12 psf Dead Load |  |  |  |  |  |
| 9-1/2" | 4'-10" | 5'-5" | 5'-2" | 5'-1" | 5'-0" |
| 11-7/8" | 8'-8" | 9'-10" | 9'-4" | $9{ }^{\prime}-3$ " | 9'-0" |
| 14 " | 12'-2" | 13'-9" | 13'-1" | 12'-11" | 12'-7" |
| 16" | 15'-5" | 17'-5" | 16'-7" | 16'-5" | 15'-11" |
| 100 psf Live Load and 12 psf Dead Load |  |  |  |  |  |
| 9-1/2" | 4'-3" | 4'-9" | 4'-7" | 4'-6" | 4'-5" |
| 11-7/8" | 7'-3" | 8'-2" | 7'-9" | 7'-8" | 7'-6" |
| 14" | 9'11" | 11'-2" | 10'-8" | 10'-6" | 10'-3" |
| 16 " | 12'-5" | 14'-0" | 13'-5" | $13^{\prime}-3^{\prime \prime}$ | 12'-11" |
| 1.6E RigidLam LVL |  |  |  |  |  |
| 1-1/2" Wide |  |  |  |  |  |
| Gross Stringer Depth | Tread Width |  |  |  |  |
|  | 36" |  | $\frac{42^{\prime \prime}}{3 \text { Stringers }}$ | 44" <br> 3 Stringers | 48" |
|  | 2 Stringers | 3 Stringers |  |  | 3 Stringers |
| 40 psf Live Load and 12 psf Dead Load |  |  |  |  |  |
| 9-1/2" | 5'-0" | 5'-8" | 5'-5" | 5'-4" | 5'-2" |
| 11-7/8" | 9'-1" | 10'-3" | 9'-9" | 9'-8" | 9'-5" |
| 14 " | 12'-8" | 14'-4" | 13'-8" | 13'-6" | 13'-2" |
| 16 " | 16'1" | 18'-2" | 17'-4" | 17'-1" | $16^{\prime}-8$ " |
| 100 psf Live Load and 12 psf Dead Load |  |  |  |  |  |
| 9-1/2" | 4'-5" | 5'-0" | 4'-9" | 4'-8" | 4'-7" |
| 11-7/8" | $7{ }^{\prime}-7$ | 8'-6" | 8'-2" | $8{ }^{\prime}-0$ | 7'-10" |
| 14 " | 10'-4" | 11'-8" | 11'-2" | 11'-0" | 10'-8" |
| 16 " | 13'-0" | 14'-8" | 14'-0" | 13'-9" | 13 '-5" |
| 2.1E RigidL_am LVL |  |  |  |  |  |
| 1-1/2" Wide |  |  |  |  |  |
| Gross Stringer Depth | Tread Width |  |  |  |  |
|  | 36" |  | $\underset{3 \text { Stringers }}{42 "}$ | 44" <br> 3 Stringers | 48" |
|  | 2 Stringers | 3 Stringers |  |  | 3 Stringers |
| 40 psf Live Load and 12 psf Dead Load |  |  |  |  |  |
| 9-1/2" | 5'-6" | 6'-2" | 5'-11' | 5'-10" | 5'-8" |
| 11-7/8" | 9'-11" | 11'-3" | 10'-8" | 10'-6" | 10'-3" |
| 14 " | 13'-10" | 15 '-8" | 15'-0" | 14'-9" | 14'-4" |
| 16 " | 17'-7" | 19'-10" | 19'-0" | 18'-9" | 18'-3" |
| 100 psf Live Load and 12 psf Dead Load |  |  |  |  |  |
| 9-1/2" | 4'-10" | 5'-5" | 5'-2" | 5'-1" | 5'-0" |
| 11-7/8" | 8'-3" | 9'-3" | 8'-10" | 8'-9" | 8'-6" |
| 14 " | 11'-3" | 12'-9" | 12'-2" | 12'-0" | 11'-8" |
| 16 " | 14'-2" | 15'-11" | 15 '-3" | 15'-0" | $14^{\prime}-8{ }^{\prime \prime}$ |

## How To Use Tables

1. Determine grade and width of Roseburg RigidLam LVL
2. Locate appropriate table
3. Locate appropriate load ( 40 or 100 psf live load)
4. Locate appropriate gross depth of LVL ( $9-1 / 2^{\prime \prime}, 11-7 / 8^{\prime \prime}, 14$ " or 16 ")
5. Determine maximum allowable horizontal stringer run based on tread width and number of stringers

## General Notes

- For 40/12 loading (residential), stringer runs are based on a rise of 7-3/4" (maximum per 2021 IRC) and a run of $11^{\prime \prime}$ (1" longer than minimum run of 10 " per 2021 IRC).
- For 100/12 loading (commercial), stringer runs are based on a rise of 7" (maximum per 2021 IBC) and a run of $11^{\prime \prime}$ (minimum per 2021 IBC).
- Consult a design professional for allowable stringer run if above rise and/or run values are exceeded.


