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ICC-ES Evaluation Report ESR-2552

DIVISION: 06 00 00—WOOD, PLASTICS AND COMPOSITES

Section: 06 05 23—Wood, Plastic, and Composite Fastenings

REPORT HOLDER:

SIMPSON STRONG-TIE COMPANY INC.

EVALUATION SUBJECT:

SIMPSON STRONG-TIE[®] FACE-MOUNT HANGERS FOR SAWN LUMBER, STRUCTURAL COMPOSITE LUMBER (SCL), PREFABRICATED WOOD I-JOISTS AND GLULAM BEAMS (ENGINEERED WOOD PRODUCTS)

1.0 EVALUATION SCOPE

Compliance with the following codes:

- 2021, 2018, 2015 and 2012 International Building Code[®] (IBC)
- 2021, 2018, 2015 and 2012 International Residential Code[®] (IRC)

For evaluation for compliance with codes adopted by the Los Angeles Department of Building and Safety (LADBS), see <u>ESR-2552 LABC and LARC Supplement</u>.

Property evaluated:

Structural

2.0 USES

The Simpson Strong-Tie[®] face-mount hangers described in this report are used as wood framing connectors in wood construction in accordance with Section <u>2304.10.4</u> of the 2021 IBC, <u>2304.10.3</u> of the 2018 and 2015 IBC and Section <u>2304.9.3</u> of the 2012 IBC. The face-mount hangers may also be used in structures regulated under the IRC when an engineered design is submitted in accordance with Section R301.1.3 of the IRC.

3.0 DESCRIPTION

3.1 General:

With the exception of the CBH, ACBH and HSKP Hangers Series, the Simpson Strong-Tie face-mount hangers described in this report are U-shaped hangers that have



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Reissued March 2023 Revised September 2023 This report is subject to renewal March 2024.

prepunched holes for the installation of nails or Simpson Strong-Drive SDS screws, depending on the hanger model, into the face of the supporting wood header or beam or ledger. The CBH Hanger is used in pairs and is a die-formed steel plate with a welded stud and prepunched holes for the installation of Simpson Strong-Drive SDS screws. The ACBH hanger is used in pairs and is manufactured in one piece from aluminum with predrilled holes for the installation of Simpson Strong-Drive SDCF screws. The HSKP hanger has a horizontal bearing plate to support the joist/beam and includes prepunched holes in its back plate for the installation of Simpson Strong-Drive SDCF screws into the face of the supporting wood Column.

3.1.1 LU Series Hangers: The LU series hangers are formed from No. 20 gage galvanized steel. See <u>Table 1</u> for hanger dimensions, required fasteners, and allowable loads; and <u>Figure 1</u> for a drawing of a typical LU series hanger.

3.1.2 LUCZ Series Hangers: The LUCZ hangers have concealed flanges to allow for installation near the end of a supporting member such as a ledger or header. The hangers are formed from No. 18 gage galvanized steel coated with a G185 zinc coating. See <u>Table 2</u> and <u>Figure 2</u> for hanger dimensions, required fastener schedule, allowable loads and a typical installation detail.

3.1.3 U Hangers: The U series hangers are formed from No. 16 gage galvanized steel. The hangers are face-nailed to the supporting wood header, and nailed to the supported solid sawn lumber, structural composite lumber or prefabricated wood I-joists with web stiffeners to accept the required size and number of joist nails shown in <u>Table 3</u>. See <u>Table 3</u> for the U series hanger model numbers, hanger dimensions, required fasteners, and allowable loads. See <u>Figure 3</u> for a drawing of a typical U series joist hanger.

3.1.4 HU and HUC Series Hangers: The HU and HUC series hangers are formed from No. 14 gage galvanized steel, and are face-nailed to the supporting wood header/beam and nailed to the solid sawn lumber, supported structural composite lumber or prefabricated wood I-joists with web stiffeners. HU hangers having a seat width (W) equal to or greater than $2^{9}/_{16}$ inches (65 mm) are available with concealed flanges and are specified with the model designation HUC. See <u>Table 4</u> for hanger model numbers, hanger dimensions, required fasteners and allowable loads based on the nailing. The HU and HUC hangers have triangular and round holes in both the

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U-shaped portion supporting the wood joist and in the flanges attached to the supporting wood header/beam. The allowable loads are achieved by filling all holes (both the round and triangular) with size of nails shown in <u>Table 4</u>. See Figures <u>4A</u> and <u>4B</u> for drawings of typical HU and HUC hangers, and <u>Figure 4C</u> shows a typical installation of an HU hanger supporting a prefabricated wood I-joist with approved web stiffeners.

3.1.5 HUCQ Series Hangers: The HUCQ series hangers are formed from No. 14 gage galvanized steel and are used to support glulam beams or other approved wood members. Other approved wood members must be laterally supported at the ends in accordance with Section 2308.4.6 of the IBC and Section <u>R502.7</u> of the IRC. The hangers have prepunched holes for the installation of Simpson Strong-Drive SDS series wood screws (SDS). See <u>Table 5</u> for the HUCQ series model numbers, hanger dimensions, required fasteners and allowable loads. See <u>Figure 5</u> for a drawing of the HUCQ series hanger.

3.1.6 LUS Series Hangers: The LUS series hangers are formed from No. 18 gage galvanized steel. The hangers have prepunched holes for the installation of nails that are driven at a 45-degree angle through the joist and into the header, which is described as double shear nailing in the installation instructions. See <u>Table 6</u> for the hanger dimensions, required fasteners, and allowable loads; and Figure 6 for a drawing of a typical LUS series hanger.

3.1.7 MUS Joist Hanger: The MUS series hangers are formed from No. 18 gage galvanized steel. The U-shaped portion of the hangers has prepunched holes for the installation of joist nails that are driven at an angle through the joist and into the header, which is described as double shear nailing in the installation instructions. See <u>Table 7</u> for the hanger dimensions, required fasteners, and allowable loads; Figure 7 for a drawing of a typical MUS series hanger.

3.1.8 HUS Series Hangers: The HUS hangers having a seat width, *W*, of $3^{9}/_{16}$ inches (90 mm) are formed from No. 14 gage galvanized steel. The HUS1.81/10 hanger with a seat width, *W*, of $1^{13}/_{16}$ inches (46 mm) is formed from No. 16 gage galvanized steel. The hangers have prepunched holes for the installation of nails that are driven at a 45-degree angle through the joist and into the header, which is described as double shear nailing in the installation instructions. See <u>Table 8</u> for HUS series hanger models, hanger dimensions, required fasteners, and allowable loads. See <u>Figure 8</u> for a drawing of a typical HUS hanger.

3.1.9 HHUS Series Hangers: The HHUS series hangers are formed from No. 14 gage galvanized steel. The hangers have pre-punched holes for the installation of nails that are driven at a 45 degree angle through the joist and into the header, which is described as double shear nailing in the installation instructions. See <u>Table 9</u> for the hanger model numbers, hanger dimensions, required fasteners, and allowable loads. See <u>Figure 9</u> for a drawing of a typical HHUS hanger.

3.1.10 HGUS Series Hangers: The HGUS series hangers are formed from No. 12 gage galvanized steel and are used to support glulam beams or other approved wood members. Other approved wood members must be laterally supported at the ends in accordance with Section 2308.4.6 of the IBC and Section R502.7 of the IRC. The hangers have prepunched holes for the installation of nails that are driven at a 45-degree angle through the joist and into the header, which is described as double shear nailing in the installation instructions. See <u>Table 10</u> for the HGUS series hanger model numbers, hanger dimensions, required fasteners and

allowable loads. See <u>Figure 10</u> for a drawing of a typical HGUS hanger.

3.1.11 HTU Series Hangers: The HTU hangers are designed to support trusses installed with full or partial heel heights and gaps between the truss and the supporting girders of up to, but not exceeding, 1/2 inch (12.7 mm), as shown in Tables 11A and 1/C, and 1/8 inch (3.2 mm) as shown in Table 11B. Minimum and maximum nailing options are given in Tables 11A, 11B, and 11C to address varying heel heights and support conditions. The HTU hangers are formed from No. 16 gage galvanized steel. See Table 11A and Figures 11A and 11B for hanger dimensions, required fastener schedule, allowable loads and an installation detail for installations in which the gap between the truss and the supporting girders is less than or equal to 1/2 inch (12.7 mm). See Table 11B and Figures 11A and 11B for hanger dimensions, required fastener schedule, allowable loads and an installation detail for installations in which the gap between the truss and the supporting girders is less than or equal to 1/8 inch (3.2 mm). See Table 11C and Figures 11A and 11C for hanger dimensions, required fastener schedule, allowable loads and an installation detail for installations in which the minimum allowable number of nails is driven into the supporting girder, and the gap between the truss and supporting girder is less than or equal to 1/2 inch (12.7 mm).

3.1.12 LGU, MGU, HGU and HHGU Series Hangers: The LGU and MGU series hangers are formed from No. 10 gage galvanized steel; HGU series hangers are formed from No. 7 gage galvanized steel; and HHGU series hangers are formed from No. 3 gage steel. The LGU, MGU, HGU and HHGU series hangers are used to support glulam beams or other approved wood members. Other approved wood members must be laterally supported at the ends in accordance with Section 2308.4.6 of the IBC and Section R502.7 of the IRC. The hangers have pre-punched holes for the installation of Simpson Strong-Drive SDS series wood screws (SDS). See <u>Table 12</u> for the LGU, MGU, HGU and HHGU series model numbers, hanger dimensions, required fasteners and allowable loads. See <u>Figure 12</u> for a drawing of the HHGU series hanger and a typical installation detail.

3.1.13 SUR/L Series Hangers: The SUR/L series hangers are formed from No. 16 gage galvanized steel. The SUR and SUL hangers are identical except they are skewed at 45 degrees to the right (SUR) and the left (SUL), respectively. See <u>Table 13</u> for hanger models, hanger dimensions, required fasteners, and allowable loads. See <u>Figure 13A</u> for a drawing of a typical SUL hanger and <u>Figure 13B</u> for a typical SUR hanger installation.

3.1.14 HSUR/L Series Hangers: The HSUR/L series hangers are formed from No. 14 gage galvanized steel. The HSUR and HSUL hangers are identical except they are skewed at 45 degrees to the right (HSUR) and the left (HSUL), respectively, and are designed to support prefabricated wood I-joists having approved web stiffeners, and structural composite lumber (SCL). See <u>Table 14</u> for the hanger model numbers, hanger dimensions, required fasteners, and allowable loads. See <u>Figure 14A</u> for a drawing of a typical HSUR hanger, and Figure 14B for a typical HSUR hanger installation.

3.1.15 IUS Series Hangers: The IUS series hangers are formed from No. 18 gage galvanized steel and are used exclusively to support prefabricated wood I-joists to a supporting wood member. See <u>Table 15A</u> for the IUS series hanger model numbers, hanger dimensions, and required fasteners; and <u>Table 15B</u> for allowable loads. See <u>Figures 15A</u>, <u>15B</u>, and <u>15C</u> for drawings of a typical IUS hanger and a typical IUS hanger installation.

3.1.16 MIU Series Joist Hangers: The MIU series hangers are formed from No. 16 gage galvanized steel and are used to support prefabricated wood I-joists or other approved wood members. Other approved wood members must be laterally supported at the ends in accordance with Section 2308.4.6 of the IBC and Section R502.7 of the IRC. See <u>Table 16</u> for the MIU series hanger model numbers, hanger dimensions, required fasteners, and allowable loads. See <u>Figure 16A</u> for a drawing of a typical MIU hanger and <u>Figure</u> 16B for a drawing of a typical MIU hanger installation.

3.1.17 DU, DHU and DHUTF Series Hangers: The DU, DHU and DHUTF series hangers are face-mount and top-flange hangers that are formed from either No. 14 gage or No. 12 gage galvanized steel. They are designed to transfer joist floor loads to a wood stud wall through two layers of $5/_8$ -inch-thick (15.9 mm) gypsum board complying with <u>ASTM C1396</u>, and are installed using a combination of nails and Simpson Strong-Drive SDS series wood screws (SDS), which are addressed in <u>ESR-2236</u>. These hangers are installed after the two layers of $5/_8$ " Type X gypsum board is installed and fastened using, at a minimum, the fastener schedule in Item 14.1-5 in IBC Table 721.1(2). See <u>Table 17A</u> for hanger dimensions and allowable loads. See <u>Table 17B</u> for fastener schedule. See Figure 17 for a drawing of the DU, DHU and DHUTF hangers and a typical installation.

3.1.18 CBH Series Hangers: The CBH series hangers are face-mounted and installed in inverted pairs. They are formed from No. 3 gage steel. They are used to support glulam beams or other approved wood members and are installed using Simpson Strong-Drive SDS series wood screws (SDS), which are addressed in <u>ESR-2236</u>. See <u>Table 18A & 18B</u> for hanger dimensions, fastener schedules and allowable loads. See <u>Figure 18</u> for a drawing of the CBH hangers and a typical installation.

3.1.19 <u>ACBH Series Hangers:</u> The ACBH series hangers are aluminum and are face-mounted and installed in inverted pairs. They are used to support glulam beams and are installed using Simpson Strong-Drive SDCF series wood screws, which are addressed in <u>ESR-3046</u>. See <u>Table 19</u> for hanger dimensions, fastener schedules and allowable loads. See <u>Figure 19</u> for a drawing of the ACBH hangers and a typical installation.

3.1.20 HSKP Series Hangers: The HSKP series hangers are factory welded with a No. 3 gage steel back plate, $\frac{1}{2}$ " steel bearing plate and a No. 3 gage steel (for HSKP5.75x14.5 and HSKP5.75x17.0) or 3/8" steel (for HSKP5.75x19.5) knife plate. The HSKP is installed using Simpson Strong-Drive SDCF series wood screws, which are addressed in ESR-3046. See Table 20 for model numbers, hanger dimensions, fasteners shedules, allowable loads and installation instructions. See Figure 20 for hanger drawings, typical installation and bearing area requirements for the carried members.

3.2 Materials:

3.2.1 Hanger Material: All hangers described in this report, with the exception of the CBH, ACBH, HSKP, HTU, HGUS and HHGU series hangers, are manufactured from galvanized steel complying with <u>ASTM A653</u>, SS designation, Grade 33, with a minimum yield strength, F_{y} , of 33,000 psi (227 MPa) and a minimum tensile strength, F_{u} , of 45,000 psi (310 MPa).

The HTU and HGUS series hangers are manufactured from galvanized steel complying with ASTM A653, SS designation, Grade 40, with a minimum yield strength, F_{y} , of 40,000 psi (276 MPa) and a minimum tensile strength, F_{u} , of 55,000 psi (379 MPa).

The HHGU and CBH series hangers are manufactured from nongalvanized steel complying with <u>ASTM A1011</u> SS, Grade 33, with a minimum yield strength, F_y , of 33,000 psi (227 MPa) and a minimum tensile strength, F_u , of 52,000 psi (358 MPa).

The ACBH series hangers are manufactured from ASTM B221 6061-T6 aluminum with a minimum yield strength, F_{y} , of 40,000 psi (276 MPa) and a minimum tensile strength, F_{u} , of 45,000 psi (310 MPa).

The HSKP series hanger is manufactured from three components: a back plate, a bearing plate and a knife plate. The HSKP back plate and knife plate are manufactured from nongalvanized steel complying with <u>ASTM A1011</u> SS, Grade 33, with a minimum yield strength, F_{y} , of 33,000 psi (227 MPa) and a minimum tensile strength, F_{u} , of 52,000 psi (358 MPa). The HSKP bearing plate is manufactured from nongalvanized steel complying with ASTM A572, Grade 50, with a minimum yield strength, F_{y} , of 50,000 psi (345 MPa) and a minimum tensile strength, F_{u} , of 65,000 psi (448 MPa).

Exception: The HSKP5.75x19.5 knife plate component is manufactured from nongalvanized steel complying with <u>ASTM A36</u> with a minimum yield strength, F_{y} , of 36,000 psi (248 MPa) and a minimum tensile strength, F_{u} , of 58,000 psi (400 MPa).

Base-metal thicknesses for the hangers in this report are as follows:

NOMINAL THICKNESS (gage No. or inch)	MINIMUM BASE-METAL THICKNESS (inch)
1/2"	0.4900
³ /8"	0.3650
No. 3	0.2285
No. 7	0.1715
No. 10	0.1275
No. 12	0.0975
No. 14	0.0685
No. 16	0.0555
No. 18	0.0444
No. 20	0.0335

For **SI:** 1 inch = 25.4 mm.

The thickness of the installed pairs of CBH and ACBH series hangers are shown in the applicable figures in this report.

The galvanized zinc coating conforms to ASTM A924 and ASTM A653 with a G90 designation. Some models (designated with a model number ending with Z) are available with a G185 zinc coating specification in accordance with ASTM A653. Some models (designated with a model number ending with HDG) are available with a hot-dip galvanization, also known as "batch" galvanization, in accordance with ASTM A123, with a minimum specified coating weight of 2.0 ounces of zinc per square foot of surface area (600 g/m²), total for both sides. With the exception of the LUCZ, model numbers in this report do not include the Z or HDG ending, but the information shown applies. The lumber treater or holder of this report (Simpson Strong-Tie Company) should be contacted for recommendations on minimum corrosion resistance of steel connectors in contact with the specific proprietary preservative treated or fire retardant treated lumber.