## INSTALLATION GUIDELINES



DO NOT notch or drill holes in stringer


DO NOT overcut stringer. Use hand saw to finish cut

| 1.4E RigidLam LVL |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 1-3/4" Wide |  |  |  |  |  |
| Gross Stringer Depth | Tread Width |  |  |  |  |
|  | 36" |  | 42" | 44" | 48 |
|  | 2 Stringers | 3 Stringers | 3 Stringers | 3 Stringers | 3 Stringers |
| 40 psf Live Load and 12 psf Dead Load |  |  |  |  |  |
| 9-1/2" | 5'-0" | 5'-8" | 5'-5" | 5'-4" | 5'-3' |
| 11-7/8" | 9'-1" | 10'-3" | 9'-10" | 9'-8" | 9'-5" |
| 14" | 12'-9" | 14'-4" | 13'-9" | 13'-6" | 13'-2" |
| 16 " | 16'-2" | 18'-2" | 17'-5" | 17'-2" | 16'-9" |
| 1.00 psf Live Load and 1.2 psf Dead Load |  |  |  |  |  |
| 9-1/2" | 4'-5" | 5'-0" | 4'-9" | 4'-9" | 4'-7" |
| 11-7/8" | 7'-7" | 8'-6" | 8'-2' | 8'-1" | 7'-10" |
| 14" | 10'-5" | 11'-8" | 11'-2" | 11'-0" | 10'-9" |
| 16 " | 13'-0" | 14'-8" | 14'-0" | 13'-10" | 13'-6" |
| 1,6E RigidL_am LVL |  |  |  |  |  |
| 1-3/4" Wide |  |  |  |  |  |
| Gross Stringer Depth | Tread Width |  |  |  |  |
|  | 36" |  | 42" | 44" | 48" |
|  | 2 Stringers | 3 Stringers | 3 Stringers | 3 Stringers | 3 Stringers |
| 40 psf Live Load and 12 psf Dead Load |  |  |  |  |  |
| 9-1/2" | 5'-3" | 5'-11" | 5'-8" | 5'-7" | 5'-5" |
| 11-7/8" | 9'-6" | 10'-9" | 10'-3" | 10'-1" | 9'-10" |
| 14" | 13'-3" | 15'-0" | 14'-4" | 14'-2" | 13'-9" |
| 16" | 16'-10" | 18'-11" | 18'-2" | 17'-11" | 17'-6" |
| 100 psf Live Load and 12 psf Dead Load |  |  |  |  |  |
| 91/2" | 4'-8" | 5'-3' | 5'-0" | 4'-11" | 4'-10" |
| 11-7/8" | 7'-11" | 8'-11" | 8'-6" | 8'-5" | 8'-2" |
| 14" | 10'-10" | 12'-3" | 11'-8" | 11'-6" | 11'-3" |
| $16 "$ | 13'-7" | 15'-4" | 14'-8" | 14'-5" | 14'-1" |
| 2,1E RigidLam LVL |  |  |  |  |  |
| 1-3/4" Wide |  |  |  |  |  |
| Gross Stringer Depth | Tread Width |  |  |  |  |
|  | 36" |  | 42" | 44" | 48" |
|  | 2 Stringers | 3 Stringers | 3 Stringers | 3 Stringers | 3 Stringers |
| 40 psf Live Load and 12 psf Dead Load |  |  |  |  |  |
| 9-1/2" | 5'-9" | 6'-6" | 6'-2" | 6'-1" | 5'-11" |
| 11-7/8" | 10'-4" | 11'-9" | 11'-3" | 11'-1" | 10'-9" |
| 14" | 14'-6" | 16'-5" | 15'-8" | 15'-6" | 15'-1" |
| 16" | 18'-5" | 20'-9" | 19'-10" | 19'-7" | 19'-1" |
| 100 psf Live Load and 12 psf Dead Load |  |  |  |  |  |
| 9-1/2" | 5'-1" | 5'-8" | 5'-5" | 5'-4" | 5'-3" |
| 11-7/8" | 8'-7" | 9'-9" | 9'-3' | 9'-2" | 8'-11" |
| 14" | 11'-10" | 13'-4" | 12'-9" | 12'-7" | 12'-3" |
| 16" | 14'-10" | 16'-9" | 15'-11" | 15'-9" | 15'-4" |

- Stringer runs are based on deflection criteria of L/360 Live Load and L/240 Total Load.
- All stringer runs are based on a $100 \%$ duration of load.
- Stringer runs account for self-weight of member.
- Stringers are unstable until connections at low and high ends are completed and treads are attached.
- Use subfloor adhesive to minimize squeaks and improve stair performance.
- When stringer is in direct contact with concrete, use moisture barrier.
- Refer to appropriate building code for story height restrictions.
- For loading and/or framing conditions outside the scope of this document, consult a design professional.
- Refer to pages 7 and 36 for RigidLam LVL storage and handling information.
- RigidLam LVL Stair Stringers shall not be used for exterior conditions exposed to elements.


## RigidLam LVL Code Evaluation ICC ESR-1210



DO NOT walk on stringers until treads are attached

RIGIDLAM® LVL ALLOWABLE DESIGN STRESSES ${ }^{1}$

|  |  | 1.4E RigidLam LVL | 1.6E RigidLam LVL | 2.1E RigidLam LVL |
| :---: | :---: | :---: | :---: | :---: |
| True Modulus of Elasticity (MOE) ${ }^{2}$ - Edgewise or Flatwise | $E$ (psi) $=$ | 1,400,000 | 1,600,000 | 2,100,000 |
| Apparent Modulus of Elasticity (MOE) ${ }^{2}$ - Edgewise or Flatwise | $E(\mathrm{psi})=$ | 1,300,000 | 1,500,000 | 2,000,000 |
| Bending-Edgewise ${ }^{3,4}$ | $\mathrm{F}_{\mathrm{b}}$ edge (psi)= | 2,250 | 2,250 | 3,100 |
| Bending - Flatwise ${ }^{5}$ | $\mathrm{F}_{\mathrm{b}}$ flat (psi)= | 2,250 | 2,250 | 3,100 |
| Horizontal Shear - Edgewise | $\mathrm{F}_{\mathrm{v}}$ edge (psi) $=$ | 200 | 220 | 290 |
| Horizontal Shear - Flatwise | $\mathrm{F}_{\mathrm{V}}$ flat (psi) $=$ | 130 | 130 | 130 |
| Compression Perp. To Grain ${ }^{2}$ - Edgewise | $\mathrm{F}_{\mathrm{C} \text { perp }}$ edge (psi) $=$ | 560 | 575 | 750 |
| Compression Perp. To Grain ${ }^{2}$ - Flatwise | $\mathrm{F}_{\text {cperp flat }}(\mathrm{psi})=$ | 650 | 650 | 650 |
| Compression Parallel to Grain | $\mathrm{F}_{\mathrm{C} \text { para }}(\mathrm{psi})=$ | 1,950 | 1,950 | 3,000 |
| Tension Parallel to Grain ${ }^{6}$ | $\mathrm{F}_{\mathrm{t}}(\mathrm{psi})=$ | 1,500 | 1,500 | 2,100 |
| MOE for stability calculations ${ }^{2}$ | $\mathrm{E}_{\text {min }}(\mathrm{psi})=$ | 687,023 | 792,718 | 1,056,958 |

1. These allowable design stresses apply to dry service conditions.
. No increase is allowed for duration of load.
2. For depths other than 12 ", multiply $F_{b}$ by (12/d) $)^{1 / 8}$ for DF (Mill \#1055) or (12/d) $)^{1 / 5}$ for SP (Mill \#1125), where $\mathrm{d}=$ depth of member (inches).
3. A factor of 1.04 may be applied for repetitive members as defined in the National Design Specification for Wood Construction.
4. Tabulated $F_{b}$ flat values are based on a width of $1-3 / 4$ ". For other widths, when loaded flatwise, multiply $\mathrm{F}_{\mathrm{b}}$ flat by $(1.75 / \mathrm{t})^{1 / 5}$, where t is the LVL width in inches. For widths less than $1-3 / 4$ ", use the tabulated value.
5. Tensile stress is based on a 4 -foot gage length. For greater lengths, multiply $F_{t}$ by $(4 / L)^{1 / 9}$ where $L=$ length in feet. For lengths less than 4 -feet, use the published value.