3.2.2 Wood: Wood headers/beams and columns to which the connectors are fastened must be either sawn lumber, structural glued-laminated timber, or engineered lumber

having a minimum specific gravity of 0.50 (minimum equivalent specific gravity of 0.50 for engineered lumber), and having a maximum moisture content of 19 percent (16 percent for engineered lumber) except as noted in Section 4.1. The thickness of the supporting wood member (header) must be equal to or greater than the length of the fasteners specified in the tables in this report, or as required by wood member design, whichever is greater.

Supported wood members that are prefabricated wood I-joists or SCL must be addressed in a current evaluation report, which will specify the allowable shear capacity and allowable reactions at supports for the proprietary engineered wood lumber. When required, web stiffeners for prefabricated wood I-joists must comply with specifications noted in the applicable evaluation report for the I-joists. The hangers described in this evaluation report may support I-joists having a current ICC-ES evaluation report. The evaluation report for the prefabricated wood I-joist must specify a minimum bearing length that is equal to or less than the hanger bearing length. When the hangers are used to support prefabricated wood I-joists, joist end reactions must not exceed the allowable joist end reaction per the I-joist manufacturer's code report. For installation with engineered wood members, minimum allowable fastener spacing and end and edge distances, as specified in the applicable evaluation report for the engineered wood product, must be met.

3.2.3 Fasteners: Nails used for hangers described in this report must comply with <u>ASTM F1667</u> and have the following minimum fastener dimensions and bending yield strengths (F_{yb}):

FASTENERS	SHANK DIAMETER (inches)	NAIL LENGTH (inches)	F _{уb} (psi)
10d × 1 ¹ / ₂	0.148	1 ¹ / ₂	90,000
10d	0.148	3	90,000
16d × 2 ¹ / ₂	0.162	2 ¹ / ₂	90,000
16d	0.162	3 ¹ / ₂	90,000

For SI: 1 inch = 25.4 mm, 1 psi = 6.895 kPa.

Unless otherwise indicated, fasteners listed in this report as 10d and 16d are 10d common and 16d common nails, respectively.

Some of the hangers and concelaed beam hangers (CBH) described in this report are attached to the wood members using Simpson Strong-Drive SDS series wood screws addressed in ESR-2236. Other concealed beam hangers (ACBH) and HSKP hangers described in this report are attached to wood members using SDCF Strong-Drive wood screws addressed in <u>ESR-3046</u>.

Fasteners used in contact with preservative treated or fire retardant treated lumber must comply with Section 2304.10.6 of the 2021 IBC, Section 2304.10.5 of the 2018 and 2015 IBC, Section 2304.9.5 of the 2012, and applicable sections of the IRC. The chemical treatment manufacturer or this report holder (Simpson Strong-Tie Company) should be contacted for recommendations on minimum corrosion resistance of fasteners and connection capacities of fasteners used with the specific proprietary preservative treated or fire retardant treated lumber.

4.0 DESIGN AND INSTALLATION

4.1 Design:

The tabulated allowable loads shown in this report are based on allowable stress design (ASD) and include the load duration factor, C_D , corresponding with the applicable loads in accordance with the NDS.

Tabulated allowable loads apply to products connected to wood used under dry conditions and where sustained temperatures are $100^{\circ}F(37.8^{\circ}C)$ or less. When products are installed to wood having a moisture content greater than 19 percent (16 percent for engineered wood products), or where wet service is expected, the allowable loads must be adjusted by the wet service factor, C_M, specified in the NDS. When connectors are installed in wood that will experience sustained exposure to temperatures exceeding $100^{\circ}F(37.8^{\circ}C)$, the allowable loads in this report must be adjusted by the temperature factor, C_t, specified in the NDS.

Connected wood members must be analyzed for loadcarrying capacity at the connection in accordance with the NDS and the applicable evaluation report for engineered wood products.

4.2 Installation:

Installation of the connectors must be in accordance with this evaluation report and the manufacturer's published installation instructions. In the event of a conflict between this report and the manufacturer's published installation instructions, this report governs.

5.0 CONDITIONS OF USE

The Simpson Strong-Tie face-mount hangers described in this report comply with, or are suitable alternatives to what is specified in, those codes listed in Section 1.0 of this report, subject to the following conditions:

- 5.1 The connectors must be manufactured, identified and installed in accordance with this report and the manufacturer's published installation instructions. A copy of the instructions must be available at the jobsite at all times during installation.
- **5.2** Calculations showing compliance with this report must be submitted to the code official. The calculations must be prepared by a registered design professional where required by the statutes of the jurisdiction in which the project is to be constructed.
- **5.3** Adjustment factors noted in Section <u>4.1</u> and the applicable codes must be considered, where applicable.
- **5.4** Connected wood members and fasteners must comply, respectively, with Sections <u>3.2.2</u> and <u>3.2.3</u> of this report.
- **5.5** Use of connectors with preservative or fire retardant treated lumber must be in accordance with Section 3.2.1 of this report. Use of fasteners with preservative treated or fire retardant treated lumber must be in accordance with Section 3.2.3 of this report.

6.0 EVIDENCE SUBMITTED

- 6.1 Data in accordance with the ICC-ES Acceptance Criteria for Joist Hangers and Similar Devices (AC13), dated October 2018 (editorially revised December 2020).
- **6.2** Tests and calculations in accordance with the ICC-ES Acceptance Criteria for Dowel-Type Threaded Fasteners used in Wood (AC233), dated June 2023.

7.0 IDENTIFICATION

7.1 The products described in this report are identified with a die-stamp or an adhesive label indicating the name of the manufacturer (Simpson Strong-Tie) the model number, and the number of an index evaluation report (<u>ESR-2523</u>) that is used as an identifier for the products addressed in this report. 7.2 The report holder's contact information is the following:

SIMPSON STRONG-TIE COMPANY INC. 5956 WEST LAS POSITAS BOULEVARD PLEASANTON, CALIFORNIA 94588 (800) 999-5099 www.strongtie.com jellis@strongtie.com

	DI	MENSIOI (inches)	NS ¹	FASTENERS ² (Quantity-Type)		ALLOWABLE LOADS ^{3,4,5} (lbf)								
MODEL						Uplift ⁶		Download						
NO.	w	н	в	Header⁵	Joist 0 10		C _D = 1.0		C _D = 1.15		C _D = 1.25			
						$C_{\rm D} = 1.6$	10d	16d	10d	16d	10d	16d		
LU24	1 ⁹ / ₁₆	3 ¹ / ₈	1 ¹ / ₂	4	2-10d x 1 ¹ / ₂	240	465	555	530	630	570	655		
LU26	1 ⁹ / ₁₆	4 ³ / ₄	1 ¹ / ₂	6	4-10d x 1 ¹ / ₂	540	695	835	800	950	860	1,030		
LU28	1 ⁹ / ₁₆	6 ³ / ₈	1 ¹ / ₂	8	6-10d x 1 ¹ / ₂	850	930	1,110	1,065	1,180	1,145	1,180		
LU210	1 ⁹ / ₁₆	7 ¹³ / ₁₆	1 ¹ / ₂	10	6-10d x 1 ¹ / ₂	850	1,160	1,390	1,330	1,580	1,430	1,615		

TABLE 1—ALLOWABLE LOADS FOR THE LU SERIES JOIST HANGERS

For **SI:** 1 inch = 25.4 mm, 1 lbf = 4.45 N.

¹Refer to Figure 1 for definitions of hanger nomenclature (W, H, B).

²Refer to Section <u>3.2.3</u> of this report for nail sizes and required minimum physical properties.

³Tabulated allowable loads must be selected based on duration of load as permitted by the applicable building code.

⁴LU Series hangers provide torsional resistance, which is defined as a moment of not less than 75 pounds (334 N) times the depth of the joist at which the lateral movement of the top or bottom of the joist with respect to its vertical position is 0.125 inch (3.2 mm). The height, H, of the joist hanger must be at least 60 percent of the height of the joist unless additional lateral restraint is provided, as designed by others.

⁵The quantity of 10d or 16d common nails specified in the "Header" column under "Fasteners" is required to achieve the tabulated allowable loads shown in the Allowable Download "10d" or "16d" columns.

⁶Allowable uplift loads are for hangers installed with either 10d or 16d common nails into the supporting header/beam, and have been increased for wind or earthquake loading with no further increase allowed. The allowable uplift loads must be reduced when other load durations govern.



FIGURE 1—LU SERIES HANGER (See Table 1)

TABLE 2—DIMENSIONS, NAILING SCHEDULES AND DESIGN VALUES FOR LUCZ SERIES HANGERS

MODEL	DIMEN: (inch	SIONS nes)	FASTEN (Quantity	IERS ¹ /-Type)	ALLOWABLE LOADS ^{2, 3, 4, 5} (lbf)						
No.			Into	Into Supported Member	Uplift ⁶			Download			
~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	w	н	Supporting Member		C _D =1.60	C _D =0.9	C _D =1.0	C _D =1.15	C _D =1.25	C _D =1.60	
LUC26Z 1 ⁹ / ₁₆		6 - 10d x 1 ¹ / ₂		730	640	710	810	875	1,100		
	1 ⁹ / ₁₆	4 ³ / ₄	6 - 10d	4 - 10d x 1 ¹ / ₂	730	640	710	810	875	1,100	
			6 - 16d		730	760	845	965	1,040	1,315	
			10 - 10d x 1 ¹ / ₂		985	1,065	1,185	1,345	1,455	1,830	
LUC210Z	1 ⁹ / ₁₆	<b>7</b> ³ / ₄	10 - 10d	6 - 10d x 1 ¹ / ₂	985	1,065	1,185	1,345	1,455	1,830	
			10 - 16d		985	1,270	1,410	1,605	1,735	2,180	

For SI: 1 inch = 25.4 mm, 1 pound = 4.45 N.

¹Allowable loads correspond to installations where all pre-punched nail holes in the hanger are filled with nails. Refer to Section <u>3.2.3</u> of this report for nail sizes and required minimum physical properties.

²Tabulated allowable loads are for installations in wood members complying with Section 3.2.2 of this report.

³Tabulated loads must be selected based on the applicable load duration factor, C_D, as permitted by the applicable building code. See Sections <u>4.1</u> and <u>4.2</u> for design and installation requirements.

⁴The maximum allowable gap between the joist end and the supporting member is ¹/₈ inch (3.2 mm).

⁵LUCZ series hangers provide torsional resistance, which is defined as a moment of not less than 75 pounds (334 N) times the depth of the joist at which the lateral movement of the top or bottom of the joist with respect to its vertical position is 0.125 inch (3.2 mm), for nominal 2x6 joists supported by the LUC26Z and nominal 2x10 joists supported by the LUC210Z.

⁶Allowable uplift loads have been increased for wind or earthquake loading with no further increase allowed. The tabulated allowable uplift loads must be reduced proportionally when other load durations govern.



FIGURE 2-LUCZ SERIES HANGER AND INSTALLATION DETAIL

	DIN			FAS			-	ALLOW	ABLE LO (lbf)	ADS ^{4,5,6}		
MODEL		(inches)		(Quar	itity-Type)	Uplift ⁶			Dow	nload		
NO.	\ <b>M</b> /		ь	Hoodor ⁵	loist	C _D =	<b>C</b> _D =	= 1.0	C _D =	1.15	C _D =	1.25
	vv	п	В	neauei	30151	1.6	10d	16d	10d	16d	10d	16d
U24	1 ⁹ / ₁₆	3 ¹ / ₈	2	4	2-10d x 1 ¹ / ₂	240	490	575	550	650	590	705
U26	1 ⁹ / ₁₆	4 ³ / ₄	2	6	4-10d x 1 ¹ / ₂	535	730	865	830	980	890	1,055
U210	1 ⁹ / ₁₆	7 ¹³ / ₁₆	2	10	6-10d x 1 ¹ / ₂	990	1,220	1,440	1,380	1,565	1,480	1,565
U214	1 ⁹ / ₁₆	10	2	12	8-10d x 1 ¹ / ₂	990	1,465	1,730	1,655	1,955	1775	2,110
U14	1 ¹³ / ₁₆	10 ¹ / ₄	2	14	6-10d x 1 ¹ / ₂	970	-	2,015	-	2,285	-	2,465
U24R	2 ¹ / ₁₆	3 ⁵ / ₈	2	4	2-10d x 1 ¹ / ₂	240	490	575	550	650	590	705
U26R	2 ¹ / ₁₆	5 ⁵ /8	2	8	4-10d x 1 ¹ / ₂	535	975	1,150	1,105	1,305	1,185	1,410
U210R	2 ¹ / ₁₆	9 ¹ / ₈	2	14	6-10d x 1 ¹ / ₂	990	1,710	2,015	1,930	2,280	2,070	2,465
U3510/14	2 ⁵ / ₁₆	9	2	14	6-10d x 1 ¹ / ₂	970	-	2,015	-	2,285	-	2,465
U3510/20	2 ⁵ / ₁₆	10 ⁵ / ₁₆	2	16	6-10d x 1 ¹ / ₂	970	-	2,305	-	2,615	-	2,820
U34	2 ⁹ / ₁₆	3 ³ / ₈	2	4	2-10d x 1 ¹ / ₂	240	490	575	550	650	590	705
U36	2 ⁹ / ₁₆	5 ³ / ₈	2	8	4-10d x 1 ¹ / ₂	535	975	1,150	1,105	1,305	1,185	1,410
U310	2 ⁹ / ₁₆	8 ⁷ / ₈	2	14	6-10d x 1 ¹ / ₂	990	1,710	2,015	1,930	2,280	2,070	2,465
U314	2 ⁹ / ₁₆	10 ¹ / ₂	2	16	6-10d x 1 ¹ / ₂	990	1,950	2,305	2,210	2,610	2,370	2,815
U24-2	3 1/8	3	2	4	2-10d	240	490	575	550	650	590	705
U26-2	3 1/8	5	2	8	4-10d	535	975	1,150	1,105	1,305	1,185	1,410
U210-2	3 1/8	8 ¹ / ₂	2	14	6-10d	990	1,750	2,015	1,930	2,280	2,070	2,465
U44	3 ⁹ / ₁₆	2 ⁷ / ₈	2	4	2-10d	240	490	575	550	650	590	705
U46	3 ⁹ / ₁₆	4 ⁷ / ₈	2	8	4-10d	535	975	1,150	1,105	1,305	1,185	1,410
U410	3 ⁹ / ₁₆	8 ³ / ₈	2	14	6-10d	990	1,710	2,015	1,930	2,280	2,070	2,465
U414	3 ⁹ / ₁₆	10	2	16	6-10d	990	1,950	2,305	2,210	2,610	2,370	2,815
U44R	4 ¹ / ₁₆	2 ⁵ /8	2	4	2-16d	240	490	575	550	650	590	705
U46R	4 ¹ / ₁₆	4 ⁵ / ₈	2	8	4-16d	535	975	1,150	1,105	1,305	1,185	1,410
U410R	4 ¹ / ₁₆	8 ¹ / ₈	2	14	6-16d	990	1,710	2,015	1,930	2,280	2,070	2,465
U26-3	4 ⁵ / ₈	4 ¹ / ₄	2	8	4-10d	535	975	1,150	1,105	1,305	1,185	1,410
U210-3	4 ⁵ / ₈	7 ³ / ₄	2	14	6-10d	990	1,710	2,015	1,930	2,280	2,070	2,465
U3510-2	4 ³ / ₄	8 ³ / ₄	2	14	6-10d x 1 ¹ / ₂	970	-	2,015	-	2,285	-	2,465
U3512-2	4 ³ / ₄	11 ¹ / ₄	2	16	6-10d x 1 ¹ / ₂	970	-	2,305	-	2,615	-	2,820
U66	5 ¹ / ₂	5	2	8	4-10d	535	975	1,150	1,105	1,305	1,185	1,410
U610	5 ¹ / ₂	8 ¹ / ₂	2	14	6-10d	990	1,710	2,015	1,930	2,280	2,070	2,465
U610	5 ¹ / ₂	8 ¹ / ₂	2	14	6-10d x 1 ¹ / ₂	970	1,705	2,015	1,930	2,285	2,075	2,465
U66R	6	5	2	8	4-16d	535	975	1,150	1,105	1,305	1,185	1,410
U610R	6	8 ¹ / ₂	2	14	6-16d	990	1,710	2,015	1,930	2,280	2,070	2,465

#### TABLE 3—ALLOWABLE LOADS FOR THE U SERIES JOIST HANGERS

For **SI:** 1 inch = 25.4 mm, 1 lbf = 4.45 N.

¹Refer to Figure 3 (this page) for definitions of hanger nomenclature (W, H, B).

²Refer to Section <u>3.2.3</u> of this report for nail sizes and required minimum physical properties.

³Tabulated allowable loads must be selected based on duration of load as permitted by the applicable building code.

⁴U Series hangers provide torsional resistance, which is defined as a moment of not less than 75 pounds (334 N) times the depth of the joist at which the lateral movement of the top or bottom of the joist with respect to the vertical position of the joist is 0.125 inch (3.2 mm). The height, H, of the joist hanger must be at least 60 percent of the height of the joist unless additional lateral restraint is provided as designed by others.