## STAIR STRINGER TERMS AND DEFINITIONS



STAIR STRINGER CONFIGURATIONS


## CONNECTION DETAILS - 40 PSF LIVE LOAD \& 12 PSF DEAD LOAD [FOR HIGHER LOADING, CONSULT DESIGN PROFESSIONAL]



High End


NOTE: Only use fasteners approved for use with the corresponding wood treatment.


| FACE MOUNT HANGERS |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Single I-Joist |  |  |  | Double l-Joist |  |  |  |
| Width | Depth | Hanger | $\begin{gathered} \text { Down } \\ \text { Looad } \end{gathered}$ | Width | Depth | Hanger | Down |
| 1-3/4" | 9-1/2" | IUS1.81/9.5 | 950 | 3-1/2" | 9-1/2" | MIU3.56/9 | 2,305 |
|  | 11-7/8" | IUS1.81/11.88 | 1,185 |  | 11-7/8" | MIU3.56/11 | 2,880 |
|  | 14 " | IUS1.81/14 | 1,420 |  | $14^{\prime \prime}$ | MIU3.56/14 | 3,170 |
|  | $16^{\prime \prime}$ | IUS1.81/16 | 1,660 |  | 16 " | MIU3.56/16 | 3,455 |
| 2-1/16" | 9-1/2" | IUS2.06/9.5 | 950 | 4-1/8" | 9-1/2" | MIU4.28/9 | 2,305 |
|  | 11-7/8" | IUS2.06/11.88 | 1,185 |  | 11-7/8" | MIU4.28/11 | 2,880 |
|  | $14{ }^{\prime \prime}$ | IUS2.06/14 | 1,420 |  | 14 " | MIU4.28/14 | 3,170 |
|  | $16{ }^{\prime \prime}$ | IUS2.06/16 | 1,660 |  | 16 " | MIU4.28/16 | 3,455 |
| 2-5/16" | 9-1/2" | IUS2.37/9.5 | 950 | 4-5/8" | 9-1/2" | MIU4.75/9 | 2,305 |
|  | 11-7/8" | IUS2.37/11.88 | 1,185 |  | 11-7/8" | MIU4.75/11 | 2,880 |
|  | 14 " | IUS2.37/14 | 1,420 |  | 14 " | MIU4.75/14 | 3,170 |
|  | $16^{\prime \prime}$ | IUS2.37/16 | 1,660 |  | 16 " | MIU4.75/16 | 3,455 |
| 2-1/2" | 9-1/2" | IUS2.56/9.5 | 950 | 5" | 9-1/2" | MIU5.12/9 | 2,305 |
|  | 11-7/8" | IUS2.56/11.88 | 1,185 |  | 11-7/8" | MIU5.12/11 | 2,880 |
|  | $14{ }^{\prime \prime}$ | IUS2.56/14 | 1,420 |  | 14 " | MIU5.12/14 | 3,170 |
|  | 16 " | IUS2.56/16 | 1,660 |  | 16 " | MIU5.12/16 | 3,455 |
| 3-1/2" | 9-1/2" | IUS3.56/9.5 | 1,185 | $7{ }^{\prime}$ | 9-1/2" | HU410-2 | 2,680 |
|  | 11-7/8" | IUS3.56/11.88 | 1,420 |  | 11-7/8" | HU412-2 | 3,275 |
|  | 14 " | IUS3.56/14 | 1,420 |  | 14 " | HU414-2 | 3,870 |
|  | $16^{\prime \prime}$ | IUS3.56/16 | 1,660 |  | 16 " | HU414-2 | 3,870 |

## TOP FLANGE HANGERS

| Single I-Joist |  |  |  | Double I-Joist |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Width | Depth | Hanger | Down Load | Width | Depth | Hanger | Down Load |
| 1-3/4" | 9-1/2" | ITS1.81/9.5 | 1,550 | 3-1/2" | 9-1/2" | MIT49.5 | 2,550 |
|  | 11-7/8" | ITS1.81/11.88 | 1,550 |  | 11-7/8" | MIT411.88 | 2,550 |
|  | 14 " | ITS1.81/14 | 1,550 |  | 14 " | MIT414 | 2,550 |
|  | 16 " | ITS1.81/16 | 1,550 |  | 16 " | MIT416 | 2,550 |
| 2-1/16" | 9-1/2" | ITS2.06/9.5 | 1,550 | 4-1/8" | 9-1/2" | MIT4.28/9.5 | 2,550 |
|  | 11-7/8" | ITS2.06/11.88 | 1,550 |  | 11-7/8" | MIT4.28/11.88 | 2,550 |
|  | 14 " | ITS2.06/14 | 1,550 |  | 14" | MIT4.28/14 | 2,550 |
|  | 16 " | ITS2.06/16 | 1,550 |  | 16 " | BA4.28/16 | 4,715 |
| 2-5/16" | 9-1/2" | ITS2.37/9.5 | 1,550 | 4-5/8" | 9-1/2" | MIT359.5-2 | 2,550 |
|  | 11-7/8" | ITS2.37/11.88 | 1,550 |  | $117 / 8$ " | MIT3511.88-2 | 2,550 |
|  | 14" | ITS2.37/14 | 1,550 |  | 14 " | MIT3514-2 | 2,550 |
|  | 16 " | ITS2.37/16 | 1,550 |  | 16 " | MIT4.75/16 | 2,550 |
| 2-1/2" | 9-1/2" | ITS2.56/9.5 | 1,550 | 5" | 9-1/2" | MIT39.5-2 | 2,550 |
|  | 11-7/8" | ITS2.56/11.88 | 1,550 |  | 11-7/8" | MIT311.88-2 | 2,550 |
|  | 14" | ITS2.56/14 | 1,550 |  | 14 " | MIT314-2 | 2,550 |
|  | 16 " | ITS2.56/16 | 1,550 |  | 16 " | MIT5.12/16 | 2,550 |
| 3-1/2" | 9-1/2" | ITS3.56/9.5 | 1,550 | 7" | 9-1/2" | HB7.12/9.5 | 5,815 |
|  | 11-7/8" | ITS3.56/11.88 | 1,550 |  | 11-7/8" | HB7.12/11.88 | 5,815 |
|  | 14" | ITS3.56/14 | 1,550 |  | 14" | HB7.12/14 | 5,815 |
|  | 16 " | ITS3.56/16 | 1,550 |  | 16 " | HB7.12/16 | 5,815 |



## ADJUSTABLE HEIGHT HANGERS

| Single I-Joist |  |  | Double I-Joist |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Width | Depth | Hanger | Down |  |  |
| Load |  |  |  |  |  | Width | Depth |
| :---: |
| Hanger | | Down |
| :---: |
| Load |



## TENSION BRIDGING FOR I-JOIST

| Joist Height | $\mathbf{1 2}$ | $\mathbf{1 6}$ | $\mathbf{1 9 . 2}$ | $\mathbf{2 4}$ | $\mathbf{3 0}$ | $\mathbf{3 2}$ | $\mathbf{3 6}$ | $\mathbf{4 2}$ | $\mathbf{4 8}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | TB20 | TB27 | TB27 | TB30 | TB36 | TB36 | TB42 | TB48 | TB54 |
| 11-7/8" | TB20 | TB27 | TB27 | TB30 | TB36 | TB36 | TB42 | TB48 | TB54 |
| $14 "$ | TB27 | TB27 | TB27 | TB36 | TB36 | TB42 | TB42 | TB48 | TB54 |
| 16" | TB27 | TB27 | TB30 | TB36 | TB42 | TB42 | TB42 | TB48 | TB54 |



## SKEWED $45^{\circ}$ HANGERS

|  | Single I-Joist |  |  |  | Double I-Joist |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |

HU4-X are special order. Specify angle and direction.