60 percent of the height of the joist unless additional lateral restraint is provided, as designed by others. ⁵The quantity of 10d or 16d common nails specified in the "Header" column under "Fasteners" is required to achieve the tabulated allowable loads shown in the Allowable Download "10d" or "16d" columns.

⁶Allowable uplift loads are for hangers installed with either 10d or 16d common nails into the supporting header/beam, and have been increased for wind or earthquake loading with no further increase allowed. The allowable uplift loads must be reduced when other load durations govern.



FIGURE 3-U SERIES JOIST HANGER

	HANG	ER DIMENS (inches)	IONS ¹	FAS (Qua	STENERS ²	AI	LOWABLE	LOADS (lbf) ²	2,3,4
MODEL NO.						Uplift⁵		Download	
	W	Н	В	Header	Joist	C _D = 1.6	C _D = 1.0	C _D = 1.15	C _D = 1.25
HU26		3 ¹ / ₁₆		4-16d	2-10d x 1 ¹ / ₂	305	595	670	720
HU28		5 ¹ / ₄		6-16d	4-10d x 1 ¹ / ₂	605	895	1,010	1,080
HU210	497	7 ¹ / ₈	01/	8-16d	4-10d x 1 ¹ / ₂	605	1,190	1,345	1,440
HU212	1% ₁₆	9	21/4	10-16d	6-10d x 1 ¹ / ₂	1,135	1,490	1,680	1,800
HU214		10 ¹ / ₈		12-16d	6-10d x 1 ¹ / ₂	1,135	1,790	2,015	2,160
HU216		12 ¹⁵ / ₁₆		18-16d	8-10d x 1 ¹ / ₂	1,510	2,680	3,025	3,240
HU1.81/5		5 ³ / ₈		16-16d	6-10d x 1 ¹ / ₂	915	2,380	2,685	2,890
HU7		6 ¹¹ / ₁₆		16-16d	8-10d x 1 ¹ / ₂	1,515	2,380	2,685	2,890
HU9	1 ¹³ / ₁₆	9 ⁵ / ₁₆	2 ¹ / ₂	24-16d	10-10d x 1 ¹ / ₂	1,795	3,570	4,030	4,335
HU11		11 ¹ / ₁₆		30-16d	10-10d x 1 ¹ / ₂	1,795	4,465	4,705	4,810
HU14		13 ¹¹ / ₁₆		36-16d	14-10d x 1 ¹ / ₂	1,795	5,055	5,275	5,420
HU2.1/9	<b>2</b> ¹ /	9 ³ / ₁₆	21/	14-16d	6-10d x 1 ¹ / ₂	915	2,085	2,350	2,530
HU2.1/11	Z /8	11	2 /2	16-16d	6-10d x 1 ¹ / ₂	915	2,380	2,685	2,890
HU359		8 ¹⁵ / ₁₆		18-16d	10-10d x 1 ¹ / ₂	1,795	2,680	3,020	3,250
HU3511	2 ³ / ₈	11 ¹ / ₁₆		22-16d	10-10d x 1 ¹ / ₂	1,795	3,275	3,695	3,970
HU3514		13 ¹ / ₂	2 ¹ / ₂	24-16d	12-10d x 1 ¹ / ₂	1,795	3,570	4,030	4,335
HU3516/22		14 ¹ / ₄		20-16d	8-10d x 1 ¹ / ₂	1,515	2,975	3,360	3,610
HU3524/30		18		24-16d	14-10d x 1 ¹ / ₂	1,795	3,570	4,030	4,335
HU34		3 ³ / ₈		4-16d	2-10d x 1 ¹ / ₂	380	595	670	720
HU36		5 ³ / ₁₆		8-16d	4-10d x 1 ¹ / ₂	605	1,190	1,345	1,440
HU38		6 ⁹ / ₁₆		10-16d	4-10d x 1 ¹ / ₂	605	1,490	1,680	1,800
HU310	2 ⁹ / ₁₆	8 ⁷ / ₈	2 ¹ / ₂	14-16d	6-10d x 1 ¹ / ₂	915	2,085	2,350	2,530
HU312		10 ⁵ /8		16-16d	6-10d x 1 ¹ / ₂	915	2,380	2,685	2,890
HU314		12 ³ / ₈		18-16d	8-10d x 1 ¹ / ₂	1,515	2,680	3,020	3,250
HU316		14 ¹ / ₈		20-16d	8-10d x 1 ¹ / ₂	1,515	2,975	3,360	3,610
HU2.75/10		9		18-16d	10-10d x 1 ¹ / ₂	1,795	2,680	3,020	3,250
HU2.75/12	2 ³ /4	10 ³ / ₄	2 ¹ /2	22-16d	10-10d x 1 ¹ / ₂	1,795	3,275	3,695	3,970
HU2.75/14	∠ /4	13	212	24-16d	14-10d x 1 ¹ / ₂	1,795	3,570	4,030	4,335
HU24-2		3 ¹ / ₁₆		4-16d	2-10d	380	595	670	720
HU26-2		4 ¹⁵ / ₁₆		8-16d	4-10d	755	1,190	1,345	1,440
HU28-2		6 ⁵ / ₁₆		10-16d	4-10d	755	1,490	1,680	1,800
HU210-2	3 ¹ /8	8 ⁹ / ₁₆	2 ¹ / ₂	18-16d	10-10d	1,795	2,680	3,020	3,250
HU212-2		10 ⁹ / ₁₆		22-16d	10-10d	1,795	3,275	3,695	3,970
HU214-2		12 ¹ / ₈		18-16d	8-10d	1,510	2,680	3,025	3,240
HU216-2		13 ⁷ / ₈		26-16d	12-10d	1,795	3,870	4,365	4,695
HU3.25/10.5	3 ¹ /4	9	2 ¹ / ₂	22-16d	10-10d	1,795	3,275	3,695	3,970
HU3.25/12		11	.2	24-16d	12-10d	1,795	3,570	4,030	4,335

# TABLE 4—ALLOWABLE LOADS FOR THE HU HANGER SERIES

Please refer to next page for continuation of Table 4.

	TABLE 4 (CONTINUED)—ALLOWABLE LOADS FOR THE HU HANGER SERIES												
	HANG	ER DIMENS	IONS ¹	FAS	STENERS ²				4.5				
MODEL NO		(inches)		(Qua	ntity-Type)	AL	LOWABLE		,-,0				
MODEL NO.	W	u	в	Hoador	loist	Uplift ⁶		Download					
	vv	п	В	neauer	50151	C _D = 1.6	C _D = 1.0	C _D = 1.15	C _D = 1.25				
HU44		2 ⁷ / ₈		4-16d	2-10d	380	595	670	720				
HU46		4 ¹¹ / ₁₆		8-16d	4-10d	755	1,190	1,345	1,440				
HU48		6 ³ / ₁₆		10-16d	4-10d	755	1,490	1,680	1,800				
HU410	3 ⁹ / ₁₆	8 ³ / ₈	2 ¹ / ₂	14-16d	6-10d	1,135	2,085	2,350	2,520				
HU412		10 ⁵ / ₁₆		16-16d	6-10d	1,135	2,385	2,690	2,880				
HU414		11 ⁷ / ₈		18-16d	8-10d	1,510	2,680	3,025	3,240				
HU416		13 ⁵ / ₈		20-16d	8-10d	1,510	2,980	3,360	3,600				
HU4.12/9	A ¹ /.	8 ⁵ / ₈	21/	18-16d	10-10d	1,795	2,680	3,020	3,250				
HU4.12/11	4 /8	10 ⁵ / ₁₆	2 /2	22-16d	10-10d	1,795	3,275	3,695	3,970				
HU4.28/9	19/	9	21/	18-16d	8-10d	1,515	2,680	3,020	3,250				
HU4.28/11	4 732	11	Z /2	22-16d	8-10d	1,515	3,275	3,695	3,970				
HU26-3		4 ⁹ / ₁₆		8-16d	4-10d	755	1,190	1,345	1,440				
HU210-3		8		14-16d	6-10d	1,135	2,085	2,350	2,520				
HU212-3	4 ¹¹ / ₁₆	9 ³ / ₄	2 ¹ / ₂	16-16d	6-10d	1,135	2,385	2,690	2,880				
HU214-3		12 ¹ / ₁₆		18-16d	8-10d	1,510	2,680	3,025	3,240				
HU216-3		13 ⁷ / ₈		20-16d	8-10d	1,510	2,980	3,360	3,600				
HU4.75/9		9		18-16d	8-10d	1,515	2,680	3,020	3,250				
HU4.75/11		11		22-16d	8-10d	1,515	3,275	3,695	3,970				
HU3514-2	4 ³ / ₄	13¹/4	2 ¹ / ₂	18-16d	8-10d	1,515	2,680	3,020	3,250				
HU3516-2		15 ¹ / ₄		26-16d	12-10d	1,795	3,870	4,365	4,695				
HU3520-2		19 ¹ / ₄		26-16d	12-10d	1,795	3,870	4,365	4,695				
HU310-2		7 ¹³ / ₁₆		14-16d	6-10d	1,135	2,085	2,350	2,530				
HU312-2	5 ¹ / ₈	10 ⁵ / ₈	2 ¹ / ₂	16-16d	6-10d	1,135	2,380	2,685	2,890				
HU314-2		12 ⁵ / ₈		18-16d	8-10d	1,510	2,680	3,020	3,250				
HU5.125/12		10 ¹ / ₄		22-16d	8-16d	1,515	3,275	3,695	3,970				
HU5.125/13.5	5 ¹ / ₄	13 ¹ / ₄	2 ¹ / ₂	26-16d	12-16d	2,695	3,870	4,365	4,695				
HU5.125/16		13 ⁷ /8		26-16d	12-16d	2,695	3,870	4,365	4,695				
HU66		4 ³ / ₁₆		8-16d	4-16d	895	1,190	1,345	1,440				
HU68		5 ¹³ / ₁₆		10-16d	4-16d	895	1,490	1,680	1,800				
HU610	<b>F</b> 1/	7 ⁵ /8	01/	14-16d	6-16d	1,345	2,085	2,350	2,520				
HU612	5'/ ₂	9 ³ / ₈	Z 1/2	16-16d	6-16d	1,345	2,385	2,690	2,880				
HU614		11 ⁵ /8		18-16d	8-16d	1,780	2,680	3,025	3,240				
HU616		13 ³ / ₈		20-16d	8-16d	1,780	2,980	3,360	3,600				
HU210-4	6 ¹ / ₈	8 ³ / ₈	2 ¹ / ₂	14-16d	6-16d	1,345	2,085	2,350	2,520				
HU410-2		8 ⁹ / ₁₆		18-16d	8-16d	1795	2680	3020	3250				
HU412-2	7 ¹ / ₈	10 ¹³ / ₁₆	2 ¹ / ₂	22-16d	8-16d	1795	3275	3695	3970				
HU414-2		13 ⁷ / ₈		26-16d	12-16d	2695	3870	4365	4695				
HU88		6 ⁵ / ₈		10-16d	4-16d	895	1,490	1,680	1,800				

For **SI:** 1 inch = 25.4 mm, 1 lbf = 4.45 N.

 $7^{1}/_{2}$ 

HU810

HU812

HU814

HU816

¹Refer to <u>Figures 4a</u> and <u>4b</u> for definitions of hanger nomenclature (W, H, B).

²Refer to Section <u>3.2.3</u> of this report for nail sizes and required minimum physical properties.

**8**³/₈

 $10^{1}/_{8}$ 

11⁷/8

13⁵/8

³HU series hangers with widths (W) equal to or greater than 2⁹/₁₆ inches (65 mm) are available with header flanges turned in (concealed) and are identified with the model designation HUC#. See Figure 4b.

6-16d

6-16d

8-16d

8-16d

1,345

1,345

1,780

1,780

2,085

2,385

2,680

2,680

2,350

2,690

3,025

3,360

2,520

2,880

3,240

3,600

14-16d

16-16d

18-16d

20-16d

⁴Tabulated allowable loads must be selected based on duration of load as permitted by the applicable building code.

 $2^{1}/_{2}$ 

⁵HU Series hangers provide torsional resistance, which is defined as a moment of not less than 75 pounds (334 N) times the depth of the joist at which the lateral movement of the top or bottom of the joist with respect to the vertical position of the joist is 0.125 inch (3.2 mm). The height, H, of the joist hanger must be at least 60 percent of the height of the joist unless additional lateral restraint is provided, as designed by others.

⁶Allowable uplift loads have been increased for wind or earthquake loading with no further increase allowed. The allowable uplift loads must be reduced when other load durations govern.





	DIN	IENSION	S ¹	FASTE	ENERS	AL	LOWABLE L	.OADS ^{2,3,4} (I	bs)
	(	inches)		(Quanti	ty-Type)	Uplift ⁴	Download		
MODEL NO.	W	н	В	Header	Joist	C _D =1.6	C _D =1.0	C _D =1.15	C _D =1.25
HUCQ1.81/9-SDS	113/16	9	3	8-SDS ¹ / ₄ " x 1 ³ / ₄ "	4-SDS ¹ / ₄ " x 1 ³ / ₄ "	1,310	2,000	2,300	2,500
HUCQ1.81/11-SDS	113/16	11	3	10-SDS ¹ / ₄ " x 1 ³ / ₄ "	4-SDS ¹ / ₄ " x 1 ³ / ₄ "	1,310	2,500	2,875	3,125
HUCQ310-SDS	2 ⁹ / ₁₆	9	3	8-SDS ¹ / ₄ " x 2 ¹ / ₂ "	4-SDS ¹ / ₄ " x 2 ¹ / ₂ "	1,220	3,120	3,590	3,685
HUCQ210-2-SDS	31/4	9	3	12-SDS ¹ / ₄ " x 2 ¹ / ₂ "	6-SDS ¹ / ₄ " x 2 ¹ / ₂ "	1,220	4,680	4,865	4,865
HUCQ410-SDS	3 ⁹ / ₁₆	9	3	12-SDS ¹ / ₄ " x 2 ¹ / ₂ "	6-SDS ¹ / ₄ " x 2 ¹ / ₂ "	2,365	4,500	4,500	4,500
HUCQ412-SDS	3 ⁹ / ₁₆	11	3	14-SDS ¹ / ₄ " x 2 ¹ / ₂ "	6-SDS ¹ / ₄ " x 2 ¹ / ₂ "	2,365	5,045	5,045	5,045
HUCQ210-3-SDS	4 ⁵ / ₈	9	3	12-SDS ¹ / ₄ " x 2 ¹ / ₂ "	6-SDS ¹ / ₄ " x 2 ¹ / ₂ "	2,215	4,680	4,865	4,865
HUCQ5.25/9-SDS	5 ¹ / ₄	9	3	12-SDS ¹ / ₄ " x 2 ¹ / ₂ "	6-SDS ¹ / ₄ " x 2 ¹ / ₂ "	2,365	4,500	4,500	4,500
HUCQ5.25/11-SDS	5 ¹ / ₄	11	3	14-SDS ¹ / ₄ " x 2 ¹ / ₂ "	6-SDS ¹ / ₄ " x 2 ¹ / ₂ "	2,365	5,045	5,045	5,045
HUCQ610-SDS	5 ⁹ / ₁₆	9	3	12-SDS ¹ / ₄ " x 2 ¹ / ₂ "	6-SDS ¹ / ₄ " x 2 ¹ / ₂ "	2,365	4,680	5,210	5,210
HUCQ612-SDS	5 ⁹ / ₁₆	11	3	14-SDS ¹ / ₄ " x 2 ¹ / ₂ "	6-SDS ¹ / ₄ " x 2 ¹ / ₂ "	2,365	5,210	5,210	5,210

# TABLE 5—ALLOWABLE LOADS FOR THE HUCQ SERIES JOIST HANGERS

For **SI:** 1 inch = 25.4 mm, 1 lbf = 4.45 N.

 $^1\mbox{Refer}$  to Figure 5 (this page) for definitions of hanger nomenclature (W, H, B).

³The uplift loads have been increased for wind or earthquake loading with no further increase allowed. The allowable uplift loads must be reduced when other load durations govern.

⁴For supported members other than glulam beams, lateral support must be provided at the ends in accordance with Section 2308.4.6 of the IBC or Section R502.7 of the IRC.