## FIELD SLOPE AND SKEW

| Single I-Joist |  |  |  | Double I-Joist |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Width | Depth | Hanger | Down Load | Width | Depth | Hanger | Down Load |
| 1-3/4" | 9-1/2"-14" | LSSR1.81Z | 1,205 | 3-1/2" | 9-1/2"-14" | LSSR410Z | 1,810 |
| 2-1/16" | 9-1/2"-14" | LSSR2.1Z | 1,205 | 4-1/8" | 9-1/2"-14" | LSU4.28 | 2,300 |
| 2-5/16" | 9-1/2"-14" | LSSR2.37Z | 1,205 | 4-5/8" | 9-1/2"-14" | LSU3510-2 | 2,300 |
| 2-1/2" | 9-1/2"-14" | LSSR2.56Z | 1,205 | 5" | 9-1/2"-14" | LSU5.12 | 1,790 |
| 3-1/2" | 9-1/2"-14" | LSSR410Z | 1,810 | 7" | - | - | - |

Orange highlighted hangers require web stiffeners at l-joist ends.

# LVL Framing Connectors 

| FACEMOUJNTMVLHANGERS |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Single Ply-1-3/4" wide |  |  | Double Ply-3-1/2" wide |  |  | Triple Ply-5-1/4" wide |  |  | Quadruple-Ply 7" wide |  |  |
| Depth | Hanger | Down Load | Depth | Hanger | Down Load | Depth | Hanger | Down Load | Depth | Hanger | Down Load |
| 9-1/4" | $\begin{gathered} \text { HU9 } \\ \text { HUS1.81/10 } \end{gathered}$ | $\begin{aligned} & 3,570 \\ & 5,510 \end{aligned}$ | 9-1/4" | $\begin{aligned} & \text { HHUS410 } \\ & \text { HGUS410 } \end{aligned}$ | $\begin{aligned} & 5,635 \\ & 9,100 \end{aligned}$ | 9-1/4" | HHUS5.50/10 HGUS5.50/10 | $\begin{aligned} & 5,635 \\ & 9,100 \end{aligned}$ | 9-1/4" | HHUS7.25/10 HGUS7.25/10 | $\begin{aligned} & 5,635 \\ & 9,100 \end{aligned}$ |
| 9-1/2" | $\begin{gathered} \text { HU9 } \\ \text { HUS1.81/10 } \end{gathered}$ | $\begin{aligned} & 3,570 \\ & 5,510 \end{aligned}$ | 9-1/2" | HHUS410 <br> HGUS410 | $\begin{aligned} & 5,635 \\ & 9,100 \end{aligned}$ | 9-1/2" | HHUS5.50/10 HGUS5.50/10 | $\begin{aligned} & 5,635 \\ & 9,100 \end{aligned}$ | 9-1/2" | HHUS7.25/10 HGUS7.25/10 | $\begin{aligned} & 5,635 \\ & 9,100 \end{aligned}$ |
| 11-1/4" | $\begin{gathered} \text { HU11 } \\ \text { HUS1.81/10 } \end{gathered}$ | $\begin{aligned} & 4,465 \\ & 5,510 \end{aligned}$ | 11-1/4" | HHUS410 <br> HGUS412 | $\begin{aligned} & 5,635 \\ & 9,100 \end{aligned}$ | 11-1/4" | HHUS5.50/10 HGUS5.50/12 | $\begin{gathered} 5,635 \\ 11,915 \end{gathered}$ | 11-1/4" | HHUS7.25/10 HGUS7.25/12 | $\begin{gathered} 5,635 \\ 11,915 \end{gathered}$ |
| 11-7/8" | $\begin{gathered} \text { HU11 } \\ \text { HUS1.81/10 } \end{gathered}$ | $\begin{aligned} & 4,465 \\ & 5,510 \end{aligned}$ | 11-7/8" | HHUS410 <br> HGUS412 | $\begin{gathered} 5,635 \\ 11,915 \end{gathered}$ | 11-7/8" | HHUS5.50/10 HGUS5.50/12 | $\begin{gathered} 5,635 \\ 11,915 \end{gathered}$ | 11-7/8" | HHUS7.25/10 HGUS7.25/12 | $\begin{gathered} 5,635 \\ 11,915 \end{gathered}$ |
| 14" | $\begin{gathered} \text { HU14 } \\ \text { HUS1.81/10 } \end{gathered}$ | $\begin{aligned} & 5,055 \\ & 5,510 \end{aligned}$ | 14" | HHUS410 <br> HGUS414 | $\begin{gathered} 5,635 \\ 13,860 \end{gathered}$ | 14" | HHUS5.50/10 HGUS5.50/14 | $\begin{gathered} 5,635 \\ 13,860 \end{gathered}$ | 14" | HGUS7.25/14 HGU7.25-SDS | $\begin{aligned} & 13,860 \\ & 13,160 \end{aligned}$ |
| 16" | $\begin{gathered} \text { HU14 } \\ \text { HUS1.81/10 } \end{gathered}$ | $\begin{aligned} & 5,055 \\ & 5,510 \end{aligned}$ | 16" | HHUS410 <br> HGUS414 | $\begin{gathered} 5,635 \\ 13,860 \end{gathered}$ | 16" | HGUS5.50/14 HGU5.50-SDS | $\begin{aligned} & 13,860 \\ & 13,160 \end{aligned}$ | 16" | $\begin{gathered} \text { HGUS7.25/14 } \\ \text { HHGU7.25-SDS } \end{gathered}$ | $\begin{aligned} & 13,860 \\ & 17,345 \end{aligned}$ |
| 18" |  |  | 18" | HHUS410 <br> HGUS414 | $\begin{gathered} 5,635 \\ 13,860 \end{gathered}$ | 18" | HGUS5.50/14 HGU5.50-SDS | $\begin{aligned} & 13,860 \\ & 13,160 \end{aligned}$ | 18" | HGUS7.25/14 <br> HHGU7.25-SDS | $\begin{aligned} & 13,860 \\ & 17,345 \end{aligned}$ |

HGU AND HHGU Hangers specify height


## TOP FLANGE LVL HANGERS

| Single Ply-1-3/4" wide |  |  | Double Ply-3-1/2" wide |  |  | Triple Ply-5-1/4" wide |  |  | Quadruple Ply-7" wide |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Depth | Hanger | Down Load | Depth | Hanger | Down Load | Depth | Hanger | Down Load | Depth | Hanger | Down Load |
| 9-1/4" | $\begin{gathered} \text { WP1.81X(H=9.25) } \\ \text { BA1.81/9.25 } \end{gathered}$ | $\begin{aligned} & 3,095 \\ & 4,715 \end{aligned}$ | 9-1/4" | BA3.56/9.25 <br> HB3.56/9.25 | $\begin{aligned} & 4,715 \\ & 5,815 \end{aligned}$ | 9-1/4" | HGLTV5.37 HB5.50/9.25 | $\begin{gathered} 10,585 \\ 5,815 \end{gathered}$ | 9-1/4" | HGLTV7. 12 <br> HB7.12/9.25 | $\begin{gathered} 10,585 \\ 5,815 \end{gathered}$ |
| 9-1/2" | $\begin{gathered} \text { MIT9.5 } \\ \text { BA1.81/9.5 } \end{gathered}$ | $\begin{aligned} & 2,550 \\ & 4,715 \end{aligned}$ | 9-1/2" | $\begin{aligned} & \text { BA3.56/9.5 } \\ & \text { HB3.56/9.5 } \end{aligned}$ | $\begin{aligned} & 4,715 \\ & 5,815 \end{aligned}$ | 9-1/2" | HGLTV5.37 HB5.50/9.5 | $\begin{gathered} 10,585 \\ 5,815 \end{gathered}$ | 9-1/2" | $\begin{aligned} & \text { HGLTV7.12 } \\ & \text { HB7.12/9.5 } \end{aligned}$ | $\begin{gathered} 10,585 \\ 5,815 \end{gathered}$ |
| 11-1/4" | $\begin{gathered} \text { WP1.81X(H=11.25) } \\ \text { BA1.81/11.25 } \end{gathered}$ | $\begin{aligned} & 3,095 \\ & 4,715 \end{aligned}$ | 11-1/4" | $\begin{aligned} & \text { BA3.56/11.25 } \\ & \text { HB3.56/11.25 } \end{aligned}$ | $\begin{aligned} & 4,715 \\ & 5,815 \end{aligned}$ | 11-1/4" | $\begin{gathered} \text { HGLTV5.37 } \\ \text { HB5.50/11.25 } \end{gathered}$ | $\begin{gathered} 10,585 \\ 5,815 \end{gathered}$ | 11-1/4" | $\begin{gathered} \text { HGLTV7.12 } \\ \text { HB7.12/11.25 } \end{gathered}$ | $\begin{gathered} 10,585 \\ 5,815 \end{gathered}$ |
| 11-7/8" | $\begin{gathered} \text { MIT11.88 } \\ \text { BA1.81/11.88 } \end{gathered}$ | $\begin{aligned} & 2,550 \\ & 4,715 \end{aligned}$ | 11-7/8" | $\begin{aligned} & \text { BA3.56/11.88 } \\ & \text { HB3.56/11.88 } \end{aligned}$ | $\begin{aligned} & 4,715 \\ & 5,815 \end{aligned}$ | 11-7/8" | $\begin{aligned} & \text { HGLTV5.37 } \\ & \text { HB5.50/11.88 } \end{aligned}$ | $\begin{gathered} 10,585 \\ 5,815 \end{gathered}$ | 11-7/8" | $\begin{aligned} & \text { HB7.12/11.88 } \\ & \text { EGQ7.25-SDS } \end{aligned}$ | $\begin{gathered} 5,815 \\ 19,800 \end{gathered}$ |
| 14" | MIT1.81/14 <br> BA1.81/14 | $\begin{aligned} & 2,550 \\ & 4,715 \end{aligned}$ | 14" | HGLTV3.514 <br> BA3.56/14 | $\begin{gathered} 10,585 \\ 4,715 \end{gathered}$ | 14" | $\begin{gathered} \text { HB5.50/14 } \\ \text { EGQ5.37-SDS } \end{gathered}$ | $\begin{gathered} 5,815 \\ 19,800 \end{gathered}$ | 14" | $\begin{gathered} \text { HGLTV7.12 } \\ \text { EGQ7.25-SDS } \end{gathered}$ | $\begin{aligned} & 10,585 \\ & 19,800 \end{aligned}$ |
| 16 " | $\begin{gathered} \text { MIT1.81/16 } \\ \text { BA1.81X(H=16) } \end{gathered}$ | $\begin{aligned} & 2,550 \\ & 4,715 \end{aligned}$ | 16 " | HGLTV3.516 <br> BA3.56/16 | $\begin{gathered} 10,585 \\ 4,715 \end{gathered}$ | 16 " | $\begin{gathered} \text { HB5.50/16 } \\ \text { EGQ5.37-SDS } \end{gathered}$ | $\begin{gathered} 5,815 \\ 19,800 \end{gathered}$ | 16 " | $\begin{gathered} \text { HGLTV7.12 } \\ \text { EGQ7.25-SDS } \end{gathered}$ | $\begin{aligned} & 10,585 \\ & 19,800 \end{aligned}$ |
| 18" | $\begin{aligned} & \text { BA1.81X(H=18) } \\ & \text { HB1.81X(H=18) } \end{aligned}$ | $\begin{aligned} & 4,715 \\ & 5,815 \end{aligned}$ | 18" | $\begin{gathered} \text { HGLTV3.518 } \\ \text { HB3.56/18 } \end{gathered}$ | $\begin{gathered} 10,585 \\ 5,815 \end{gathered}$ | 18 " | $\begin{aligned} & \text { HGLTV5.37 } \\ & \text { EGQ5.37-SDS } \end{aligned}$ | $\begin{aligned} & 10,585 \\ & 19,800 \end{aligned}$ | 18" | $\begin{gathered} \text { HGLTV7.12 } \\ \text { EGQ7.25-SDS } \end{gathered}$ | $\begin{aligned} & 10,585 \\ & 19,800 \end{aligned}$ |

EGQ Hanger specify height

## General Notes

1. Loads listed are the lowest hanger/header limitations assuming header material is Douglas-fir-Larch, Southern Pine, or LVL manufactured in the United States. Top Flange LVL Hanger loads assume header material is LVL. Joist reaction should be checked by a qualified designer to ensure proper hanger selection.
2. Refer to current Simpson Strong-Tie Wood Construction Connectors catalog to verify allowable loads and fastener size and quantity.
3. Loads shown are gravity (floor) loads (100\% D.O.L.). Other load durations may apply. Refer to the current version of Wood Construction Connectors for allowable increases.
4. Top Flange Hanger configurations and thickness of top flange needs to be considered for flush frame conditions.
5. All loads shown are based on 16 d common nails into the header and all nail holes filled (Exceptions: IUS and ITS use 10d common nails and some hangers use SDS screws which are supplied with the hanger).