FIGURE 5-DIMENSIONS AND TYPICAL INSTALLATION OF THE HUCQ SERIES HANGERS

MODEL	DII	MENSIONS ¹ (inches)		COMMON (Quantity	l NAILS² γ-Type)		ALLOWABLE (lbf)	LOADS ^{3,4}	
NO.	۱۸/	ц	B	Header	loist ⁵	Uplift ⁶		Download	-
	~~~		Ъ	Header	30130	C _D = 1.6	C _D = 1.0	C _D = 1.15	C _D = 1.25
LUS24	1 ⁹ / ₁₆	3 ¹ / ₈	1 ³ /4	4-10d	2-10d	435	670	765	820
LUS26	1 ⁹ / ₁₆	4 ³ / ₄	$1^{3}/_{4}$	4-10d	4-10d	1,165	865	990	1,060
LUS28	1 ⁹ / ₁₆	6 ⁵ /8	$1^{3}/_{4}$	6-10d	4-10d	1,165	1,100	1,260	1,350
LUS210	1 ⁹ / ₁₆	7 ¹³ / ₁₆	$1^{3}/_{4}$	8-10d	4-10d	1,165	1,335	1,530	1,640
LUS36	2 ⁹ / ₁₆	5 ¹ / ₄	2	4-16d	4-16d	1,060	1,030	1,170	1,265
LUS310	2 ⁹ / ₁₆	7 ⁹ / ₃₂	2	6-16d	4-16d	1,070	1,315	1,490	1,610
LUS24-2	3 ¹ / ₈	3 ¹ / ₈	2	4-16d	2-16d	410	800	905	980
LUS26-2	3 ¹ / ₈	4 ¹⁵ / ₁₆	2	4-16d	4-16d	1,060	1,030	1,170	1,265
LUS28-2	3 ¹ / ₈	7	2	6-16d	4-16d	1,060	1,315	1,490	1,610
LUS210-2	3 ¹ / ₈	8 ¹⁵ / ₁₆	2	8-16d	6-16d	1,445	1,830	2,075	2,245
LUS214-2	3 ¹ / ₈	1015/16	2	10-16d	6-16d	1,445	2,110	2,395	2,590
LUS44	3 ⁹ / ₁₆	3	2	4-16d	2-16d	410	800	905	980
LUS46	3 ⁹ / ₁₆	4 ³ / ₄	2	4-16d	4-16d	1,060	1,030	1,170	1,265
LUS48	3 ⁹ / ₁₆	6 ³ /4	2	6-16d	4-16d	1,060	1,315	1,490	1,610
LUS410	3 ⁹ / ₁₆	8 ³ / ₄	2	8-16d	6-16d	1,445	1,830	2,075	2,245
LUS414	3 ⁹ / ₁₆	10 ³ / ₄	2	10-16d	6-16d	1,445	2,110	2,395	2,590
LUS26-3	4 ⁵ /8	$4^{1}/_{8}$	2	4-16d	4-16d	1,060	1,030	1,170	1,265
LUS28-3	4 ⁵ /8	6 ¹ / ₄	2	6-16d	4-16d	1,060	1,315	1,490	1,610
LUS210-3	4 ⁵ /8	813/16	2	8-16d	6-16d	1,445	1,830	2,075	2,245

TABLE 6—ALLOWABLE LOADS FOR THE LUS SERIES JOIST HANGERS

For SI: 1 inch = 25.4 mm, 1 lbf = 4.45 N.

¹Refer to Figure 6 (this page) for definitions of hanger nomenclature (W, H, B).

²Refer to Section <u>3.2.3</u> of this report for nail sizes and required minimum physical properties.

³Tabulated allowable loads must be selected based on duration of load as permitted by the applicable building code.

⁴LUS Series hangers provide torsional resistance, which is defined as a moment of not less than 75 pounds (334 N) times the depth of the joist at which the lateral movement of the top or bottom of the joist with respect to the vertical position of the joist is 0.125 inch (3.2 mm). The height, H, of the joist at which the tateral 60 percent of the height of the joist unless additional lateral restraint is provided, as designed by others. 5 Joist nails must be driven at a 45 degree angle through the joist into the header/beam (double shear nailing) to achieve the tabulated loads.

⁶Allowable uplift loads have been increased for wind or earthquake loading with no further increase is allowable uplift loads must be reduced when other load durations govern.



FIGURE 6—LUS SERIES HANGER (See Table 6)

MODEL	D	IMENSION (inches)	S ¹	COMMO (Quantit	N NAILS ² ty-Type)	ALLOWABLE LOADS ^{3,4} (lbf)				
NO.	W/	и в и		Heador Joist ⁵		Uplift ⁶	Download			
	vv	п	D	neauer	JOIST	C _D = 1.6	C _D = 1.0	$C_{D} = 1.15$	C _D = 1.25	
MUS26	1 ⁹ / ₁₆	5 ³ / ₁₆	2	6–10d	6–10d	930	1,295	1,480	1,560	
MUS28	1 ⁹ / ₁₆	6 ³ / ₄	2	8–10d	8–10d	1,320	1,730	1,975	2,125	

For SI: 1 inch = 25.4 mm, 1 lbf = 4.45 N.

¹Refer to Figure 7 (this page) for definitions of hanger nomenclature (W, H, B).

²Refer to Section <u>3.2.3</u> of this report for nail sizes and required minimum physical properties.

³Tabulated allowable loads must be selected based on duration of load as permitted by the applicable building code.

⁴MUS series hangers provide torsional resistance, which is defined as a moment of not less than 75 pounds (334 N) times the depth of the joist at which the lateral movement of the top or bottom of the joist with respect to the vertical position of the joist is 0.125 inch (3.2 mm). The height, H, of the joist hanger must be at least 60 percent of the height of the joist unless additional lateral restraint is provided, as designed by others.

⁵Joist nails must be driven at a 45 degree angle through the joist into the header/beam (double shear nailing) to achieve the tabulated loads.

⁶Allowable uplift loads have been increased for wind or earthquake loading with no further increase is allowed. The allowable uplift loads must be reduced when other load durations govern.



FIGURE 7—MUS HANGER (See Table 7)

MODEL	DI	MENSION (inches)	IS ¹	COMMO (Quantit	N NAILS ² ty-Type)		ALLOWABLE LOADS ^{3,4,7} (lbf)				
NO.						Uplift ⁶	Download				
	w	н	В	Header	Joist⁵	C _D = 1.6	C _D = 1.0	C _D = 1.15	C _D = 1.25		
HUS26	1 ⁵ /8	5 ³ / ₈	3	14-16d	6-16d	1,320	2,735	3095	3230		
HUS28	1 ⁵ /8	7 ¹ / ₁₆	3	22-16d	8-16d	1,760	4095	4095	4095		
HUS210	1 ⁵ /8	9 ¹ / ₁₆	3	30-16d	10-16d	2,635	5,450	5,795	5,830		
HUS1.81/10	1 ¹³ / ₁₆	8 ⁷ / ₈	3	30-16d	10-16d	2,675	5,510	5,830	5,830		
HUS46	3 ⁹ / ₁₆	4 ⁵ / ₁₆	2	4-16d	4-16d	1,165	1,090	1,225	1,320		
HUS48	3 ⁹ / ₁₆	6 ¹⁵ / ₁₆	2	6-16d	6-16d	1,320	1,630	1,840	1,980		
HUS410	3 ⁹ / ₁₆	8 ¹⁵ / ₁₆	2	8-16d	8-16d	3,265	2,175	2,455	2,640		
HUS412	3 ⁹ / ₁₆	10 ³ / ₄	2	10-16d	10-16d	3,435	2,720	3,070	3,300		
HUS26-2	3 ¹ / ₈	5 ³ / ₁₆	2	4-16d	4-16d	1,165	1,090	1,225	1,320		
HUS28-2	3 ¹ / ₈	7 ³ / ₁₆	2	6-16d	6-16d	1,320	1,630	1,840	1,980		
HUS210-2	3 ¹ / ₈	9 ³ / ₁₆	2	8-16d	8-16d	3,285	2,175	2,455	2,640		
HUS212-2	31/8	11	2	10-16d	10-16d	3,435	2,720	3,070	3,300		

TABLE 8—ALLOWABLE LOADS FOR THE HUS SERIES HANGERS

For SI: 1 inch = 25.4 mm, 1 pound = 4.45 N.

¹Refer to Figure 8 (this page) for definitions of hanger nomenclature (W, H, B).
²Refer to Section <u>3.2.3</u> of this report for nail sizes and required minimum physical properties.
³Tabulated allowable loads must be selected based on duration of load as permitted by the applicable building code.
⁴HUS series hangers provide torsional resistance, which is defined as a moment of not less than 75 pounds (334 N) times the depth of the joist at which the lateral movement of the top or bottom of the joist with respect to the vertical position of the joist is 0.125 inch (3.2 mm). The height, H, of the joist hanger must be at least 60 percent of the height of the joist unless additional lateral restraint is provided, as designed by others.

⁵ Joist nails must be driven at a 45 degree angle through the joist into the header/beam (double shear nailing) to achieve the tabulated loads.

⁶Allowable uplift loads have been increased for wind or earthquake loading with no further increase is allowed. The allowable uplift loads must be reduced when other load durations govern.



FIGURE 8—HUS SERIES HANGER (See Table 8)

	DI	MENSION (inches)	S1	COMMO (Quanti	N NAILS² ty-Type)	ALLOWABLE LOADS ^{3,4} (lbf)					
MODEL NO.	14/		Р	Lloodor	loist ⁵	Uplift ⁶		Download			
	vv	п	Б	Header	JOISL	C _D = 1.6	C _D = 1.0	C _D = 1.15	C _D = 1.25		
HHUS26-2	3 ⁵ / ₁₆	5 ⁷ / ₁₆	3	14-16d	6-16d	1,320	2,820	3,185	3,425		
HHUS28-2	3⁵/ ₁₆	7 ¹ / ₂	3	22-16d	8-16d	1,760	4,255	4,800	5,165		
HHUS210-2	3 ⁵ / ₁₆	9 ¹ / ₈	3	30-16d	10-16d	3,550	5,690	6,420	6,485		
HHUS46	3 ⁵ /8	5 ¹ / ₄	3	14-16d	6-16d	1,320	2,820	3,185	3,425		
HHUS48	3 ⁵ /8	7 ¹ /8	3	22-16d	8-16d	1,760	4,255	4,800	5,165		
HHUS410	3 ⁵ /8	9	3	30-16d	10-16d	3,550	5,690	6,420	6,485		
HHUS5.50/10	5 ¹ / ₂	9	3	30-16d	10-16d	3,550	5,690	6,420	6,485		
HHUS7.25/10	7 ¹ / ₄	9	3 ⁵ / ₁₆	30-16d	10-16d	3,550	5,690	6,420	6,485		

TABLE 9-ALLOWABLE LOADS FOR THE HHUS SERIES HANGERS

For SI: 1 inch = 25.4 mm, 1 lbf = 4.45 N.

¹Refer to Figure 9 (this page) for definitions of hanger nomenclature (W, H, B).
²Refer to Section <u>3.2.3</u> of this report for nail sizes and required minimum physical properties.
³Tabulated allowable loads must be selected based on duration of load as permitted by the applicable building code.
⁴HUS series hangers provide torsional resistance, which is defined as a moment of not less than 75 pounds (334 N) times the depth of the joist at which the lateral movement of the top or bottom of the joist with respect to the vertical position of the joist is 0.125 inch (3.2 mm). The height, H, of the joist hanger must be at least 60 percent of the height of the joist unless additional lateral restraint is provided, as designed by others.

⁵ Joist nails must be driven at a 45 degree angle through the joist into the header/beam to achieve the tabulated loads. ⁶Allowable uplift loads have been increased for wind or earthquake loading with no further increase is allowed. The allowable uplift loads must be reduced when other load durations govern.



FIGURE 9—HHUS SERIES HANGER (See Table 9)

TABLE 10—ALLOWABLE LOADS FOR T	THE HGUS SERIES JOIST HANGERS
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	DIN	MENSIONS ¹		FASTEN	IERS ²		ALLOWABLE	LOADS ^{3,5,6} (lbs)	
MODEL NO.		(Inches)	r	(Quantity	/-Туре)	Uplift ⁴		Download	
	W	Н	В	Header	Joist ³	C _D = 1.6	C _D = 1.0	C _D = 1.15	C _D = 1.25
HGUS26	1 ⁵ /8	5 ³ /8	5	20-16d	8-16d	1,040	4,355	4,875	5,230
HGUS28	1 ⁵ /8	7 ¹ / ₈	5	36-16d	12-16d	1,650	7,275	7,275	7,275
HGUS210	1 ⁵ /8	9 ¹ / ₈	5	46-16d	16-16d	2,090	9,100	9,100	9,100
HGUS2.75/10	2 ³ / ₄	8 ⁷ / ₈	4	46-16d	16-16d	4,095	9,100	9,100	9,100
HGUS2.75/12	2 ³ / ₄	10 ⁷ / ₈	4	56-16d	20-16d	5,205	11,915	13,050	13,545
HGUS2.75/14	2 ³ /4	12 ⁷ / ₈	4	66-16d	22-16d	5,360	12,900	13,700	14,240
HGUS3.25/10	31/4	8 ⁵ /8	4	46-16d	16-16d	4,095	9,100	9,100	9,100
HGUS3.25/12	3 ¹ / ₄	10 ⁵ /8	4	56-16d	20-16d	5,205	11,915	13,330	14,290
HGUS26-2	3 ⁷ / ₁₆	5 ⁷ / ₁₆	4	20-16d	8-16d	2,155	4,355	4,875	5,230
HGUS28-2	3 ⁷ / ₁₆	7 ³ / ₁₆	4	36-16d	12-16d	3,235	7,460	7,460	7,460
HGUS210-2	3 ⁷ / ₁₆	9 ³ / ₁₆	4	46-16d	16-16d	4,095	9,100	9,100	9,100
HGUS46	3 ⁵ /8	5 ⁵ / ₁₆	4	20-16d	8-16d	2,155	4,355	4,875	5,230
HGUS48	3 ⁵ /8	7 ¹ / ₁₆	4	36-16d	12-16d	3,235	7,460	7,460	7,460
HGUS410	3 ⁵ / ₈	9 ¹ / ₁₆	4	46-16d	16-16d	4,095	9,100	9,100	9,100
HGUS412	3 ⁵ /8	10 ⁷ / ₁₆	4	56-16d	20-16d	5,205	11,915	13,330	14,290
HGUS414	3 ⁵ /8	12 ⁷ / ₁₆	4	66-16d	22-16d	5,360	13,860	14,350	14,350
HGUS26-3	4 ¹⁵ / ₁₆	5 ⁷ / ₁₆	4	20-16d	8-16d	2,155	4,355	4,875	5,230
HGUS28-3	4 ¹⁵ / ₁₆	7 ³ / ₁₆	4	36-16d	12-16d	3,235	7,460	7,460	7,460
HGUS210-3	4 ¹⁵ / ₁₆	9 ³ / ₁₆	4	46-16d	16-16d	4,095	9,100	9,100	9,100
HGUS212-3	4 ¹⁵ / ₁₆	10 ³ / ₄	4	56-16d	20-16d	5,205	11,915	13,330	14,290
HGUS214-3	4 ¹⁵ / ₁₆	12 ³ /4	4	66-16d	22-16d	5,360	13,860	14,350	14,350
HGUS5.25/10	5 ¹ / ₄	9 ¹ / ₁₆	4	46-16d	16-16d	4,095	9,100	9,100	9,100
HGUS5.25/12	5 ¹ /4	10 ⁵ /8	4	56-16d	20-16d	5,205	11,915	13,330	14,290
HGUS5.5/8	5 ¹ / ₂	6 ¹⁵ / ₁₆	4	36-16d	12-16d	3,235	7,460	7,460	7,460
HGUS5.5/10	5 ¹ / ₂	8 ¹⁵ / ₁₆	4	46-16d	16-16d	4,095	9,100	9,100	9,100
HGUS5.5/12	5 ¹ / ₂	10 ¹ / ₂	4	56-16d	20-16d	5,205	11,915	13,330	14,290
HGUS5.5/14	5 ¹ / ₂	12 ¹ / ₂	4	66-16d	22-16d	5,515	9,695	9,695	9,695
HGUS5.62/10	5 ⁵ /8	8 ⁷ /8	4	46-16d	16-16d	4,095	9,100	9,100	9,100
HGUS5.62/12	5⁵/ ₈	107/16	4	56-16d	20-16d	5,205	11,915	13,330	14,290
HGUS5.62/14	55/8	12 ⁷ / ₁₆	4	66-16d	22-16d	5,515	9,695	9,695	9,695
HGUS26-4	6 ⁹ / ₁₆	5 ⁷ / ₁₆	4	20-16d	8-16d	2,155	4,355	4,875	5,230
HGUS28-4	6 ⁹ / ₁₆	7 ³ / ₁₆	4	36-16d	12-16d	3,235	7,460	7,460	7,460
HGUS210-4	6 ⁹ / ₁₆	9 ³ / ₁₆	4	46-16d	16-16d	4,095	9,100	9,100	9,100
HGUS212-4	6 ⁹ / ₁₆	10 ⁹ / ₁₆	4	56-16d	20-16d	5,205	11,915	13,330	14,290
HGUS214-4	6 ⁹ / ₁₆	12 ⁹ / ₁₆	4	66-16d	22-16d	5,360	13,735	14,360	14,360
HGUS6.88/10	6 ⁷ /8	8 ¹³ / ₁₆	4	46-16d	16-16d	4,095	9,100	9,100	9,100
HGUS6.88/12	6 ⁷ / ₈	10 ¹³ / ₁₆	4	56-16d	20-16d	5,205	11,915	13,330	14,290
HGUS6.88/14	6 ⁷ /8	12 ¹³ / ₁₆	4	66-16d	22-16d	5,515	9,695	9,695	9,695
HGUS7.25/8	7 ¹ / ₄	7 ¹ / ₄	4	36-16d	12-16d	3,235	7,460	7,460	7,460
HGUS7.25/10	7 ¹ / ₄	8 ⁵ / ₈	4	46-16d	16-16d	4,095	9,100	9,100	9,100
HGUS7.25/12	7 ¹ / ₄	10 ⁵ /8	4	56-16d	20-16d	5,205	11,915	13,330	14,290
HGUS7.25/14	7 ¹ / ₄	12 ⁵ / ₈	4	66-16d	22-16d	5,515	9,695	9,695	9,695
HGUS7.37/10	7 ³ /8	8 ⁹ / ₁₆	4	46-16d	16-16d	4,065	9,095	9,095	9,095
HGUS7.37/12	7 ³ / ₈	109/16	4	56-16d	20-16d	5,205	11,915	13,330	14,290
HGUS7.37/14	7 ³ / ₈	12 ⁹ / ₁₆	4	66-16d	22-16d	5,515	13,735	14,360	14,360

For **SI:** 1 inch = 25.4 mm, 1 lbf = 4.45 N.

¹Refer to Figure 10 (this page) for definitions of hanger nomenclature (W, H, B).