All hangers listed are manufactured by Simpson Strong-Tie ${ }^{\oplus}$ Co., Inc. For additional information, refer to the current Simpson Strong-Tie literature, www.strongtie.com or contact Simpson Strong-Tie at 800-999-5099.

| Single I-Joists |  |  |  | Double I-Joists |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Width | Depth | Hanger | Down Load | Width | Depth | Hanger | Down Load |
| 1-3/4" | 9-1/2" | IHFL17925 | 960 |  | 9-1/2" | IHF35925 | 3,530 |
|  | 11-7/8" | IHFL17112 | 1,200 | 3-1/2" | 11-7/8" | IHF35112 | 3,530 |
|  | 14" | IHFL1714 | 1,680 |  | 14" | IHF3514 | 4,115 |
| 2-1/16" | 9-1/2" | IHFL20925 | 960 |  | 9-1/2" | IHF20925-2 | 3,530 |
|  | 11-7/8" | IHFL20112 | 1,200 | 4-1/8" | 11-7/8" | IHF20112-2 | 3,530 |
|  | 14" | IHFL2014 | 1,680 | 4-1/8 | 14" | IHF2014-2 | 3,960 |
|  | 16" | IHFL2016 | 1,920 |  | 16" | IHF2014-2 | 3,960 |
| 2-5/16" | 9-1/2" | IHFL23925 | 960 |  | 9-1/2" | IHF23925-2 | 3,530 |
|  | 11-7/8" | IHFL23112 | 1,200 | 4-5/8" | 11-7/8" | THF23118-2 | 1,890 |
|  | 14" | IHFL2314 | 1,680 | 4-5/8" | 14" | THF23140-2 | 2,660 |
|  | 16" | IHFL2316 | 1,920 |  | 16" | THF23160-2 | 3,190 |
| 2-1/2" | 9-1/2" | THFI2595 | 960 |  | 9-1/2" | IHF25925-2 | 3,530 |
|  | 11-7/8" | THFI25118 | 1,200 | 5" | 11-7/8" | IHF25112-2 | 3,530 |
|  | 14" | THFI2514 | 1,680 | 5 | 14" | THF25140-2 | 2,660 |
|  | 16" | IHFL2516 | 1,920 |  | 16" | THF25160-2 | 3,190 |
| 3-1/2" | 9-1/2" | IHFL35925 | 1,200 |  | 9-1/2" | HD7100 | 2,770 |
|  | 11-7/8" | IHFL35112 | 1,440 | 7" | 11-7/8" | HD7120 | 3,390 |
|  | 14" | IHFL3514 | 1,680 | 7 | 14" | HD7140 | 4,005 |
|  | 16" | IHFL3516 | 1,920 |  | 16" | HD7160 | 3,695 |

MiTek Notes: (1) Loads assume maximum nailing schedule for single I-Joists.

## TOP FLANGE HANGERS

| Single I-Joists |  |  |  | Double I-Joists |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Width | Depth | Hanger | Down Load | Width | Depth | Hanger | Down Load |
| 1-3/4" | 9-1/2" | THO17950 | 1,235 | 3-1/2" | 9-1/2" | THO35950 | 2,370 |
|  | 11-7/8" | THO17118 | 1,235 |  | 11-7/8" | THO35118 | 2,525 |
|  | 14" | TFL1714 | 1,585 |  | 14" | THO35140 | 2,400 |
| 2-1/16" | 9-1/2" | TFL2095 | 1,585 | 4-1/8" | 9-1/2" | THO20950-2 | 2,920 |
|  | 11-7/8" | TFL20118 | 1,585 |  | 11-7/8" | THO20118-2 | 2,920 |
|  | 14" | TFL2014 | 1,585 |  | 14" | THO20140-2 | 3,640 |
|  | 16" | TFL2016 | 1,585 |  | 16" | THO20160-2 | 3,640 |
| 2-5/16" | 9-1/2" | TFL2395 | 1,585 | 4-5/8" | 9-1/2" | THO23950-2 | 3,640 |
|  | 11-7/8" | TFL23118 | 1,585 |  | 11-7/8" | THO23118-2 | 3,640 |
|  | 14" | TFL2314 | 1,585 |  | 14 " | THO23140-2 | 4,420 |
|  | 16" | TFL2316 | 1,585 |  | 16" | THO23160-2 | 4,420 |
| 2-1/2" | 9-1/2" | TFL2595 | 1,585 | 5" | 9-1/2" | THO25950-2 | 3,640 |
|  | 11-7/8" | TFL25118 | 1,585 |  | 11-7/8" | THO25118-2 | 3,640 |
|  | 14" | TFL2514 | 1,585 |  | 14" | THO25140-2 | 4,420 |
|  | 16" | TFL2516 | 1,585 |  | 16" | THO25160-2 | 4,420 |
| 3-1/2" | 9-1/2" | THO35950 | 2,370 | 7" | 9-1/2" | BPH7195 | 3,100 |
|  | 11-7/8" | THO35118 | 2,525 |  | 11-7/8" | BPH71118 | 3,075 |
|  | 14" | THO35140 | 2,400 |  | 14" | BPH7114 | 3,075 |
|  | 16" | THO35160 | 2,400 |  | 16" | BPH7116 | 3,075 |

MiTek Notes: For I-Joists, consult MiTek for joist limitations.

## ADJUSTABLE HEIGHT HANGERS

| Single I-Joists |  |  |  | Double l-Joists |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Width | Depth | Hanger | Down Load | Width | Depth | Hanger | Down |
| 1-3/4" | 9-1/2" | MSH1722 | 2,390 | 3-1/2" | 9-1/2" | MSH422 | 2,530 |
|  | 11-7/8" | MSH1722 | 2,390 |  | 11-7/8" | MSH422 | 2,530 |
|  | 14 " | MSH1722 | 2,390 |  | $14^{\prime \prime}$ | MSH422 | 2,530 |
| 2-1/16" | 9-1/2" | MSH2022 | 2,390 | 4-1/8" | 9-1/2" |  |  |
|  | 11-7/8" | MSH2022 | 2,390 |  | 11-7/8" |  |  |
|  | $14^{\prime \prime}$ | MSH2022 | 2,390 |  | $14{ }^{\prime \prime}$ |  |  |
|  | $16^{\prime \prime}$ | MSH2022 | 2,390 |  | $16^{\prime \prime}$ | ---- |  |
| 2-5/16" | 9-1/2" | MSH2322 | 2,395 | 4-5/8" | 9-1/2" | MSH2322-2 | 2,530 |
|  | 11-7/8" | MSH2322 | 2,395 |  | 11-7/8" | MSH2322-2 | 2,530 |
|  | 14 " | MSH2322 | 2,395 |  | 14 " | MSH2322-2 | 2,530 |
|  | 16 " | MSH2322 | 2,395 |  | 16 " | MSH2322-2 | 2,530 |
| 2-1/2" | 9-1/2" | MSH322 | 2,395 | 5" | 9-1/2" | MSH2622-2 | 2,530 |
|  | 11-7/8" | MSH322 | 2,395 |  | 11-7/8" | MSH2622-2 | 2,530 |
|  | $14^{\prime \prime}$ | MSH322 | 2,395 |  | $14^{\prime \prime}$ | MSH2622-2 | 2,530 |
|  | $16^{\prime \prime}$ | MSH322 | 2,395 |  | 16 " | MSH2622-2 | 2,530 |
| 3-1/2" | 9-1/2" | MSH422 | 2,530 | 7" | 9-1/2" | MSH422-2 | 3,740 |
|  | 11-7/8" | MSH422 | 2,530 |  | 11-7/8" | MSH422-2 | 3,740 |
|  | 14 " | MSH422 | 2,530 |  | 14 " | MSH422-2 | 3,740 |
|  | $16^{\prime \prime}$ | MSH422 | 2,530 |  | 16 " | MSH422-2 | 3,740 |
| Blue highlighted areas require web stiffeners at joist end |  |  |  |  |  |  |  |

## LVL Framing Connectors

## FACE MOUNT HANGERS

| Single Ply -1-3/4" wide |  |  | Double Ply - 3-1/2" wide |  |  | Triple Ply -5-1/4" wide |  |  | Quadruple Ply - 7" wide |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Depth | Hanger | Down Load | Depth | Hanger | Down Load | Depth | Hanger | Down Load | Depth | Hanger | Down Load |
| 9-1/4", 9-1/2" | HD17925² | 3,695 | 9-1/4", 9-1/2" |  |  | 9-1/4", 9-1/2" | THD610 |  | 9-1/4", 9-1/2" | THD7210 THDH7210 | 6,535 |
| 11-1/4", | HD17112 ${ }^{2}$ | 4,320 | 11-1/4", | THD410 | 5,850 | 11-1/4", | THD610 | 6,535 | 11-1/4", | THD7210 | 6,535 |
| 11-7/8" | HUS179 ${ }^{1}$ | 5,580 | 11-7/8" | THDH412 ${ }^{1}$ | 9,710 | 11-7/8" | THDH612 ${ }^{1}$ | 9,530 | 11-7/8" | THDH7212 ${ }^{1}$ | 9,020 |
| 14" | HD1714 ${ }^{2}$ <br> HUS179 ${ }^{1}$ | $\begin{aligned} & 4,580 \\ & 5,580 \end{aligned}$ | 14" | $\begin{aligned} & \text { THD410 } \\ & \text { THDH414 } \end{aligned}$ | $\begin{gathered} 5,850 \\ 11,325 \end{gathered}$ | 14" | $\begin{aligned} & \text { THD610 } \\ & \text { THDH614 }{ }^{1} \end{aligned}$ | $\begin{gathered} 6,535 \\ 11,325 \end{gathered}$ | 14 " | $\begin{aligned} & \text { THD7210 } \\ & \text { THDH7214 } \end{aligned}$ | $\begin{gathered} 6,535 \\ 11,325 \end{gathered}$ |
| 16" | $\text { HD1714 }{ }^{2}$ | $4,580$ | $16 "$ | $\begin{aligned} & \text { THD412 } \\ & \text { THDH414 }{ }^{1} \end{aligned}$ | $\begin{gathered} 7,045 \\ 11325 \end{gathered}$ | $16 "$ | $\begin{gathered} \text { THD612 } \\ \text { THDH614 } \end{gathered}$ | $\begin{gathered} 8,255 \\ 11,325 \end{gathered}$ | $16 "$ | $\begin{gathered} \mathrm{HD}^{2} 120^{2} \\ \text { THDH7214 } \end{gathered}$ | $\begin{gathered} 3,390 \\ 11,325 \end{gathered}$ |
| 18" | HD1714 ${ }^{2}$ | 4,580 | 18" | $\begin{gathered} \text { THD412 } \\ \text { THDH414 } \end{gathered}$ | $\begin{gathered} 7,045 \\ 11 \end{gathered}$ | 18" | $\begin{gathered} \text { THD612 } \\ \text { THDH614 } \end{gathered}$ | $\begin{gathered} 8,255 \\ 11,325 \end{gathered}$ | 18" | $\begin{gathered} \text { HD7140 } \\ \text { THDH7214 } \end{gathered}$ | $\begin{gathered} 4,005 \\ 11,325 \end{gathered}$ |

MiTek Notes: (1) Joist nails need to be toe nailed at a $30^{\circ}$ to $45^{\circ}$ angle to achieve listed loads. (2) Loads assume maximum nailing schedule.