³Tabulated allowable loads must be selected based on duration of load as permitted by the applicable building code. ⁴Joist nails must be driven at a 45 degree angle through the joist into the header/beam (double shear nailing) to achieve tabulated loads. ⁵Allowable uplift loads have been increased for wind or earthquake loading with no further increase allowed. The allowable uplift loads must be reduced when

⁶HGUS series hangers provide torsional resistance, which is defined as the moment of not less than 75 pounds (334 N) times the depth of the joist at which the lateral movement of the top or bottom of the joist with respect to the vertical position of the joist is 0.125" (3.2 mm). The height, H, of the joist hanger must be at least 60 percent of the height of the joist unless additional lateral restraint is provided, as designed by others.



FIGURE 10—HGUS SERIES JOIST HANGER

TABLE 11A—DIMENSIONS, NAILING SCHEDULES AND DESIGN VALUES FOR HTU SERIES HANGERS (¹/₂ Inch Maximum Gap between Supporting Member and Supported Member – Maximum Number of Nails into Supporting Member)

	DIM (ENSIO inches	NS ³)	FASTE (Quantit	ALLOWABLE LOADS 5, 6, 7 (lbf)						
MODEL No.			_	Into	Into	Uplift ⁸			Download	1	
	w	н	В	Supporting Member	Supported Member	C _D =1.6	C _D =0.9	C _D =1.0	C _D =1.15	C _D =1.25	C _D =1.6
				Sing	le 2X Sizes						
	1 ⁵ / ₈	5 ⁷ / ₁₆	3 ¹ / ₂	20-16d	11-10d x 1 ¹ / ₂	635	2,395	2,395	2,395	2,395	2,395
111020 (7 ₂ Gap – Milt Nail)	1 ⁵ /8	5 ⁷ / ₁₆	3 ¹ / ₂	20-16d	14-10d x 1 ¹ / ₂	1,175	2,640	2,940	3,100	3,100	3,100
HTU26 (1/2" Gap – Max Nail)2	1 ⁵ / ₈	5 ⁷ / ₁₆	3 ¹ / ₂	20-16d	20-10d x 1 ¹ / ₂	1,215	2,640	2,940	3,320	3,580	3,630
HTU28 (1/2" Gap – Min Nail)1	1 ⁵ / ₈	7 ¹ / ₁₆	3 ¹ / ₂	26-16d	14-10d x 1 ¹ / ₂	1,110	3,430	3,770	3,770	3,770	3,770
HTU28 (1/2" Gap – Max Nail)2	1 ⁵ / ₈	7 ¹ / ₁₆	3 ¹ / ₂	26-16d	26-10d x 1 ¹ / ₂	1,920	3,430	3,820	4,315	4,655	5,015
HTU210 (¹ / ₂ " Gap – Min Nail) ¹	1 ⁵ / ₈	9 ¹ / ₁₆	3 ¹ / ₂	32-16d	14-10d x 1 ¹ / ₂	1250	3,600	3,600	3,600	3,600	3,600
HTU210 (1/2" Gap – Max Nail)2	1 ⁵ /8	9 ¹ / ₁₆	3 ¹ / ₂	32-16d	32-10d x 1 ¹ / ₂	3255	4,225	4,705	5,020	5,020	5,020
				Doub	le 2X Sizes						
HTU26-2 (1/2" Gap – Min Nail)1	3 ⁵ / ₁₆	5 ⁷ / ₁₆	3 ¹ / ₂	20-16d	14-10d	1,515	2,640	2,940	3,320	3,500	3,500
HTU26-2 (1/2" Gap – Max Nail)2	3 ⁵ / ₁₆	5 ⁷ / ₁₆	3 ¹ / ₂	20-16d	20-10d	1,910	2,640	2,940	3,320	3,500	3,500
HTU28-2 (1/2" Gap – Min Nail)1	3 ⁵ / ₁₆	7 ¹ / ₁₆	3 ¹ / ₂	26-16d	14-10d	1,490	3,430	3,820	3,980	3,980	3,980
HTU28-2 (1/2" Gap – Max Nail)2	3 ⁵ / ₁₆	7 ¹ / ₁₆	3 ¹ / ₂	26-16d	26-10d	3,035	3,430	3,820	4,315	4,655	5,520
HTU210-2 (1/2" Gap – Min Nail)1	3 ⁵ / ₁₆	9 ¹ / ₁₆	3 ¹ / ₂	32-16d	14-10d	1,755	4,225	4,255	4,255	4,255	4,255
HTU210-2 (1/2" Gap – Max Nail)2	3 ⁵ / ₁₆	9 ¹ / ₁₆	3 ¹ / ₂	32-16d	32-10d	3,855	4,225	4,705	5,310	5,730	6,470

For **SI:** 1 inch = 25.4 mm, 1 pound = 4.45 N.

¹The suffix '(¹/₂" Gap – Min Nail)' corresponds to installed conditions where the gap between the supporting member and supported member is more than ¹/₈ inch (3.2 mm) and less than or equal to ¹/₂ inch (12.7 mm), and, at a minimum, the number of nails specified in the table above are installed into the supported wood truss. Refer to Figure <u>11B</u> on page 19 for a typical installation detail.

²The suffix '(¹/₂" Gap – Max Nail)' corresponds to installed conditions where the gap between the supporting member and supported member is more than ¹/₈ inch (3.2 mm) and less than or equal to ¹/₂ inch (12.7 mm), and all of the pre-punched nail holes in the U-shaped portion of the hanger supporting the truss (joist) are filled with nails. This is designated in the table as "Max Nail" and is shown in Figure <u>11B</u>.

³Refer to Figure <u>11A</u> for definitions of hanger nomenclature (W, H, B).

⁴Allowable loads correspond to installations where the maximum possible number of nails is driven into the supporting member. Refer to Section <u>3.2.3</u> of this report for nail sizes and required minimum physical properties.

⁵Tabulated allowable loads are for instal1ations in wood members complying with Section <u>3.2.2</u> of this report.

⁶Tabulated loads must be selected based on the applicable load duration factor, C_D, as permitted by the applicable building code. See Sections <u>4.1</u> and <u>4.2</u> for design and installation requirements.

⁷HTU series hangers provide torsional resistance, which is defined as a moment of not less than 75 pounds (334 N) times the depth of the joist at which the lateral movement of the top or bottom of the joist with respect to its vertical position is 0.125 inch (3.2 mm), for joists having a height no greater than the height (H) of the hanger.

⁸Allowable uplift loads have been increased for wind or earthquake loading with no further increase allowed. The tabulated allowable uplift loads must be reduced proportionally when other load durations govern.



FIGURE 11A-HTU SERIES HANGER

TABLE 11B—DIMENSIONS, NAILING SCHEDULES AND DESIGN VALUES FOR HTU SERIES HANGERS (¹/₈ Inch Maximum Gap between Supporting Member and Supported Member – Maximum Number of Nails into Supporting Member)

	DIMENSIONS ³ (inches)			FASTE (Quantit	ALLOWABLE LOADS ^{5, 6, 7} (lbf)						
MODEL No.			_	Into	Into	Uplift ⁸			Download	ł	
	w	н	В	Supporting Member	Supported Member	C _D =1.6	C _D =0.9	C _D =1.0	C _D =1.15	C _D =1.25	C _D =1.6
				Sing	le 2X Sizes						
HTU26 (1/ " Con Min Noil)1	1 ⁵ /8	5 ⁷ / ₁₆	3 ¹ / ₂	20-16d	11-10d x 1 ¹ / ₂	640	2,640	2,670	2,670	2,670	2,670
$H^{-1}O_{2}O(7_{8} \text{ Gap} - \text{Will Nall})$	1 ⁵ / ₈	5 ⁷ / ₁₆	3 ¹ / ₂	20-16d	14-10d x 1 ¹ / ₂	1,250	2,640	2,940	3,200	3,200	3,200
HTU26 (¹ / ₈ " Gap – Max Nail) ²	1 ⁵ /8	5 ⁷ / ₁₆	3 ¹ / ₂	20-16d	20-10d x 1 ¹ / ₂	1,555	2,640	2,940	3,320	3,580	4,010
HTU28 (¹/ଃ" Gap – Min Nail)¹	1 ⁵ /8	7 ¹ / ₁₆	3 ¹ / ₂	26-16d	14-10d x 1 ¹ / ₂	1,235	3,430	3,820	3,895	3,895	3,895
HTU28 (1/8" Gap – Max Nail)2	1 ⁵ /8	7 ¹ / ₁₆	3 ¹ / ₂	26-16d	26-10d x 1 ¹ / ₂	2,020	3,430	3,820	4,315	4,655	5,435
HTU210 (¹ / ₈ " Gap – Min Nail) ¹	1 ⁵ /8	9 ¹ / ₁₆	3 ¹ / ₂	32-16d	14-10d x 1 ¹ / ₂	1,330	4,225	4,300	4,300	4,300	4,300
HTU210 (1/8" Gap – Max Nail)2	1 ⁵ / ₈	9 ¹ / ₁₆	3 ¹ / ₂	32-16d	32-10d x 1 ¹ / ₂	3,315	4,225	4,705	5,310	5,730	5,995
				Dout	ole 2X Sizes						
HTU26-2 (¹ / ₈ " Gap – Min Heel) ¹	3 ⁵ / ₁₆	5 ⁷ / ₁₆	3 ¹ / ₂	20-16d	14-10d	1,515	2,640	2,940	3,320	3,580	3,910
HTU26-2 (¹ / ₈ " Gap – Max Nail) ²	3 ⁵ / ₁₆	5 ⁷ / ₁₆	3 ¹ / ₂	20-16d	20-10d	2,175	2,640	2,940	3,320	3,580	4,480
HTU28-2 (1/ ₈ " Gap – Min Nail)1	3 ⁵ / ₁₆	7 ¹ / ₁₆	3 ¹ / ₂	26-16d	14-10d	1,530	3,430	3,820	4,310	4,310	4,310
HTU28-2 (¹ / ₈ " Gap – Max Nail) ²	3 ⁵ / ₁₆	7 ¹ / ₁₆	3 ¹ / ₂	26-16d	26-10d	3,485	3,430	3,820	4,315	4,655	5,825
HTU210-2 (¹ / ₈ " Gap – Min Nail) ¹	3 ⁵ / ₁₆	9 ¹ / ₁₆	3 ¹ / ₂	32-16d	14-10d	1,755	4,225	4,705	4,815	4,815	4,815
HTU210-2 (1/8" Gap – Max Nail)2	3 ⁵ / ₁₆	9 ¹ / ₁₆	31/2	32-16d	32-10d	4,110	4,225	4,705	5,310	5,730	6,515

For SI: 1 inch = 25.4 mm, 1 pound = 4.45 N.

¹The suffix '(¹/₈" Gap – Min Nail)' corresponds to installed conditions where the gap between the supporting member and supported wood truss is ¹/₈ inch (3.2 mm) or less, and at a minimum, the number of nails specified in the table above are installed into the supported wood truss. Refer to Figure 11B for a typical installation detail.

²The suffix '(¹/₈" Gap – Max Nail)' corresponds to installed conditions where the gap between the supporting member and supported wood truss is ¹/₈ inch (3.2 mm) or less, and the all of the pre-punched nail holes in the U-shaped portion of the hanger supporting the truss (joist) are filled with nails. This is designated in the table as "Max Nail" and is shown in Figure 11B on this page.

³Refer to Figure 11A for definitions of hanger nomenclature (W, H, B).

⁴Allowable loads correspond to installations where the maximum possible number of nails is installed into the supporting member. Refer to Section <u>3.2.3</u> of this report for nail sizes and required minimum physical properties.

⁵Tabulated allowable loads are for installations in wood members complying with Section <u>3.2.2</u> of this report.

⁶Tabulated loads must be selected based on the applicable load duration factor, C_D, as permitted by the applicable building code. See Sections <u>4.1</u> and <u>4.2</u> for design and installation requirements.

⁷HTU series hangers provide torsional resistance, which is defined as a moment of not less than 75 pounds (334 N) times the depth of the joist at which the lateral movement of the top or bottom of the joist with respect to its vertical position is 0.125 inch (3.2 mm), for joists having a height no greater than the height (H) of the hanger.

⁸Allowable uplift loads have been increased for wind or earthquake loading with no further increase allowed. The tabulated allowable uplift loads must be reduced proportionally when other load durations govern.



FIGURE 11B—TYPICAL HTU INSTALLATION

TABLE 11C—DIMENSIONS, NAILING SCHEDULES AND DESIGN VALUES FOR HTU SERIES HANGERS – ALTERNATE INSTALLATION (1/2 Inch Maximum Gap between Supporting Member and Supported Member - Minimum Number of Nails into Supporting Member)

MODEL	DIM (ENSIO inches	NS ³ 5)	FASTENERS ⁴ (Quantity-Type)		ALLOWABLE LOADS ^{5, 6, 7} (lbf)						
NO. ^{1,2}				Into	Into	Uplift ⁸			Download			
	w	Н	В	Supporting Member	Supported Member	C _D =1.6	C _D =0.9	C _D =1.0	C _D =1.15	C _D =1.25	C _D =1.6	
HTU26 (¹/₂" Gap – Min Nail)¹	1 ⁵ / ₈	5 ⁷ / ₁₆	3 ¹ / ₂	10-16d	14-10d x 1 ¹ / ₂	845	1,320	1,470	1,660	1,790	1,875	
HTU26 (1/2" Gap – Max Nail)2	1 ⁵ / ₈	5 ⁷ / ₁₆	3 ¹ / ₂	10-16d	20-10d x 1 ¹ / ₂	1,240	1,320	1,470	1,660	1,790	2,220	
HTU28 (1/2" Gap – Max Nail)2	1 ⁵ / ₈	7 ¹ / ₁₆	3 ¹ / ₂	20-16d	26-10d x 1 ¹ / ₂	1,920	2,640	2,940	3,320	3,580	3,905	
HTU210 (¹ / ₂ " Gap – Max Nail) ²	1 ⁵ /8	9 ¹ / ₁₆	3 ¹ / ₂	20-16d	32-10d x 1 ¹ / ₂	2,880	2,640	2,940	3,320	3,580	3,905	

For SI: 1 inch = 25.4 mm, 1 pound = 4.45 N.

¹The suffix '(¹/₂" Gap – Min Nail)' corresponds to installed conditions where the gap between the supporting member and supported member is more than ¹/₈ inch (3.2 mm) and less than or equal to ¹/₂ inch (12.7 mm), and at a minimum, the number of nails specified in the table above are installed into the supported wood truss. Refer to Figure 11C for an alternate installation detail.

²The suffix '(¹/₂" Gap – Max Nail)' corresponds to installed conditions where the gap between the supporting member and supported member is more than ¹/₈ inch (3.2 mm) and less than or equal to ¹/₂ inch (12.7 mm), and the all of the pre-punched nail holes in the U-shaped portion of the hanger supporting the truss (joist) are filled with nails. This is designated in the table as "Max Nail" and is shown in Figure 11A.

³Refer to Figure <u>11A</u> for definitions of hanger nomenclature (W, H, B).

⁴Allowable loads correspond to installations where the minimum allowable number of nails is installed into the supporting member. Refer to Section <u>3.2.3</u> of this report for nail sizes and required minimum physical properties.

⁵Tabulated allowable loads are for installations in wood members complying with Section <u>3.2.2</u> of this report.

⁶Tabulated loads must be selected based on the applicable load duration factor, C_D, as permitted by the applicable building code. See Sections <u>4.1</u> and <u>4.2</u> for design and installation requirements.

⁷HTU series hangers provide torsional resistance, which is defined as a moment of not less than 75 pounds (334 N) times the depth of the joist at which the lateral movement of the top or bottom of the joist with respect to its vertical position is 0.125 inch (3.2 mm), for joists having a height no greater than the height (H) of the hanger. ⁸Allowable uplift loads have been increased for wind or earthquake loading with no further increase allowed. The tabulated allowable uplift loads must be reduced