## TOP FLANGE HANGERS

| Single Ply - 1-3/4" wide |  |  | Double Ply - 3-1/2" wide |  |  | Triple Ply -5-1/4" wide |  |  | Quadruple Ply - 7" wide |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Depth | Hanger | Down Load | Depth | Hanger | Down Load | Depth | Hanger | Down Load | Depth | Hanger | Down Load |
| 9-1/4" | $\begin{gathered} \text { BPH17925 } \\ \text { PHXU17925 } \end{gathered}$ | $\begin{aligned} & 2,970 \\ & 4,350 \end{aligned}$ | 9-1/4" | $\begin{aligned} & \text { HBPH35925 } \\ & \text { HLBH35925 } \end{aligned}$ | $\begin{gathered} 6,310 \\ 10,045 \end{gathered}$ | 9-1/4" | $\begin{aligned} & \text { HBPH55925 } \\ & \text { HLBH55925 } \end{aligned}$ | $\begin{gathered} 6,185 \\ 10,045 \end{gathered}$ | 9-1/4" | $\begin{aligned} & \text { HBPH71925 } \\ & \text { HLBH71925 } \end{aligned}$ | $\begin{gathered} 6,185 \\ 10,045 \end{gathered}$ |
| 9-1/2" | $\begin{gathered} \text { BPH1795 } \\ \text { PHXU1795 } \end{gathered}$ | $\begin{aligned} & 2,970 \\ & 4,350 \end{aligned}$ | 9-1/2" | $\begin{aligned} & \text { HBPH3595 } \\ & \text { HLBH3595 } \end{aligned}$ | $\begin{gathered} 6,310 \\ 10,045 \end{gathered}$ | 9-1/2" | $\begin{aligned} & \text { HBPH5595 } \\ & \text { HLBH5595 } \end{aligned}$ | $\begin{gathered} 6,185 \\ 10,045 \end{gathered}$ | 9-1/2" | $\begin{aligned} & \text { HBPH7195 } \\ & \text { HLBH7195 } \end{aligned}$ | $\begin{gathered} 6,185 \\ 10,045 \end{gathered}$ |
| 11-1/4" | $\begin{gathered} \text { BPH17112 } \\ \text { PHXU17112 } \end{gathered}$ | $\begin{aligned} & 2,970 \\ & 4,350 \end{aligned}$ | 11-1/4" | $\begin{aligned} & \text { HBPH35112 } \\ & \text { HLBH35112 } \end{aligned}$ | $\begin{gathered} 6,310 \\ 10,045 \end{gathered}$ | 11-1/4" | $\begin{aligned} & \text { HBPH55112 } \\ & \text { HLBH55112 } \end{aligned}$ | $\begin{gathered} 6,185 \\ 10,045 \end{gathered}$ | 11-1/4" | $\begin{aligned} & \text { HBPH71112 } \\ & \text { HLBH71112 } \end{aligned}$ | $\begin{gathered} 6,185 \\ 10,045 \end{gathered}$ |
| 11-7/8" | $\begin{gathered} \mathrm{BPH} 17118 \\ \text { PHXU17118 } \end{gathered}$ | $\begin{aligned} & 2,970 \\ & 4,350 \end{aligned}$ | 11-7/8" | $\begin{aligned} & \text { HBPH35118 } \\ & \text { HLBH35118 } \end{aligned}$ | $\begin{gathered} 6,310 \\ 10,045 \end{gathered}$ | 11-7/8" | $\begin{aligned} & \text { HBPH55118 } \\ & \text { HLBH55118 } \end{aligned}$ | $\begin{gathered} 6,185 \\ 10,045 \end{gathered}$ | 11-7/8" | $\begin{aligned} & \text { HBPH71118 } \\ & \text { HLBH71118 } \end{aligned}$ | $\begin{gathered} 6,185 \\ 10,045 \end{gathered}$ |
| 14" | BPH1714 PHXU1714 | $\begin{aligned} & 2,970 \\ & 4,350 \end{aligned}$ | 14" | $\begin{aligned} & \text { HBPH3514 } \\ & \text { HLBH3514 } \end{aligned}$ | $\begin{gathered} 6,310 \\ 10,045 \end{gathered}$ | 14" | $\begin{aligned} & \text { HBPH5514 } \\ & \text { HLBH5514 } \end{aligned}$ | $\begin{gathered} 6,185 \\ 10,045 \end{gathered}$ | 14" | $\begin{aligned} & \text { HBPH7114 } \\ & \text { HLBH7114 } \end{aligned}$ | $\begin{gathered} 6,185 \\ 10,045 \end{gathered}$ |
| $16 "$ | BPH1716 | $2,970$ | 16" | HBPH3516 <br> HLBH3516 | $\begin{gathered} 6,310 \\ 10,045 \end{gathered}$ | $16 "$ | HBPH5516 <br> HLBH5516 | $\begin{gathered} 6,185 \\ 10,045 \end{gathered}$ | 16" | HBPH7116 <br> HLBH7116 | $\begin{gathered} 6,185 \\ 10,045 \end{gathered}$ |
| 18" | -- | -- | 18" | HBPH3518 <br> HLBH3518 | $\begin{gathered} 6,310 \\ 10,045 \end{gathered}$ | 18" | $\begin{aligned} & \text { HBPH5518 } \\ & \text { HLBH5518 } \end{aligned}$ | $\begin{gathered} 6,185 \\ 10,045 \end{gathered}$ | 18" | HBPH7118 <br> HLBH7118 | $\begin{gathered} 6,185 \\ 10,045 \end{gathered}$ |

## General Notes

1. Loads listed are the lowest hanger/header limitations assuming header material is Douglas-fir-Larch, Southern Pine, or LVL manufactured in the United States. Top Flange LVL Hanger loads assume header material is LVL. Joist reaction should be checked by a qualified designer to ensure proper hanger selection.
2. Refer to current MiTek product catalog to verify allowable loads and fastener size and quantity.
3. Loads shown are gravity (floor) loads (100\% D.O.L.). Other load durations may apply. Refer to the current MiTek product catalog for allowable increases.
4. Top Flange Hanger configurations and thickness of top flange needs to be considered for flush frame conditions.
[^3]| CODE REPORT INDEX |  |
| :--- | :--- |
| Roseburg EWP Code Reports | Product |
| ICC ESR-1251 (with LABC/LARC supplement, CBC/CRC supplement <br> including DSA \& OSHPD, and FBC supplement) | I-JOIST |
| ICC ESR-1210 (with LABC/LARC supplement, CBC/CRC supplement <br> including DSA \& OSHPD, and FBC supplement) | LVL \& LVL Rim |
| APA PR-L259 (U.S.) and APA PR-L259C (Canada) | I-JOIST |
| APA PR-L289 (U.S.) and APA PR-L289C (Canada) | LVL |
| APA PR-L270 | LVL STUDS |
| Florida FL2440 | I-JOIST \& LVL |
| CCMC 13323-R (Canada) | I-JOIST |
| CCMC 13310-R (Canada) | LVL |

The code reports listed above are available at Roseburg.com, in the Engineered Wood Products section under Code Reports.

## PRODUCT \& PERFORMANCE WARRANTY

Roseburg Forest Products warrants that its RFPI ${ }^{\circ}$-Joists, RigidLam ${ }^{\circ}$ laminated veneer lumber (LVL) and RigidRim ${ }^{*}$ Rimboard will be free from manufacturing errors and defects in workmanship and materials in accordance with our specifications.

Furthermore, we warrant that these products, when properly stored, installed and used in dry use service conditions, will meet or exceed our performance specifications for the expected life of the structure.

RFPl", RigidLam", RigidRim" are registered trademarks of Roseburg Forest Products, Roseburg, Oregon.


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An electronic version of this Design Guide can be found at www.Roseburg.com under "Design Guides" in the Engineered Wood section.

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## R Roseburg


[^0]:    (1) Comparison chart based on uniform loads (Live load = 40 psf, Dead load = 10 psf ).
    (2) Spans taken from 2021 International Residential Code.

[^1]:    (1)This table is for preliminary design use only. Final design should include a complete analysis.
    (2)Span = clear span for simply supported member with uniform loads only.
    (3) Joints in rimboard shall not be located within opening.
    (4)Spans shown can conservatively be used for 1-1/4" wide RigidRim Plus and 1.4E RigidRim LVL.

[^2]:    1. Allowable shear and moment values are for $100 \%$ Duration of Load and may be adjusted for other durations of load. El shall not be adjusted for duration of load.
    2. For 2-Ply, 3-Ply and 4-Ply LVL members, the values in the tables may be multiplied by 2,3 and 4 respectively.
    3. For $1-1 / 2^{\prime \prime}$ wide LVL members, allowable design values may be obtained by multiplying the table values by 0.857 .
    4. $1-1 / 2^{\prime \prime}$ wide members 14 " and deeper must be a minimum of two plies unless designed by a design professional for a specific application.
    5. $1-3 / 4$ " wide members 16 " and deeper must be a minimum of two plies unless designed by a design professional for a specific application.
    6. Single ply $1-1 / 2^{\prime \prime}$ wide members are assumed to be laterally braced at $16^{\prime \prime}$ o.c. or less.
    7. Single ply $1-3 / 4$ " wide members are assumed to be laterally braced at $24^{\prime \prime}$ o.c. or less.
    8. Single ply for any depth are allowed for ledgers.
[^3]:    All hangers listed are manufactured by Mitek ${ }^{\circledR}$. For more information refer to the current MiTek literature, www.mitek-us.com or contact MiTek at 800-328-5934.