proportionally when other load durations govern.



FIGURE 11C—ALTERNATE HTU INSTALLATION

						,						
		DIN	IENSION	IS ¹		FASTE	ENERS	А	LLOWABLE I	LOADS ^{2,6,7} (Ib	os)	
MODEL No.			(inches)	-	-	(Quanti	ty-Type)	Uplift⁵		Download		
MODEL NO.	w	H⁴ (min)	в	h _a ³	а	Header	Joist	C _D =1.6	C _D =1.0	C _D =1.15	C _D =1.25	
LGU3.25-SDS	3 ¹ / ₄											
LGU3.63-SDS	3 ⁵ / ₈	8	4 ¹ / ₂	7 ³ / ₈	3 ¹ / ₄	16 SDS ¹ / ₄ " x 2 ¹ / ₂ "	12 SDS ¹ / ₄ " x 2 ¹ / ₂ "	5,555	6,720	7,310	7,310	
LGU5.25-SDS	5 ¹ / ₄											
MGU3.63-SDS	3 ⁵ / ₈											
MGU5.25-SDS	5 ¹ / ₄											
MGU5.50-SDS	5 ¹ / ₂	9 ¹ / ₄	4 ¹ / ₂	8 ⁵ / ₈	4	24 SDS ¹ / ₄ " x 2 ¹ / ₂ "	16 SDS ¹ / ₄ " x 2 ¹ / ₂ "	7,260	9,450	9,450	9,450	
MGU5.62-SDS	5 ⁵ / ₈					· -						
MGU7.00-SDS	7											
HGU3.63-SDS	3 ⁵ / ₈											
HGU5.25-SDS	5 ¹ / ₄											
HGU5.50-SDS	5 ¹ / ₂											
HGU5.62-SDS	5 ⁵ / ₈	11	5 ¹ / ₄	10 ³ / ₈	4 ³ / ₄	36 SDS ¹ / ₄ " x 2 ¹ / ₂ "	24 SDS ¹ / ₄ " x 2 ¹ / ₂ "	9.460	13,160	13,160	13,160	
HGU7.00-SDS	7											
HGU7.25-SDS	7 ¹ / ₄											
HGU9.00-SDS	9											
HHGU5.25-SDS	5 ¹ / ₄											
HHGU5.50-SDS	5 ¹ / ₂							9,700	18,480	20,455	20,455	
HHGU5.62-SDS	5 ⁵ / ₈	12	51/	103/	13/	44	28					
HHGU7.00-SDS	7	15	574	12 /8	4 /4	SDS ¹ / ₄ " x 2 ¹ / ₂ "	SDS ¹ / ₄ " x 2 ¹ / ₂ "					
HHGU7.25-SDS	7 ¹ / ₄							14,550	18,480	20,455	20,445	
HHGU9.00-SDS	9											

TABLE 12-ALLOWABLE LOADS FOR THE LGU, MGU, HGU AND HHGU SERIES HANGERS

For SI: 1 inch = 25.4 mm, 1 lbf = 4.45 N.

¹Refer to Figure 12 (this page) for definitions of hanger nomenclature (W, H, B, h_a and a).

²Tabulated allowable loads must be selected based on duration of load as permitted by the applicable building code.

³The supporting member (header) height must be at least at high as the flange height, ha.

⁴The H dimension must be specified. The maximum H dimension is 30 inches.

⁵The uplift loads have been increased for wind or earthquake loading with no further increase allowed. The allowable uplift loads must be reduced when other load durations ⁶The allowable loads are based on the use of an engineered wood joist member with an allowable F_c⊥ of 650 psi. When use is to support members having an F_c⊥ of less

than 650 psi, it must be verified that the combination of bearing capacity and joist nail capacity is adequate. ⁷For supported members other than glulam beams, lateral support must be provided at the ends in accordance with Section 2308.4.6 of the IBC or Section R502.7 of the

IRC.





FIGURE 12-DIMENSIONS AND TYPICAL INSTALLATION OF THE LGU, MGU, HGU AND HHGU SERIES HANGERS

		DI	MENSION	IS ¹		FAST	ENERS	ALLOWABLE LOADS ^{2,6,7} (lbs)				
MODEL			(inches)			(Quant	tity-Type)	Uplift⁵		Download		
No.	w	н	В	A ₁	A ₂	Header	Joist	C _D =1.6	C _D =1.0	C _D =1.15	C _D =1.25	
SUR/L24	1 ⁹ / ₁₆	3 ¹ / ₁₆	2	1 ¹ / ₈	1 ¹ / ₄	4-16d	4-10d x 1 ¹ / ₂	395	575	650	705	
SUR/L26	1 ⁹ / ₁₆	5	2	1 ¹ / ₈	1 ⁵ / ₁₆	6-16d	6-10d x 1 ¹ / ₂	675	865	980	1,055	
SUR/L210	1 ⁹ / ₁₆	8	2	1 ¹ / ₈	1 ⁵ / ₁₆	10-16d	10-10d x 1 ¹ / ₂	1,445	1,440	1,635	1,760	
SUR/L214	1 ⁹ / ₁₆	10	2	1 ¹ / ₈	1 ⁵ / ₁₆	12-16d	12-10d x 1 ¹ / ₂	1,890	1,730	1,960	2,115	
SUR/L1.81/9	1 ¹³ / ₁₆	9	3	1 ⁵ /8	2 ⁵ / ₁₆	12-16d	2-10d x 1 ¹ / ₂	210 ^{3,4}	1,730	1,955	1,955	
SUR/L1.81/11	1 ¹³ / ₁₆	11	3	1 ⁵ / ₈	2 ⁵ / ₁₆	16-16d	2-10d x 1 ¹ / ₂	210 ^{3,4}	2,300	2,560	2,560	
SUR/L1.81/14	1 ¹³ / ₁₆	13 ³ / ₄	3	1 ⁵ /8	2 ⁵ / ₁₆	20-16d	2-10d x 1 ¹ / ₂	210 ^{3,4}	2,560	2,560	2,560	
SUR/L2.06/9	2 ¹ / ₁₆	9 ¹ / ₁₆	3 ³ / ₁₆	1 ⁵ / ₈	2 ¹ / ₈	14-16d	2-10d x 1 ¹ / ₂	210 ³	2,015	2,285	2,465	
SUR/L2.06/11	2 ¹ / ₁₆	11 ¹ / ₄	3 ³ / ₁₆	1 ⁵ /8	2 ¹ / ₈	16-16d	2-10d x 1 ¹ / ₂	210 ³	2,300	2,610	2,665	
SUR/L2.06/14	2 ¹ / ₁₆	13 ⁵ /8	3 ³ / ₁₆	1 ⁵ /8	2 ¹ / ₈	18-16d	2-10d x 1 ¹ / ₂	210 ³	2,590	2,665	2,665	
SUR/L2.1/9	2 ¹ / ₈	9 ¹ / ₁₆	3 ³ / ₁₆	1 ⁹ / ₁₆	2 ¹ / ₈	14-16d	2-10d x 1 ¹ / ₂	210 ³	2,015	2,285	2,465	
SUR/L2.1/11	2 ¹ / ₈	11 ³ / ₁₆	3 ³ / ₁₆	1 ⁹ / ₁₆	2 ¹ / ₈	16-16d	2-10d x 1 ¹ / ₂	210 ³	2,300	2,610	2,665	
SUR/L2.1/14	2 ¹ / ₈	13 ⁹ / ₁₆	3 ³ / ₁₆	1 ⁹ / ₁₆	2 ¹ / ₈	18-16d	2-10d x 1 ¹ / ₂	210 ³	2,590	2,665	2,665	
SUR/L2.37/9	2 ³ / ₈	8 ¹⁵ / ₁₆	3 ³ / ₁₆	1 ⁵ / ₁₆	2 ¹ / ₈	14-16d	2-10d x 1 ¹ / ₂	210 ³	2,015	2,285	2,465	
SUR/L2.37/11	2 ³ /8	11 ³ / ₁₆	3 ³ / ₁₆	1 ⁵ / ₁₆	2 ¹ / ₈	16-16d	2-10d x 1 ¹ / ₂	210 ³	2,300	2,610	2,665	
SUR/L2.37/14	2 ³ / ₈	13 ⁷ / ₁₆	3 ³ / ₁₆	1 ⁵ / ₁₆	2 ¹ / ₈	18-16d	2-10d x 1 ¹ / ₂	210 ³	2,590	2,665	2,665	
SUR/L2.56/9	2 ⁹ / ₁₆	8 ¹³ / ₁₆	3 ³ / ₁₆	1 ¹ / ₈	2 ¹ / ₈	14-16d	2-10d x 1 ¹ / ₂	210 ³	2,015	2,285	2,465	
SUR/L2.56/11	2 ⁹ / ₁₆	11 ³ / ₁₆	3 ³ / ₁₆	1 ¹ / ₈	2 ¹ / ₈	16-16d	2-10d x 1 ¹ / ₂	210 ³	2,300	2,615	2,665	
SUR/L2.56/14	2 ⁹ / ₁₆	13 ⁵ / ₁₆	3 ³ / ₁₆	1 ¹ / ₈	2 ¹ / ₈	18-16d	2-10d x 1 ¹ / ₂	210 ³	2,590	2,665	2,665	
SUR/L26-2	3 ¹ / ₈	4 ¹⁵ / ₁₆	2 ⁵ /8	1 ¹ / ₂	2 ³ / ₈	8-16d	4-16d x 2 ¹ / ₂	725	1.150	1,305	1,325	
SUR/L210-2	3 ¹ / ₈	8 ¹¹ / ₁₆	2 ⁵ /8	1 ⁷ / ₁₆	2 ³ / ₈	14-16d	6-16d x 2 ¹ / ₂	1,160	2,015	2,285	2,345	
SUR/L214-2	3 ¹ / ₈	12 ¹¹ / ₁₆	2 ⁷ / ₁₆	1 ⁷ / ₁₆	2 ³ / ₈	18-16d	8-16d x 2 ¹ / ₂	1,505	2,345	2,345	2,345	
SUR/L46	3 ⁹ / ₁₆	4 ³ / ₄	2 ⁵ /8	1	2 ³ / ₈	8-16d	4-16d	730	1,150	1,305	1,325	
SUR/L410	3 ⁹ / ₁₆	8 ¹ / ₂	2 ⁵ /8	1	2 ³ / ₈	14-16d	6-16d	1,160	2,015	2,285	2,345	
SUR/L414	3 ⁹ / ₁₆	12 ¹ / ₂	2 ⁵ /8	1	2 ³ / ₈	18-16d	8-16d	1,490	2,345	2,345	2,345	

TABLE 13—ALLOWABLE LOADS FOR THE SUR/SUL SERIES JOIST HANGERS

For SI: 1 inch = 25.4 mm, 1 lbf = 4.45 N.

¹Refer to Figure 13a (this page) for definitions of hanger nomenclature (W, H, B, A1, A2).

²Tabulated allowable loads must be selected based on duration of load as permitted by the applicable building code.

³Optional triangle holes may be filled (requires web stiffeners for I-joist) for additional uplift resistance. When a total of six 10d x 1¹/₂-inch-long nails are installed into the joist for 9- and 11-inch models, an uplift value of 825 lbs may be used. When a total of eight 10dx1¹/₂" nails are installed into the joist for 14-inch models, an uplift value of 1,190 lbs may be used.

⁴Uplift value based on use of solid sawn lumber or LVL joist member. When using an I-joist as the joist member, allowable uplift must be taken as 140 lbs. ⁵The uplift loads have been increased for wind or earthquake loading with no further increase allowed. The allowable uplift loads must be reduced when other load durations govern.

durations govern. ⁶SUR/L series hangers provide torsional resistance, which is defined as a moment of not less than 75 pounds (334 N) times the depth of the joist at which the lateral movement of the top or bottom of the joist with respect to its vertical position is 0.125 inch (3.2 mm). The height (H) of the hanger must be at least 60 percent of the height of the joist unless additional lateral restraint is provided, as designed by others.

The 2-2x and a 4x SUR/L models are available with the A2 flanges concealed and are specified with the model designation SUR/LC



FIGURE 13A—SUL SERIES JOIST HANGER

FIGURE 13B—TYPICAL SUR HANGER INSTALLATION

MODEL		DIMENS	IONS¹ (i	nches)		FAST (Quanti	ENERS ty-Type)	ALLOWABLE LOADS ^{2,3,6,7} (lbs)				
NO.	w	н	в	Δ.	Δ.	Header	Joist	Uplift⁴		Download		
		••			72	Ticadei	UCIST	C _D = 1.6	C _D = 1.0	C _D = 1.15	C _D = 1.25	
HSUR/L26-2	3 ¹ / ₈	4 ¹⁵ / ₁₆	2 ⁷ / ₁₆	1 ³ / ₄	2 ³ / ₈	12-16d	4-16d x 2 ¹ / ₂	725	1,785	1,795	1,795	
HSUR/L210-2	3 ¹ / ₈	8 ¹¹ / ₁₆	2 ⁷ / ₁₆	1 ³ / ₄	2 ³ / ₈	20-16d	6-16d x 2 ¹ / ₂	1,160	2,975	3,355	3,410	
HSUR/L214-2	3 ¹ / ₈	12 ¹¹ / ₁₆	2 ⁷ / ₁₆	1 ³ / ₄	2 ³ / ₈	26-16d	8-16d x 2 ¹ / ₂	1,490	3,865	4,360	4,685	
HSUR/L46	3 ⁹ / ₁₆	4 ³ / ₄	2 ⁷ / ₁₆	1	2 ³ / ₁₆	12-16d	4-16d	725	1,785	1,795	1,795	
HSUR/L410	3 ⁹ / ₁₆	8 ¹ / ₂	2 ⁷ / ₁₆	1	2 ³ / ₁₆	20-16d	6-16d	1,150	2,975	3,355	3,410	
HSUR/L414	3 ⁹ / ₁₆	12 ¹ / ₂	2 ⁷ / ₁₆	1	2 ³ / ₁₆	26-16d	8-16d	1,490	3,865	4,360	4.685	
HSUR/L4.12/9	4 ¹ / ₈	9	3	1 ⁷ / ₁₆	2 ³ / ₈	12-16d	2-10d x 1 ¹ / ₂	145 ⁴	1,725	1,725	1,725	
HSUR/L4.12/11	4 ¹ / ₈	11 ¹ / ₈	3	1 ⁷ / ₁₆	2 ³ / ₈	16-16d	2-10d x 1 ¹ / ₂	145 ⁴	2,380	2,685	2,885	
HSUR/L4.12/14	4 ¹ / ₈	13 ³ / ₄	3	1 ⁷ / ₁₆	2 ³ / ₈	20-16d	2-10d x 1 ¹ / ₂	145 ⁴	2,975	3,330	3,330	
HSUR/L4.12/16	4 ¹ / ₈	15 ³ / ₄	3	1 ⁷ / ₁₆	2 ³ / ₈	24-16d	2-10d x 1 ¹ / ₂	145 ⁴	3,330	3,330	3,330	
HSUR/L4.28/9	4 ⁵ / ₁₆	9	3	1 ⁷ / ₁₆	2 ³ / ₈	12-16d	2-10d x 1 ¹ / ₂	145 ⁴	1,7255	1,725	1,725	
HSUR/L4.28/11	4 ⁵ / ₁₆	11 ¹ / ₈	3	1 ⁷ / ₁₆	2 ³ / ₈	16-16d	2-10d x 1 ¹ / ₂	145 ⁴	2,380	2,685	2,885	
HSUR/L4.75/9	4 ³ / ₄	8 ¹⁵ / ₁₆	2 ³ / ₄	1 ⁷ / ₁₆	2 ³ / ₈	12-16d	2-10d x 1 ¹ / ₂	145 ⁴	1,725	1,725	1,725	
HSUR/L4.75/11	4 ³ / ₄	10 ¹⁵ / ₁₆	2 ³ / ₄	1 ⁷ / ₁₆	2 ³ / ₈	16-16d	2-10d x 1 ¹ / ₂	145 ⁴	2,380	2,685	2,885	
HSUR/L4.75/14	4 ³ / ₄	13 ³ / ₄	2 ³ / ₄	1 ⁷ / ₁₆	2 ³ / ₈	20-16d	2-10d x 1 ¹ / ₂	145 ⁴	2,975	3,330	3,330	
HSUR/L4.75/16	4 ³ / ₄	15 ³ / ₄	2 ³ / ₄	1 ⁷ / ₁₆	2 ³ / ₈	24-16d	2-10d x 1 ¹ / ₂	145 ⁴	3,330	3,330	3,330	
HSUR/L5.12/9	5 ¹ / ₈	9	2 ¹³ / ₁₆	1 ⁷ / ₁₆	2 ³ / ₈	12-16d	2-10d x 1 ¹ / ₂	145 ⁴	1,725	1,725	1,725	
HSUR/L5.12/11	5 ¹ / ₈	11	2 ¹³ / ₁₆	1 ⁷ / ₁₆	2 ³ / ₈	16-16d	2-10d x 1 ¹ / ₂	145 ⁴	2,380	2,685	2,885	
HSUR/L5.12/14	5 ¹ / ₈	13 ³ / ₄	2 ¹³ / ₁₆	1 ⁷ / ₁₆	2 ³ / ₈	20-16d	2-10d x 1 ¹ / ₂	145 ⁴	2,975	3,330	3,330	
HSUR/L5.12/16	5 ¹ / ₈	15 ³ / ₄	2 ¹³ / ₁₆	1 ⁷ / ₁₆	2 ³ / ₈	24-16d	2-10d x 1 ¹ / ₂	145 ⁴	3,330	3,330	3,330	

TABLE 14-ALLOWABLE LOADS FOR THE HSUR/HSUL SERIES JOIST HANGERS

For SI: 1 inch = 25.4 mm, 1 lbf = 4.45 N.

¹Refer to Figure 14a (this page) for definitions of hanger nomenclature (W, H, B, A1, A2).

²Tabulated allowable loads must be selected based on duration of load as permitted by the applicable building code.

³When I-joists are the supported member as shown in Figure 14b, each I-joist must have web stiffeners installed in accordance with the I-joist manufacturer's evaluation report, and the minimum required quantity and type of nails, as specified in the column entitled FASTENERS must be nailed directly into the web stiffeners and/or I-joist.

⁴Optional triangular holes may be filled (requires web stiffeners) for additional uplift. When a total of six 10d x 1¹/2-inch-long nails are installed into the joist, an uplift value of 505 lbs may be used.

⁵The uplift loads have been increased for wind or earthquake loading with no further increase allowed. The allowable uplift loads must be reduced when other load durations govern.

⁶HSUR/L series hangers provide torsional resistance, which is defined as a moment of not less than 75 pounds (334 N) times the depth of the joist at which the lateral movement of the top or bottom of the joist with respect to its vertical position is 0.125 inch (3.2 mm). The height (H) of the hanger must be at least 60 percent of the height of the joist unless additional lateral restraint is provided, as designed by others. ⁷The 2-2x and a 4x HSUR/L models are available with the A2 flanges concealed and are specified with the model designation HSUR/LC

W



FIGURE 14B—TYPICAL HSUR JOIST HANGER INSTALLATION (Web Stiffeners not shown for Clarity–See Footnote 3 to Table 7)

FIGURE 14A—HSUR JOIST HANGER

MODEL	DI	MENSIOI (Inches)	NS ²	FASTENERS (Quantity-Type)		MODEL	DIMENSIONS ² (Inches)			FASTENERS (Quantity-Type)	
NO.	w	н	в	Header	Joist	NO.	¥	н	в	Header	Joist
						IUS2.37/16(min)	27/10	16	2	14-10d	_
						IUS2.37/16(max)	2 / 16	10	2	16-10d	—
IUS1.81/9.5		9 ¹ / ₂		8-10d	—	IUS2.56/9.25		9 ¹ / ₄		8-10d	—
IUS1.81/11.88		11 ⁷ / ₈		10-10d	—	IUS2.56/9.5		9 ¹ / ₂		8-10d	—
IUS1.81/14(min)	17/	1.1	2	12-10d	—	IUS2.56/11.88		11 ⁷ / ₈		10-10d	—
IUS1.81/14(max)	1/8	14	2	14-10d	—	IUS2.56/14(min)	2 ⁵ /8	1.4	2	12-10d	—
IUS1.81/16(min)		16		14-10d	_	IUS2.56/14(max)		14		14-10d	_
IUS1.81/16(max)		10		16-10d	—	IUS2.56/16(min)		10		14-10d	—
IUS2.06/9.5		9 ¹ / ₂		8-10d	-	IUS2.56/16(max)		10		16-10d	_
IUS2.06/11.88		11 ⁷ /8		10-10d	—	IUS3.56/9.5		9 ¹ / ₂		10-10d	—
IUS2.06/14(min)	21/	1.1	2	12-10d	-	IUS3.56/11.88		11 ⁷ / ₈		12-10d	-
IUS2.06/14(max)	∠ /8	14	2	14-10d		IUS3.56/14(min)	25/	14	2	12-10d	
IUS2.06/16(min)		16		14-10d	—	IUS3.56/14(max)	378	14	2	14-10d	—
IUS2.06/16(max)		10		16-10d	_	IUS3.56/16(min)		16		14-10d	_
IUS2.37/9.5		9 ¹ / ₂		8-10d	—	IUS3.56/16(max)		10		16-10d	_
IUS2.37/11.88	07/	11 ⁷ / ₈		10-10d	—						
IUS2.37/14(min)	∠ 16	16	2	12-10d	—	(This sec	tion of th	tion of the table is intentionally blank)			
IUS2.37/14(max)		14		14-10d	—						

TABLE 15A—APPLICABLE MODEL NUMBERS FOR THE IUS SERIES I-JOIST HANGERS¹

For **SI:** 1 inch = 25.4 mm.

¹Model numbers ending with (min) refer to nails installed into only round holes of the hanger, and the model numbers ending with (max) refer to nails installed into both round and triangle holes of the hanger.

²Refer to Figures 15a and 15b (next page) for definitions of hanger nomenclature (W, H, B).

TABLE 15B—ALLOWABLE LOADS FOR THE IUS SERIES I-JOIST HANGERS

	I	DIMENSIONS ¹			N NAILS	li -	ALLOWABLE	LOADS ^{2,5} (lbs)		
MODEL		(inches)		(Quanti	ty-Size)	Uplift ^{3,4}	Download			
NO.	w	н	В	Header	Joist	C _D = 1.6	C _D = 1.0	C _D = 1.15	C _D = 1.25	
				8-10d	—	70	950	1,080	1,165	
	IUS to 2 ⁵ /8	9 ¹ / ₄ to 16		10-10d	—	70	1,185	1,345	1,455	
IUS			2	12-10d	—	70	1,420	1,615	1,745	
				14-10d	—	70	1,660	1,805	1,805	
				16-10d	—	70	1,805	1,805	1,805	
				10-10d	—	70	1,185	1,345	1,455	
1119	IUS to 3 ⁵ /8	9 ¹ / ₂	2	12-10d	—	70	1,420	1,615	1,745	
103		to 16	2 -	14-10d	—	70	1,660	1,805	1,805	
					16-10d	_	70	1,805	1,805	1,805

For **SI:** 1 inch = 25.4 mm, 1 pound = 4.45 N.

¹Refer to Figures 15a and 15b (next page) for definitions of hanger nomenclature (W, H, B).

²Tabulated allowable loads must be selected based on duration of load as permitted by the applicable building code.

³The uplift loads have been increased for wind or earthquake loading with no further increase allowed. The allowable uplift loads need not be reduced when other load durations govern.

⁴Additional uplift capacity is available when installing 2-10d x $1^{1/2}$ -inch-long nails through the triangular holes that are pre-punched in the U-shaped portion of the hanger and into the stiffened web of the prefabricated wood I-joist. When these additional nails are used, the maximum allowable uplift load is 345 lbs (C_D = 1.6).

⁵The allowable loads are based on the use of prefabricated wood I-joists having flanges with an allowable compression perpendicular-to-grain, F_eL, of 750 psi. When use is to support wood I-joists with flanges having an F_eL of less than 750 psi, it must be verified that the combination of bearing capacity and joist nail capacity is adequate.



FIGURE 15A TYPICAL IUS HANGER (SEAT STYLE A)



FIGURE 15B TYPICAL IUS HANGER (SEAT STYLE B)



FIGURE 15C—TYPICAL IUS HANGER INSTALLATION (Supported Wood I-joist without Web Stiffeners)

	DIMENSIONS (in.)		5 ¹	FAOTE	NED034	AL	LOWABLE L	OADS (lbs) ²	2,5,6,7					
MODEL		(in.)		FASTE	NER5	Uplift ^{3,4,5}		Download						
NO.	w	Н	В	Header	Joist	C _D = 1.6	C _D = 1.0	C _D = 1.15	C _D = 1.25					
MIU1.56/7		6 ¹⁵ / ₁₆		10-16d	4-10d x 1 ¹ / ₂	555	1,440	1,635	1,760					
MIU1.56/9	49/	8 ¹⁵ / ₁₆	01/	16-16d	6-10d x 1 ¹ / ₂	965	2,305	2,615	2,820					
MIU1.56/11	1°/ ₁₆	11 ¹ / ₁₆	Z'/2	20-16d	6-10d x 1 ¹ / ₂	965	2,880	3,135	3,135					
MIU1.56/14		13 ⁷ / ₁₆		22-16d	6-10d x 1 ¹ / ₂	965	3,170	3,550	3,610					
MIU1.81/7		6 ¹⁵ / ₁₆		10-16d	4-10d x 1 ¹ / ₂	555	1,440	1,635	1,760					
MIU1.81/9		8 ¹³ / ₁₆		16-16d	6-10d x 1 ¹ / ₂	965	2,305	2,615	2,820					
MIU1.81/11	1 13/	11 ¹ / ₁₆	21/	20-16d	6-10d x 1 ¹ / ₂	965	2,880	3,135	3,135					
MIU1.81/14	I **/16	13 ⁵ / ₁₆	21/2	22-16d	6-10d x 1 ¹ / ₂	965	3,170	3,595	3,875					
MIU1.81/16		15 ⁵ / ₁₆		24-16d	6-10d x 1 ¹ / ₂	965	3,455	3,902	4,045					
MIU1.81/18		17 ⁵ / ₁₆		26-16d	6-10d x 1 ¹ / ₂	965	3,745	4,020	4,045					
MIU2.1/11	2 ¹ / ₈	11 ¹ / ₁₆	2 ¹ / ₂	20-16d	6-10d x 1 ¹ / ₂	965	2,880	3,135	3,135					
MIU2.37/7		6 ¹⁵ / ₁₆		10-16d	4-10d x 1 ¹ / ₂	555	1,440	1,635	1,760					
MIU2.37/9		9		16-16d	6-10d x 1 ¹ / ₂	965	2,305	2,615	2,820					
MIU2.37/11		11 ¹ / ₁₆		20-16d	6-10d x 1 ¹ / ₂	965	2,880	3,135	3,135					
MIU2.37/14	2 ³ / ₈	13 ¹ / ₂	2 ¹ / ₂	22-16d	6-10d x 1 ¹ / ₂	965	3,170	3,595	3,875					
MIU2.37/16		15 ¹ / ₂		24-16d	6-10d x 1 ¹ / ₂	965	3,455	3,920	4,045					
MIU2.37/18		17 ¹ / ₂ 19 ¹ / ₂		26-16d	6-10d x 1 ¹ / ₂	965	3,745	4,045	4,045					
MIU2.37/20				28-16d	6-10d x 1 ¹ / ₂	965	4,030	4,060	4,060					
MIU2.56/9		8 ¹⁵ / ₁₆		16-16d	6-10d x 1 ¹ / ₂	965	2,305	2,615	2,820					
MIU2.56/11		11 ¹ / ₁₆		20-16d	6-10d x 1 ¹ / ₂	965	2,880	3,135	3,135					
MIU2.56/13		12 ¹ / ₄		20-16d	6-10d x 1 ¹ / ₂	965	2,880	3,135	3,135					
MIU2.56/14	2 ⁹ / ₁₆	2 ⁹ / ₁₆ 13 ⁷ / ₁₆	2 ¹ / ₂	2 ¹ / ₂	2 ¹ / ₂	2 ¹ / ₂	2 ¹ / ₂	2 ¹ / ₂	22-16d	6-10d x 1 ¹ / ₂	965	3,170	3,595	3,875
MIU2.56/16		15 ⁷ / ₁₆		24-16d	6-10d x 1 ¹ / ₂	965	3,455	3,920	4,045					
MIU2.56/18		17 ⁷ / ₁₆		26-16d	6-10d x 1 ¹ / ₂	965	3,745	4,045	4,045					
MIU2.56/20		19 ⁷ / ₁₆		28-16d	6-10d x 1 ¹ / ₂	965	4,030	4,060	4,060					
MIU3.12/9	21/	9 ¹ / ₁₆	01/	01/	21/	16-16d	6-10d x 1 ¹ / ₂	965	2,305	2,615	2,820			
MIU3.12/11	37 ₈	11 ¹ / ₈	Z /2	20-16d	6-10d x 1 ¹ / ₂	965	2,880	3,135	3,135					
MIU3.56/9		8 ¹³ / ₁₆		16-16d	6-10d x 1 ¹ / ₂	965	2,305	2,615	2,820					
MIU3.56/11		11 ¹ / ₈		20-16d	6-10d x 1 ¹ / ₂	965	2,880	3,135	3,135					
MIU3.56/14	29/	13 ⁵ / ₁₆	21/	22-16d	6-10d x 1 ¹ / ₂	965	3,170	3,595	3,875					
MIU3.56/16	3 7 ₁₆	15 ⁵ / ₁₆	Z /2	24-16d	6-10d x 1 ¹ / ₂	965	3,455	3,920	4,045					
MIU3.56/18		17 ⁵ / ₁₆		26-16d	6-10d x 1 ¹ / ₂	965	3,745	4,045	4,045					
MIU3.56/20		19 ⁵ / ₁₆		28-16d	6-10d x 1 ¹ / ₂	965	4,030	4,060	4,060					
MIU4.12/9		9 ¹ / ₁₆		16-16d	6-10d x 1 ¹ / ₂	965	2,305	2,615	2,820					
MIU4.12/11	4 1/	11 ¹ / ₈	21/	20-16d	6-10d x 1 ¹ / ₂	965	2,880	3,135	3,135					
MIU4.12/14	4 /8	13 ⁹ / ₁₆	2 /2	22-16d	6-10d x 1 ¹ / ₂	965	3,170	3,595	3,875					
MIU4.12/16		15 ⁹ / ₁₆		24-16d	6-10d x 1 ¹ / ₂	965	3,455	3,920	4,045					
MIU4.28/9		9		16-16d	6-10d x 1 ¹ / ₂	965	2,305	2,615	2,820					
MIU4.28/11	A5/	11 ¹ / ₈	21/	20-16d	6-10d x 1 ¹ / ₂	965	2,880	3,135	3,135					
MIU4.28/14	+ / ₁₆	13 ¹ / ₂	∠ / ₂	22-16d	6-10d x 1 ¹ / ₂	965	3,170	3,595	3,875					
MIU4.28/16		15 ¹ / ₂		24-16d	6-10d x 1 ¹ / ₂	965	3,455	3,920	4,045					

TABLE 16-ALLOWABLE LOADS FOR THE MIU SERIES JOIST HANGERS

Please refer to next page for continuation of Table 16.

	D	IMENSIONS	5 ¹	FACTE	NEDO34	ALLOWABLE LOADS (lbs) ^{2,5,6,7}					
MODEL NO.		(in.)		FASTE	NERS"	Uplift ^{3,4,5}	Download				
	W	Н	В	Header	Joist	C _D = 1.6	C _D = 1.0	C _D = 1.15	C _D = 1.25		
MIU4.75/9		9		16-16d	6-10d x 1 ¹ / ₂	965	2,305	2,615	2,820		
MIU4.75/11		11 ¹ / ₁₆	2 ¹ / ₂	20-16d	6-10d x 1 ¹ / ₂	965	2,880	3,135	3,135		
MIU4.75/14	43/	13 ¹ / ₂		22-16d	6-10d x 1 ¹ / ₂	965	3,170	3,595	3,875		
MIU4.75/16	4%	15 ¹ / ₂		24-16d	6-10d x 1 ¹ / ₂	965	3,455	3,920	4,045		
MIU4.75/18		17 ¹ / ₂		26-16d	6-10d x 1 ¹ / ₂	965	3,745	4,045	4,045		
MIU4.75/20		19 ¹ / ₂		28-16d	6-10d x 1 ¹ / ₂	965	4,030	4,060	4,060		
MIU5.12/7		6 ¹⁵ / ₁₆		10-16d	4-10d x 1 ¹ / ₂	555	1,440	1,635	1,760		
MIU5.12/9		8 ¹³ / ₁₆		16-16d	6-10d x 1 ¹ / ₂	965	2,305	2,615	2,820		
MIU5.12/11		11 ¹ / ₈		20-16d	6-10d x 1 ¹ / ₂	965	2,880	3,135	3,135		
MIU5.12/14	5 ¹ / ₈	13 ⁵ / ₁₆	2 ¹ / ₂	22-16d	6-10d x 1 ¹ / ₂	965	3,170	3,595	3,875		
MIU5.12/16		15 ⁵ / ₁₆		24-16d	6-10d x 1 ¹ / ₂	965	3,455	3,920	4,045		
MIU5.12/18		17 ⁵ / ₁₆		26-16d	6-10d x 1 ¹ / ₂	965	3,745	4,045	4,045		
MIU5.12/20		19 ⁵ /16		28-16d	6-10d x 1 ¹ / ₂	965	4.030	4.060	4.060		

TABLE 16—ALLOWABLE LOADS FOR THE MIU SERIES JOIST HANGERS (Continued)

For SI: 1 inch = 25.4 mm, 1 lbf = 4.45 N.

¹Refer to Figure 16a (this page) for definitions of hanger nomenclature (W, H and B).

²Tabulated allowable loads must be selected based on duration of load as permitted by the applicable building code.

³The uplift loads have been increased for wind or earthquake loading with no further increase allowed. The allowable uplift loads must be reduced when other load durations govern.

⁴Total number of joist nails specified in the table must be installed to resist tabulated uplift loads. The bottom two 10d x 1¹/₂-inch-long nails must be installed at a 45degree angle into the bottom flange of the prefabricated wood I-joist, as shown in the detail in Figure 16b (on this page). These nails are designated as PAN nails. ⁵Web stiffeners must be installed on wood I-joists, with the following exception: An alternate reduced uplift capacity of 230 lbs can be obtained without using web stiffeners and all of the specified joist nails indicated in the table by installing two angled PAN nails at the hanger seat into the bottom flange of the wood I-joist as shown in the detail in Figure 8b (on this page).

⁶For joist members other than prefabricated wood I-joists, lateral support must be provided at the ends in accordance with Section 2308.4.6 of the IBC or Section R502.7 of the IRC.

⁷The tabulated allowable loads are based on the use of prefabricated wood I-joists having flanges with an allowable compression perpendicular-to-grain, $F_{c\perp}$, of 750 psi. When use is to support members having an $F_{c\perp}$ of less than 750 psi, it must be verified that the combination of bearing capacity and joist nail capacity is adequate.



FIGURE 16A-MIU JOIST HANGER



FIGURE 16B—ALTERNATE MIU JOIST HANGER INSTALLATION (See Footnotes 3 and 4 to Table 16)

TABLE 17A-ALLOWABLE LOADS FOR DU, DHU AND DHUTF SERIES JOIST HANGERS

MODEL	PRODUC	CT DIMENSIONS	(inches)		ALLOWABLE LOADS (lbs)		
SERIES	w	н	Ga	В		Uplift C _D =1.60 ^{3,4}	Download C _D =1.00 ²	
DU	1 ⁹ / ₁₆ to 2 ³ / ₈	9 ¹ / ₈ to 15 ¹⁵ / ₁₆	14	2.5	Over (2) layers ⁵ / ₈ " gypsum wallboard	95	1,110	
DHU DHUTF	1 ⁹ / ₁₆ to 3 ⁹ / ₁₆	9 ¹ / ₈ to 23 ¹⁵ / ₁₆	12	2.5	Over (2) layer ⁵ / ₈ " gypsum wallboard	95	1,410 ⁵	

For SI: 1 inch = 25.4 mm, 1 lbf = 4.45N.

¹Loads assume two layers of ⁵/₈-inch-thick Type X gypsum wallboard fastened using, at a minimum, the fastener schedule in Items 14.1-5 in IBC Table 721.1(2). Wall assembly must consist of minimum two 2x4 top plate members with studs spaced not more than 16 inches on center. Top plate member splices must occur at a stud location.

²Hangers spaced closer than 16 inches on center shall reduce capacity proportionately.

³Triangle nail holes may be filled with (4) additional 10d x 1¹/₂inch-long nails to achieve an allowable uplift load of 750 lbs.

⁴DF carried members with minimum 1½ inches x 2¹/₂ inches solid sawn dimensions increase the allowable uplift load to 170 lbs.

⁵Allowable download may be increased to 1,610 lbf for DF/SP using the DHU3.56/24 or DHUTF3.56/24 model hangers.

⁶DU and DHU are mounted with the top of the hanger flush with the top of the wall and tight to the gypsum wallboard.

⁷Wall top plates must be restrained to prevent rotation. Use an SSP stud plate tie to connect the wall top plate to each wall stud on the opposite face of the wall or provide equivalent restraint by another method as determined by the designer. The Simpson Strong-tie Company SSP is a cold-formed, galvanized steel, strap connector that has a bent end that when installed bears on the top of the wall top plate. The SSP is to be attached to the side of the wall top plates with three 10d common nails, and attached to the wall stud with four 10d common nails.

TABLE 17B—FASTENER SCHEDULE FOR DHU, DHU AND DHUTF SERIES HANGERS

SEDIES	FASTENERS						
JERIEJ	Joist ¹	Face	Тор				
DU	(2) 10d x 1 ¹ / ₂ "	(4) SDS ¹ / ₄ " x 3.5"	-				
DHU	(2) 10d x 1 ¹ / ₂ "	(8) SDS ¹ / ₄ " x 3.5"	-				
DHUTF	(2) 10d x 1 ¹ / ₂ "	(8) SDS ¹ / ₄ " x 3.5"	(6) 10d x 1 ¹ / ₂ "				



¹The two nails must be installed in the positive angle nailing (PAN) holes close to the hanger seat.



DHU (DU Similiar)



DHUTF



Typical DHU Installation

FIGURE 17-DU, DHU AND DHUTF HANGERS

Install SSP at each stud or equivalent

MODELNO	DIMENSIONS ¹ (inches)		FASTI (Quanti	ALLOWABLE DOWNLOADS ²	
WIODEL NO.	w	н	Joist	Beam	C _D =1.00/ 1.15/1.25
CBH2.37x5.5	2 ³ / ₈	5 ¹ / ₂	13-SDS ¹ / ₄ " x 6"	13-SDS ¹ / ₄ " x 3"	5,045
CBH2.37x7.63	2 ³ / ₈	7 ⁵ /8	18-SDS ¹ / ₄ " x 6"	18-SDS ¹ / ₄ " x 3"	6,790
CBH2.37x9.75	2 ³ / ₈	9 ³ / ₄	23-SDS ¹ / ₄ " x 6"	23-SDS ¹ / ₄ " x 3"	9,175

TABLE 18A—CBH JOIST TO BEAM ALLOWABLE LOADS^{3,4}

TABLE 18B—CBH JOIST TO COLUMN ALLOWABLE LOADS³

MODELNO	DIMEN (inc	ISIONS ¹ hes)	FASTI (Quanti	ALLOWABLE DOWNLOADS ²		
MODEL NO.	wн		Joist	Beam	C _D =1.00/ 1.15/1.25	
CBH2.37x5.5	2 ³ / ₈	5 ¹ / ₂	13-SDS ¹ / ₄ " x 6"	13-SDS ¹ / ₄ " x 6"	5,460	
CBH2.37x7.63	2 ³ / ₈	7 ⁵ /8	18-SDS ¹ / ₄ " x 6"	18-SDS ¹ / ₄ " x 6"	6,925	
CBH2.37x9.75	2 ³ / ₈	9 ³ / ₄	23-SDS ¹ / ₄ " x 6"	23-SDS ¹ / ₄ " x 6"	9,335	

For SI: 1 inch = 25.4 mm, 1 lbf = 4.45 N.

¹Refer to Figure 18 (this page) for definitions of hanger nomenclature (W, H). ²Tabulated allowable loads must be selected based on duration of load as permitted by the applicable building code.

³The provided capacities are for the following installation conditions:

• The hanger must be installed at the center of the width of the joist, with the stud toward the top of the joist.

• The joist width must be a minimum of 5.5 inches.

• The minimum distance measured vertically from the top of the hanger to the top fibers of the joist must be a minimum of 17/_b inches.

The minimum distance measured vertically from the bottom of the hanger to the bottom fibers of the joist must be a minimum of 7/8 inch. ٠

• The minimum distance measured vertically from the top of the hanger to the top fibers of the beam must be a minimum of 7/8 inch.

• The minimum distance measured vertically from the bottom of the hanger to the bottom fibers of the beam must be a minimum of 13/16 inches.

• The supporting member must be thick enough to allow the fasteners to be fully encapsulated in the wood.

⁴The torsion capacity of the CBH hanger is outside the scope of this evaluation report.







CBH Series Hanger

Typical CBH Beam-Beam Installation Beam-Column Installation Similar

Typical CBH Assembly

FIGURE 18—CBH CONCEALED BEAM HANGERS

TABLE 19—ACBH ALLOWABLE LOADS^{3,4}

MODEL	DIMEN (inc	SIONS ¹ hes)		FASTENERS (Quantity – Type)	ALLOWABLE DOWNLOADS ² (lbf)		
MODEL	w	н	Supported Beam	Supportin Straight⁵	lg Column Inclined⁵	C _D = 1.00	C _D = 1.15/1.25
ACBH3x15.37	3	15 ³ / ₈	(24) SDCF22614	(7) SDCF22434	(17) SDCF22434	19,870	21,025
ACBH3x15.37	3	15 ³ / ₈	(24) SDCF22614	(7) SDCF22614	(17) SDCF22614	20,050	21,285

For SI: 1 inch = 25.4 mm, 1 lbf = 4.45 N.

¹Refer to Figure 19 (this page) for definitions of hanger nomenclature (W, H).

²Tabulated allowable loads must be selected based on duration of load as permitted by the applicable building code.

³The provided capacities are for the following installation conditions:

The hanger must be installed at the center of the width of the beam, with the stud toward the top of the beam. •

- The beam width must be a minimum of $5^{3}/_{4}$ inches. •
- The minimum distance measured vertically from the top of the hanger to the top fibers of the beam must be a minimum of 1³/₄ inches.
- The minimum distance measured vertically from the bottom of the hanger to the bottom fibers of the beam must be a minimum of 1 inch.
 The column must be wide enough to fit the full width of the beam and deep enough to allow the fasteners to be fully encapsulated in the wood. ⁴The torsion capacity of the ACBH hanger is outside the scope of this evaluation report.

⁵SDCF screws are installed perpendicular to the wood face (straight) and at a 45 degree inclined angle (inclined).



ACBH Series Hanger



Typical ACBH Beam-Column Installation



Typical ACBH Assembly

FIGURE 19—ACBH CONCEALED BEAM HANGER

	DIN	IENSIONS ¹	(inche	s)	BEAR	ING ³	SDCF FASTENERS (Quantity) Type					
MODEL		H L H _s					Suported Beam		Supporting Column			(lbf)
	w		B _{w-min} B _{I-min}	Beam Seat	Bearing Reinf.	Shear Group A	Inclined Group A	Shear Group B	C₀=1.00/ 1.15/1.25			
HSKP5.75x14.5	5 ³ /4	14 ¹ / ₂	9	7	2 ¹ / ₂	8	(2)27614	-	(6)27614	(8)27614	(6)22858	22,420
HSKP5.75x17	5 ³ /4	17	9	8	2 ¹ / ₂	8	(2)27614	(2)22858	(6)27614	(12)27614	(6)22858	26,800
HSKP5.75x19.5	5 ³ / ₄	19 ¹ / ₂	9	9 ¹ / ₄	2 ³ / ₈	8	(2)27614	(4)22858	(6)27614	(16)27614	(6)22858	31,930

For SI: 1 inch = 25.4 mm, 1 lbf = 4.45 N

¹Refer to Figure 20 (this page) for definitions of hanger nomenclature (W,H,L,H_s) and bearing area requirements (B_{w-min}, B_{I-min})

²The provided capacities are for the following conditions:

• The hanger must be installed at the center of the width of the beam.

• The carried beam width must be a minimum of 5³/₄ inches and its height above the HSKP bearing plate must be greater than or equal to H.

• The column must have a minimum width of 81/2 inches and must be deep enough to allow the fasteners to be fully encapsulated in the wood.

³Bw-min and Bi-min are the minimum bearing width and length, respectively. The reinforced bearing connection between the HSKP5.75x17 and HSKP5.75x19.5 hanger and the supported beam is outside the scope of this evaluation report.

⁴The torsion capacity of the HSKP hanger is outside the scope of this evaluation report.

⁵Instructions for installation of HSKP fasteners:

- Install SDCF27614 screws, perpendicular to the wood face, through the six round holes in Shear Group A, depicted in Figure 20, and SDCF22858 screws, perpendicular to the wood face, through the six round holes in Shear Group B.
- Install SDCF27614 screws through the shear group A tab features inclined 45 degrees up and rotated 15 degrees toward the middle of the back plate filling all tab hole locations. See Figure 20 for inclined and rotated SDCF orientation.
- Install SDCF22858 bearing reinforcement screws into the bottom of the supported member as required. To insure SDCF fasteners are normal to the bottom face
 of the supported member, pre-drill ⁵/₃₂-inch (4 mm) pilot holes prior to installation. See Figure 20 for SDCF bearing reinforcement location requirements.
- With the carried beam in position on the HSKP bearing plate, install the two SDCF27614 screws into the bottom of the carried member through the two countersunk holes in the bottom of the HSKP bearing plate.





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Typical HSKP (HSKP5.75X19.5 shown)

HSKP Dimensions and Inclined SDCF Orientation



Top View of HSKP Indicating SDCF Bearing Reinforcement Locations and Carried Member Minimum Bearing Area Requirements



Typical HSKP Assembly

FIGURE 20—HEAVY SEATED KNIFE PLATE HANGER



ICC-ES Evaluation Report

ESR-2552 LABC and LARC Supplement

Reissued March 2023 Revised September 2023

This report is subject to renewal March 2024.

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DIVISION: 06 00 00—WOOD, PLASTICS AND COMPOSITES Section: 06 05 23—Wood, Plastic, and Composite Fastenings

REPORT HOLDER:

SIMPSON STRONG-TIE COMPANY INC.

EVALUATION SUBJECT:

SIMPSON STRONG-TIE[®] FACE-MOUNT HANGERS FOR SAWN LUMBER, STRUCTURAL COMPOSITE LUMBER (SCL), PREFABRICATED WOOD I-JOISTS AND GLULAM BEAMS (ENGINEERED WOOD PRODUCTS)

1.0 REPORT PURPOSE AND SCOPE

Purpose:

The purpose of this evaluation report supplement is to indicate that Simpson Strong-Tie[®] face-mount hangers for sawn lumber, structural composite lumber (SCL), prefabricated wood I-joists and glulam beams (engineered wood products), described in ICC-ES evaluation report <u>ESR-2552</u>, have also been evaluated for compliance with the codes noted below as adopted by the Los Angeles Department of Building and Safety (LADBS).

Applicable code editions:

- 2023 City of Los Angeles Building Code (LABC)
- 2023 City of Los Angeles Residential Code (LARC)

2.0 CONCLUSIONS

The Simpson Strong-Tie[®] face mount hangers for sawn lumber, structural composite lumber (SCL), prefabricated wood l-joists and glulam beams (engineered wood products), described in Sections 2.0 through 7.0 of the evaluation report <u>ESR-2552</u>, comply with the LABC Chapter 23, and the LARC, and are subjected to the conditions of use described in this supplement.

3.0 CONDITIONS OF USE

The Simpson Strong-Tie[®] face mount hangers for sawn lumber, structural composite lumber (SCL), prefabricated wood I-joists and glulam beams (engineered wood products), described in this evaluation report supplement, must comply with all of the following conditions:

- All applicable sections in the evaluation report ESR-2552.
- The design, installation, conditions of use and identification are in accordance with the 2021 International Building Code[®] (2021 IBC) provisions noted in the evaluation report <u>ESR-2552</u>.
- The design, installation and inspection are in accordance with additional requirements of LABC Chapters 16 and 17, as applicable.
- The supported end of a joist or beam must be within ¹/₄-inch from the supporting member.
- Solid blocking must be required for all joist hangers supporting roof joists having one end twisted more than one-half degree per foot of length relative to the other end, except as specifically noted in the evaluation report.
- Under the LARC, an engineered design in accordance with LARC Section R301.1.3 must be submitted.

This supplement expires concurrently with the evaluation report, reissued March 2023 and revised September 2024.

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ICC-ES Evaluation Report

ESR-2552 FBC Supplement

Reissued March 2023 Revised September 2023 This report is subject to renewal March 2024.

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DIVISION: 06 00 00—WOOD, PLASTICS AND COMPOSITES Section: 06 05 23—Wood, Plastic, and Composite Fastenings

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SIMPSON STRONG-TIE COMPANY INC.

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SIMPSON STRONG-TIE[®] FACE-MOUNT HANGERS FOR SAWN LUMBER, STRUCTURAL COMPOSITE LUMBER (SCL), PREFABRICATED WOOD I-JOISTS AND GLULAM BEAMS (ENGINEERED WOOD PRODUCTS)

1.0 REPORT PURPOSE AND SCOPE

Purpose:

The purpose of this evaluation report supplement is to indicate that the Simpson Strong-Tie[®] face-mount hangers, described in ICC-ES evaluation report ESR-2552, have also been evaluated for compliance with the codes noted below.

Applicable code editions:

- 2023 Florida Building Code—Building
- 2023 Florida Building Code—Residential

2.0 CONCLUSIONS

The Simpson Strong-Tie[®] face-mount hangers, described in Sections 2.0 through 7.0 of ICC-ES evaluation report ESR-2552, comply with the *Florida Building Code—Building,* and the *Florida Building Code—Residential,* provided the design requirements are determined in accordance with the *Florida Building Code—Building* or the *Florida Building Code—Residential,* as applicable. The installation requirements noted in ICC-ES evaluation report ESR-2552 for the 2021 *International Building Code*[®] meet the requirements of the *Florida Building Code—Building* or the *Florida Building Code—Residential,* as applicable.

Use of the Simpson Strong-Tie[®] face-mount hangers has also been found to be in compliance with the High-Velocity Hurricane Zone provisions of the *Florida Building Code—Building*, and the *Florida Building Code—Residential* with the following condition:

a. For connections subject to uplift, the connection must be designed for no less than 700 pounds (3114 N).

For products falling under Florida Rule 61G20-3, verification that the report holder's quality assurance program is audited by a quality assurance entity approved by the Florida Building Commission for the type of inspections being conducted is the responsibility of an approved validation entity (or the code official when the report holder does not possess an approval by the Commission).

This supplement expires concurrently with the evaluation report, reissued March 2023 and revised September 2024.

